US ERA ARCHIVE DOCUMENT

Coal Combustion Waste Impoundment Round 5 - Dam Assessment Report

Plant Wansley (Site # 006)
Ash Pond Separation Dike

Georgia Power Carrolton, Georgia

Prepared for:

United States Environmental Protection Agency Office of Resource Conservation and Recovery

Prepared by:

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Under Contract Number: EP-09W001727 **July 2010**

INTRODUCTION, SUMMARY, CONCLUSION AND RECOMMENDATIONS

The release of over five million cubic yards of coal ash from the Tennessee Valley Authority's Kingston, Tennessee facility in December 2008, which flooded more than 300 acres of land, damaging homes and property, is a wake-up call for diligence on coal combustion waste disposal units. . A first step to prevent such catastrophic failure and damage is to assess the stability and functionality of ash impoundments and other units, then quickly take any needed corrective measures.

This assessment of the stability and functionality of the Plant Wansley fly ash management unit is based on a review of available documents and on the site assessment conducted by Dewberry personnel on June 30, 2010. We found the supporting technical information adequate (Section 1.1.3). As detailed in Section 1.2.6 there are recommendations that may help to maintain a safe and trouble-free operation,

In summary, the Wansley Plant ash ponds are SATISFACTORY for continued safe and reliable operation, with no apparent existing or potential management unit safety deficiencies.

PURPOSE AND SCOPE

The U. S. Environmental Protection Agency (EPA) is investigating the potential for catastrophic failure of Coal Combustion Surface Impoundments (i.e. management unit) at electric utilities in an effort to protect lives and property from the consequences of a dam failure or the improper release of impoundment slurry. The EPA initiative is intended to identify conditions that may adversely affect the structural stability and functionality of a management unit and its appurtenant structures (if present); to note the extent of deterioration (if present); status of maintenance and/or a need for immediate repair; to evaluate conformity with current design and construction practices, and to determine the hazard potential classification for units not currently classified by the management unit owner or by a state or federal agency. The initiative addresses power plant management units that have a classification of Less-than-Low, Low, Significant or High Hazard Potential ranking. (For Classification, see pp. 3-8 of the 2004 Federal Guidelines for Dam Safety)

In December 2009, the EPA sent letters to coal-fired electric utilities seeking information on the safety of surface impoundments and similar facilities that receive liquid-borne material that store or dispose if coal combustion waste. This letter was issued under the authority of the Comprehensive Environmental Response, Compensation and Liability Act (CERCLA) Section 104(e), to assist the Agency in assessing the structural stability and functionality of such management units, including which facilities should be visited to perform a safety assessment of the berms, dikes, and dams used in the construction of these impoundments.

EPA asked utility companies to identify all management units: surface impoundments or similar diked or bermed structures; and; landfills receiving liquid-borne materials that store or dispose of

coal-combustion residuals or by-products, including, but not limited to, fly ash, bottom ash, boiler slag, and flue gas emission control residuals. Utility companies responded with information on the size, design, age, and the amount of material placed in the units so that EPA could gauge which management units had or potential could rank as having High Hazard Potential. The USEPA and its contractors used the following definitions for this study:

"Surface Impoundment or impoundment means a facility or part of a facility which is a natural topographic depression, man-made excavation, or diked area formed primarily of earthen materials (although it may be lined with man-made materials), which is designed to hold an accumulation of liquid wastes or wastes containing free liquids, and which is not an injection well. Examples of surface impoundments are holding, storage, settling and aeration pits, ponds, and lagoons."

For this study, the earthen materials could include coal combustion residuals. EPA did not provide an exclusion for small units based on whether the placement was temporary or permanent. Furthermore, the study covers not only waste units designated as surface impoundments, but also other units designated as landfills which receive free liquids.

EPA is addressing any land-based units that receive fly ash, bottom ash, boiler slag, or flue gas emission control waster along with free liquids. If the landfill is receiving coal combustion wastes with liquids limited to that for proper compaction, then there should not be free liquids present and the EPA did not seek information on such units which are appropriately designated a landfill.

In some cases coal combustion wastes are separated from the water, and the water containing de minimum levels of fly ash, bottom ash, boiler slag, or flue gas emission control wastes are sent to an impoundment. EPA is including such impoundments in this study, because chemicals of concern may have leached from the solid coal combustion wastes into the waster waters, and the suspended solids from the coal combustion wastes remain.

The purpose of this report is to **evaluate the condition and potential of waste release from management units that have not been rated for hazard potential classification**. A two-person team reviewed the information submitted to EPA, reviewed any relevant publicly available information from state or federal agencies regarding the unit potential hazard classification (if any) and accepted information provided via telephone communication with a management unit representative.

This evaluation included a site visit. EPA sent two engineers, one licensed in the State of Georgia, for a one-day visit. The two-person team met with the owner of the management unit as well as technical and several technical representative and management unit supervisors to discuss

the engineering characteristics of the unit as part of the site visit. During the site visit the team collected additional information about the management unit to be used in determining the hazard potential classifications of the management unit(s). Subsequent to the site visit the management unit owner provided additional engineering data pertaining to the management unit(s).

Factors considered in determining the hazard potential classification of the management unit(s) included the age and size of the impoundment, the quantity of coal combustion residuals or byproducts that were stored or disposed in these impoundments, its past operating history, and its geographic location relative to down gradient population centers and/or sensitive environmental systems.

This report presents the opinion of the assessment team as to the potential of catastrophic failure and reports on the condition of the management units(s). The team considered criteria in evaluating the dams under the National Inventory of Dams in making these determinations.

LIMITATIONS

The assessment of dam safety reported herein is based on field observations and review of readily available information provided by the owner/operator of the subject coal combustion waste management unit(s). Qualified Dewberry engineering personnel performed the field observations and review and made the assessment in conformance with the required scope of work and in accordance with reasonable and acceptable engineering practices. No other warranty, either written or implied, is made with regard to our assessment of dam safety.

Contents

1.0	CONC	CLUSIONS AND RECOMMENDATIONS	1-1
1	.1 CO	NCLUSIONS	1-1
	1.1.1	Conclusions Regarding the Structural Soundness of the Management Unit(s)	1-1
	1.1.2	Conclusions Regarding the Hydrologic/Hydraulic Safety of the Management Unit 1-1	t(s)
	1.1.3	Conclusions Regarding the Adequacy of Supporting Technical Documentation	1-1
	1.1.4	Conclusions Regarding the Description of the Management Unit(s)	1-1
	1.1.5	Conclusions Regarding the Field Observations	
	1.1.6	Conclusions Regarding the Adequacy of Maintenance and Methods of Operation	
	1.1.7	Conclusions Regarding Adequacy of the Surveillance and Monitoring Program.	
	1.1.8	Classification Regarding Suitability for Continued Safe and Reliable Operation	
1	1.2 RE	COMMENDATIONS	
	1.2.1	Recommendations Regarding the Structural Stability	
	1.2.2	Recommendations Regarding the Hydrologic/Hydraulic Safety	
	1.2.3	Recommendations Regarding the Supporting Technical Documentation	
	1.2.4	Recommendations Regarding the Description of the Management Unit(s)	
	1.2.5	Recommendations Regarding the Field Observations	
	1.2.6	Recommendations Regarding the Maintenance and Methods of Operation	
	1.2.7	0 0	
	1.2.8	Recommendations Regarding Continued Safe and Reliable Operation	
1		RTICIPANTS AND ACKNOWLEDGEMENT	
	1.3.1	List of Participants	
	1.3.2	Acknowledgement and Signature	1-3
2.0	DESC	CRIPTION OF THE COAL COMBUSTION WASTE MANAGEMENT UNIT(S).	2-1
2		CATION	
_		E AND HAZARD CLASSIFICATION	2-2
		IOUNT AND TYPE OF RESIDUALS CURRENTLY CONTAINED IN THE	
	` ′) AND MAXIMUM CAPACITY	
2	2.4 PRI	NCIPAL PROJECT STRUCTURES	
	2.4.1		
	2.4.2	Outlet Structures	
2	2.5 CR	ITICAL INFRASTRUCTURE WITHIN FIVE MILES DOWN GRADIENT	2-8
3.0	SUM	MARY OF RELEVANT REPORTS, PERMITS AND INCIDENTS	3-1
3	3.1 SU	MMARY OF REPORTS ON THE SAFETY OF THE MANAGEMENT UNIT(S)	3-1
3	3.2 SU	MMARY OF LOCAL, STATE AND FEDERAL ENVIRONMENTAL PERMITS.	3-2
3	3.3 SU	MMARY OF SPILL/RELEASE INCIDENTS (IF ANY)	3-2

4.0 SUMMARY OF HISTORY OF CONSTRUCTION AND OPERATION	4-1
4.1 SUMMARY OF CONSTRUCTION HISTORY	4-1
4.1.1 Original Construction	
4.1.2 Significant Changes/Modifications in Design since Original Construction	4-1
4.1.3 Significant Repairs/Rehabilitation since Original Construction	
4.2 SUMMARY OF OPERATIONAL HISTORY	4-2
4.2.1 Original Operational Procedures	4-2
4.2.2 Significant Changes in Operational Procedures since Original Startup	4-2
4.2.3 Current Operational Procedures	
4.2.4 Other Notable Events since Original Startup	4-2
5.0 FIELD OBSERVATIONS	5-1
5.1 PROJECT OVERVIEW AND ASSESSMENT	
5.2 EARTH EMBANKMENT DAM	
5.2.1 II Crest	
5.2.2 Upstream Slope5.2.3 Downstream Slope and Toe	
5.2.4 Abutments and Groin Areas	
5.2.4 Adulments and Groth Areas	
5.3.1 Overflow Structure	
5.3.2 Outlet Conduit	
5.3.3 Emergency Spillway (If Present)	
5.3.4 Low Level Outlet	
6.0 HYDROLOGIC/HYDRAULIC SAFETY	
6.1 SUPPORTING TECHNICAL DOCUMENTATION	
6.1.1 Floods of Record	6-1
6.1.2 Inflow Design Flood	
6.1.3 Spillway Rating	
6.1.4 Downstream Flood Analysis	
6.2 ADEQUACY OF SUPPORTING TECHNICAL DOCUMENTATION	
6.3 ASSESSMENT OF HYDROLOGIC/HYDRAULIC SAFETY	6-2
7.0 STRUCTURAL STABILITY	7-1
7.1 SUPPORTING TECHNICAL DOCUMENTATION	7 1
7.1.1 Stability Analyses and Load Cases Analyzed	
7.1.2 Design Properties and Furameters of Materials	
7.1.4 Factors of Safety and Base Stresses	
7.1.5 Liquefaction Potential	
7.1.6 Critical Geological Conditions and Seismicity	
7.2 ADEQUACY OF SUPPORTING TECHNICAL DOCUMENTATION	7-7
7.3 ASSESSMENT OF STRUCTURAL STABILITY	7-7
8.0 MAINTENANCE AND METHODS OF OPERATION	8_1
X II. MAANA HINANG HAMID MHIHUUN OHOPHRA HUN	X_ I

8.1 OPERATIONAL PROCEDURES	8-1
8.2 MAINTENANCE OF THE DAM AND PROJECT FACILITIES	8-1
8.3 ASSESSMENT OF MAINTENANCE AND METHODS OF OPERATION	8-1
8.3.1 Adequacy of Operational Procedures	8-1
8.3.2 Adequacy of Maintenance	
9.0 SURVEILLANCE AND MONITORING PROGRAM	
9.1 SURVEILLANCE PROCEDURES	9-1
9.2 INSTRUMENTATION MONITORING	9-1
9.2.1 Instrumentation Plan	9-1
9.2.2 Instrumentation Monitoring Results	9-1
9.2.3 Evaluation	9-1
9.3 ASSESSMENT OF SURVEILLANCE AND MONITORING PROGRAM	
9.3.1 Adequacy of Inspection Program	9-1
9.3.2 Adequacy of Instrumentation Monitoring Program	

APPENDICES

APPENDIX A – REFERENCE DOCUMENTS

- Doc 1: 2009 4th Quarter Dam Safety Inspection Report.pdf
- Doc 2: 2009 1st Quarter Dam Safety Inspection Report.pdf
- Doc 3: 2008 4th Quarter Dam Safety Inspection Report.pdf
- Doc 4: 2008 3rd Quarter Dam Safety Inspection Report.pdf
- Doc 5: 2008 1st Quarter Dam Safety Inspection Report.pdf
- Doc 6: Earth Embankment Report.pdf
- Doc 7: Ash Pond Profile.pdf
- Doc 8: Separation Dike Stability Analysis (1of3).pdf
- Doc 9: Separation Dike Stability Analysis (2of3).pdf
- Doc 10: Separation Dike Stability Analysis (3of3).pdf
- Doc 11: Separation Dike Profiles.pdf
- Doc 12: Gypsum Overall Plant View.pdf
- Doc 13: Ash Pond Construction Dwg.pdf
- Doc 14: Separation Dam Plan View.pdf
- Doc 15: Ash Pond Overall.pdf
- Doc 16: Plant Wansley Ash Pond Discharge Structure.pdf
- Doc 17: Slope Stability Analysis.pdf
- Doc 18: Liquefaction Potential.pdf
- Doc 19: Stormwater Capcity.pdf

APPENDIX B - SITE ASSESSMENT DOCUMENTATION

- Doc 1: Coal Combustion Dam Inspection Checklist Form Georgia Power Wansley
- Doc 2: Separation Dike Photo Logs.pdf
- Doc 3: West Dike Photo Log.pdf
- Doc 4: Photographs.pdf

APPENDIX C - CORRESPONDENCE & ADDITIONAL REFERENCE DOCUMENTATION

Reserved

1.0CONCLUSIONS AND RECOMMENDATIONS

1.1 CONCLUSIONS

Conclusions are based on visual observations from the one-day site visit, review of technical documentation provided by Georgia Power, and review of state inspection reports.

1.1.1 Conclusions Regarding the Structural Soundness of the Management Unit(s)

The structural stability of the Ash Pond embankments appears to be in **Satisfactory** condition. Slope stability analyses were not provided for the western dike; however, foundation preparation, embankment soils and geometry are consistent. Performance of a separate slope stability analysis is not deemed critical.

1.1.2 Conclusions Regarding the Hydrologic/Hydraulic Safety of the Management Unit(s)

Adequate capacity and freeboard exist to safely pass the design storm.

1.1.3 Conclusions Regarding the Adequacy of Supporting Technical Documentation Supporting technical documentation was adequate.

1.1.4 Conclusions Regarding the Description of the Management Unit(s)

Descriptions provided are appropriate.

1.1.5 Conclusions Regarding the Field Observations

The emergency overflow concrete channel was cracking and showed a potential to be undermined in future rain events. This is not a safety issue at this time, but needs to be monitored.

1.1.6 Conclusions Regarding the Adequacy of Maintenance and Methods of Operation

Maintenance and methods of operation are adequate.

1.1.7 Conclusions Regarding the Adequacy of the Surveillance and Monitoring Program

Existing surveillance and monitoring programs are adequate.

1.1.8 Classification Regarding Suitability for Continued Safe and Reliable Operation

Facility is SATISFACTORY for continued safe and reliable operation. A classification of "satisfactory" is appropriate when no existing or potential management unit safety deficiencies are recognized. Acceptable performance is expected under all applicable loading conditions (static, hydrologic, seismic) in accordance with the applicable criteria. Minor maintenance items may be required.

1.2 RECOMMENDATIONS

1.2.1 Recommendations Regarding the Structural Stability

Continue with the current maintenance and inspection programs set in place.

1.2.2 Recommendations Regarding the Hydrologic/Hydraulic Safety

None appear warranted at this time.

1.2.3 Recommendations Regarding the Supporting Technical Documentation

Although not deemed a critical analysis, additional documentation is needed to assess the slope stability analysis of the western dike.

1.2.4 Recommendations Regarding the Description of the Management Unit(s)

None appear warranted at this time.

1.2.5 Recommendations Regarding the Field Observations

None appear warranted at this time.

1.2.6 Recommendations Regarding the Maintenance and Methods of Operation

None appear warranted at this time.

1.2.7 Recommendations Regarding the Surveillance and Monitoring Program

Continue current program. Begin monitoring erosion at concrete channel to avoid channel being undermined. Monitor cracking along the concrete channel.

1.2.8 Recommendations Regarding Continued Safe and Reliable Operation

 Perform slope stability analyses with seismic loading conditions for western dike.

1.3 PARTICIPANTS AND ACKNOWLEDGEMENT

1.3.1 List of Participants

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Justin Story, E.I. – Dewberry & Davis, Inc.

1.3.2 Acknowledgement and Signature

We acknowledge that the management unit	referenced herein has been assessed on
June 30, 2010.	
Frederic M. Shmurak, PE, Civil Engineer	Justin R. Story, E.I., Civil Designer

2.0DESCRIPTION OF THE COAL COMBUSTION WASTE MANAGEMENT UNIT(S)

2.1 LOCATION

Plant Wansley's ash pond facility is located just south of Carrolton, Georgia. The ash pond dike is approximately 0.5 miles from the Chattahoochee River. The Town of Centralhatchee is approximately 4 miles downstream of the ash pond embankments. Figure 2.1a depicts a vicinity map around Plant Wansley, while Figure 2.1b depicts an aerial view of Plant Wansley.



Figure 2.1 a: Plant Wansley Vicinity Map



Figure 2.1 b: Plant Wansley Aerial View

2.2 SIZE AND HAZARD CLASSIFICATION

The ash pond is impounded by an earthen embankment system consisting of a dike (hereby referred to as the separation dike) that separates the pond from a large reservoir. A smaller dike (hereby referred to as the western dike) existing near the emergency spillway and outlet of the ash pond was also observed. Based on data provided by Georgia Power, Inc. the ash pond embankment system was constructed to a maximum height of 110 feet with a crest width of 35.4 feet, side slopes of 3(H):1(V) to 2.5(H):1(V) and a length just under 3,000 feet. The maximum storage volume corresponding to the top of the embankment is 16,920 acre-feet or 27,297,333 cubic yards (see Table 2.2b). The water elevation ranges from 795 to 799' and at 799' there is approximately 1,001 acre-feet of storage remaining. The classification for size, based on the height of the dam and storage capacity, is Intermediate in accordance with the USACE Recommended Guidelines for Safety Inspection of Dams ER 1110-2-106 criteria (see Table 2.2a for size classification criteria).

Table 2.2a USACE ER 1110-2-106 Size Classification					
	Impour	ndment			
Category	Storage (Ac-ft)	Height (ft)			
Small	< 1,000	< 40			
Intermediate 1,000 to < 50,000 40 to < 100					
Large > 50,000 > 100					

Table 2.2b: Summary of Dam Dimensions and Size					
Ash Pond					
Dam Height (ft)	110				
Crest Width (ft)	35.4				
Length (ft)	≈3,000				
Side Slopes (upstream) H:V	3 to 2.5:1				
Side Slopes (downstream) H:V 3 to 2.5:1					
Hazard Classification	Hazard Classification Low				

The ash pond embankment system has been assigned a Hazard Classification of <u>Low</u> by the Georgia Safe Dams Program (GSDP). The low hazard classification was assigned by the state due to the minimal economic damage that would result from improper operation or dam failure. Per the Federal Guidelines for Dam Safety dated April 2004, a low hazard potential classification applies to those dams where failure or misoperation results in no probable loss of human life and low economic and/or environmental losses. Losses are principally limited to the owner's property. Considering the low probability of loss of life as well as the low economic and/or environmental impacts, a Federal Hazard Classification of <u>Low</u> is appropriate for this facility (see Table 2.2c Federal Guidelines for Hazard Classification criteria).

* GSDP assigned a hazard classification of Low due to potential minimal economic loss due to failure.

Table 2.2c FEMA Federal Guidelines for Dam Safety Hazard Classification					
Hazard Potential Economic, Environmental, Lifel Classification Loss of Human Life Losses					
Low	None Expected	Low and generally limited to owner			
Significant	None Expected	Yes			
High	Probable. One or more expected	Yes (but not necessary for this classification)			

2.3 AMOUNT AND TYPE OF RESIDUALS CURRENTLY CONTAINED IN THE UNIT(S) AND MAXIMUM CAPACITY

Per Georgia Power, the ash pond primarily contains fly, bottom ash, boiler slag, flue gas emission control residues, pyrites and other low volume waste. Other materials that the pond may contain are ash sluice water, categorical low volume wastewater, coal pile storm water runoff and other storm water. The drainage area for the ash pond is approximately 711 acres while the surface area of the pond is approximately 343 acres. The maximum design storage capacity is approximately 16,920 acre-feet or 27,297,333 cubic yards.

Table 2.3: Amount of Residuals and Maximum Capacity of Unit*				
Ash Pond				
Surface Area (acre)	711			
Current Storage Volume (acre-feet) 8,321				
Max. Design Storage Capacity (acre-				
feet)	16,920			

2.4 PRINCIPAL PROJECT STRUCTURES

2.4.1 Earth Embankment Dam

The dam embankment generally consists of lean clays and silts obtained from borrow areas. Some sandy material was found in the borrow areas, but was tested and reported to be satisfactory per the design standards. A plan view of the Ash Pond is depicted in Figure 2.4.1 a. (Figures 2.4.1 a and b reflect conditions of the Ash Pond, per the Design Drawings prepared in 1976 and 1973 respectively. Additional drawings of the ash pond are included within Appendix A (Doc 07: Plant Wansley Unit No. 1 Ash Pond.pdf, Doc 13: Ash Pond Construction Drawing, Doc 14: Separation Dam Plan View.pdf and Doc 15: Ash Pond Overall).

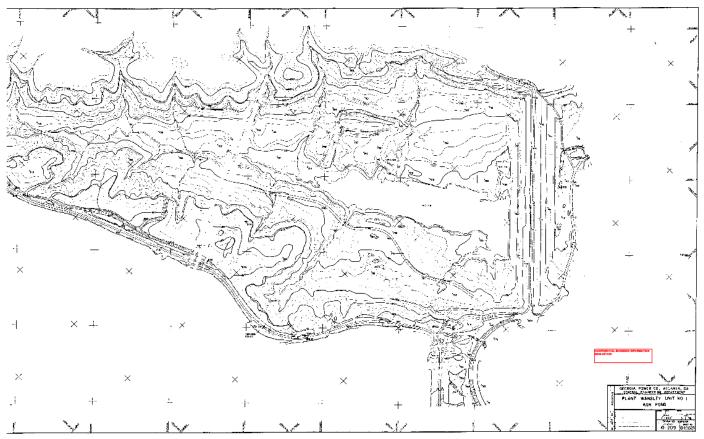


Figure 2.4.1 a: Plant Wansley Unit No 1 – Ash Pond

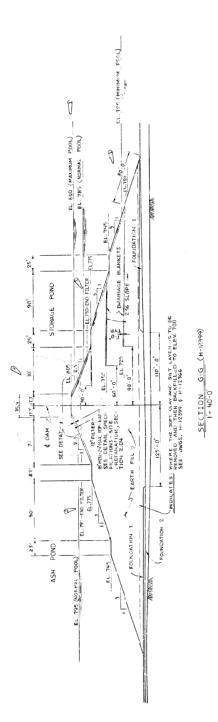


Figure 2.4.1 b: Ash Pond Separation Dike

2.4.2 Outlet Structures

The outlet works consist of a broad crested weir and an open channel emergency spillway. Due to circulation and re-use of sluice water, the plant regulates the ash pond water surface elevation below the invert of the outlet works and has no record of the skimmer weir or the spillway ever being used. During the design of the ash

pond unit, some late changes were made to divert storm water flow away from the pond which eliminates a majority of the elevation fluctuation due to storm events. Once the skimmer weir elevation is breached the flow travels through a Corrugated Metal Pipe (CMP) to the downstream storm water pond.

2.5 CRITICAL INFRASTRUCTURE WITHIN FIVE MILES DOWN GRADIENT

All critical infrastructures were located using aerial photography and might not accurately represent what currently exists down-gradient of the site. Figure 2.5 shows Plant Wansley and associated critical infrastructure, listed in Table 2.5.

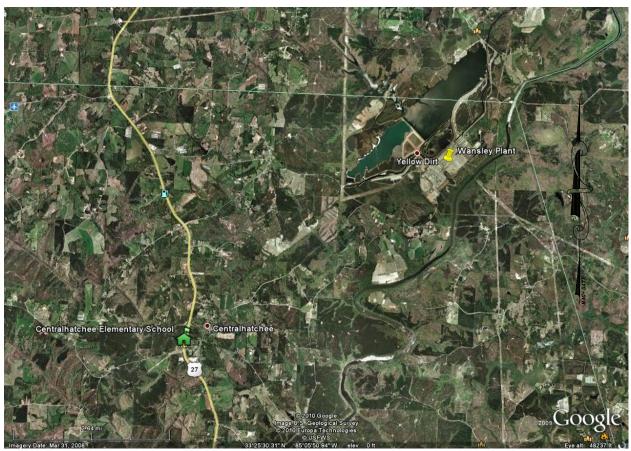


Figure 2.5: Plant Wansley Critical Infrastructure Map

Table 2.5: Plant Wansley Critical Infrastructure Within 5 Miles							
Schools	Transportation	Nursing Homes					
Central Hatchee Elementary	Martha Berry Hwy (US Hwy						
School	27)	None					
315 Central Hatchee							
Parkway							
Franklin, GA 30217							
		Fire Stations					
		None					

3.0SUMMARY OF RELEVANT REPORTS, PERMITS AND INCIDENTS

3.1 SUMMARY OF REPORTS ON THE SAFETY OF THE MANAGEMENT UNIT(S)

Approximately thirteen (13) quarterly safety inspection reports were provided by Georgia Power dating back to 2005. Information pertaining to the separation dike of the most recent reports is summarized below.

Southern Company Generation 2009 Inspection Report for Plant Wansley Ash Pond Complex, REA # WN-08900, December 9, 2009 (Appendix A, Doc 1: 2009 4th Quarter Dam Safety Inspection Report):

• The piezometers on the separation dike are generally registering within their historical ranges. Piezometer BB is registering an elevated water level and has generally followed the change in the ash pond elevation. This piezometer was repaired after being damaged in 2008.

Southern Company Generation 2009 Inspection Report for Plant Wansley Ash Pond Complex, REA # WN-08900, January 12, 2009 (Appendix A, Doc 3: 2008 4th Quarter Dam Safety Inspection Report):

- It was noticed the piezometer BB had been broken off and buried during the recent gypsum storage facility construction activities. Water levels in the piezometer appear to be higher, but this may be caused by inspectors using a different reference point to measure the water levels with the pipe being broken off. Plant personnel are inspecting and will advise Hydro Services.
- Current Recommendations: Cracks in concrete lined ditches should be cleaned out and caulked. **Pending completion open.**
- Status of Previous Recommendations: Upstream and Downstream slopes Localized erosion rills/gullies need to be repaired to mitigate further erosion. Fixed
 closed.
- Status of Previous Recommendations: Runoff erosion at crest of upstream slope repaired. **Closed.**

Southern Company Generation 2008 Inspection Report for Plant Wansley Ash Pond Complex, REA # WN-08900, September 9, 2008 (Appendix A, Doc 4: 2008 3rd Quarter Dam Safety Inspection Report):

- The piezometers are registering in their historic ranges. They exhibit a muted relationship to the storage pond elevation, but little relationship to the ash pond elevation.
- Current Recommendations: Upstream and Downstream slopes Localized erosion rills/gullies need to be repaired to mitigate further erosion

• Status of Previous Recommendations: Runoff erosion at crest of upstream slope repaired. **Pending completion - open.**

Southern Company Generation 2008 Inspection Report for Plant Wansley Ash Pond Complex, REA # WN-08900, May 28, 2008 (Appendix A, Doc 5: 2008 1st Quarter Dam Safety Inspection Report):

- The piezometers are registering in their historic ranges. They exhibit a muted relationship to the storage pond elevation, but little relationship to the ash pond elevation.
- Current Recommendations: Upstream and Downstream slopes Localized erosion rills/gullies need to be repaired to mitigate further erosion
- Status of Previous Recommendations: Runoff erosion at crest of upstream slope repaired. **Pending completion open.**

3.2 SUMMARY OF LOCAL, STATE AND FEDERAL ENVIRONMENTAL PERMITS

The ash pond facility is under regulation by the Georgia Department of Natural Resources (GDNR), Environmental Protection Division Safe Dams Program (EPDSDP). The discharges of the ash pond are permitted under the Federal National Pollutant Discharge Elimination Program. (NPDES Permit # GA0026778)

3.3 SUMMARY OF SPILL/RELEASE INCIDENTS (IF ANY)

No spills or releases from the Ash Pond facilities have been noted by Georgia Power for this site.

4.0SUMMARY OF HISTORY OF CONSTRUCTION AND OPERATION

4.1 SUMMARY OF CONSTRUCTION HISTORY

4.1.1 Original Construction

Construction was started on the ash pond separation dike in 1973. The original designer for the ash pond management unit was Southern Services, Inc. Plant Wansley has disposed of coal combustion by-products (ash) in one main storage impoundment since 1976. The Plant Wansley ash pond was commissioned in 1975.

The dam assessor did not meet with, or receive information from, the design engineer of record regarding foundation preparation for the ash pond. However, the dam assessor did receive documentation from Georgia Power regarding impoundment materials for the ash pond. Information in the report from Georgia Power Company in May of 1975 and the Drawings for the ash pond (1973) provide documentation on the impoundment material (see Appendix A (Doc 06: Earth Embankment Report.pdf, Doc 13: Ash Pond Construction Dwg.pdf and Doc 08: Plant Wansley Separation Dam Stability Analysis, 1 of 3.pdf)). These drawings include soil descriptions for the separation dike. The dike was constructed over a core area that was undercut to weathered rock and then fill was compacted onto the inspected rock.

Gypsum dewatering cells from drawings dated in 2007 were recently added on to the existing ash pond (see Appendix A (Doc 12: Gypsum Overall Plan View)).

4.1.2 Significant Changes/Modifications in Design since Original Construction

No significant changes/modifications were noted for the Ash Pond before 2007.

Plans and specifications dated 2007 show new construction of gypsum dewatering cells that were installed on the existing ash pond facility. These dewatering cells were assumed to have been built to local codes and standards. Upon visual observation of the four dikes for this facility, everything appeared to be well maintained. See Appendix A Doc 12: Gypsum Overall Plan View for a drawing of the dewatering cells.

4.1.3 Significant Repairs/Rehabilitation since Original Construction

No significant repairs or rehabilitation had been noted other than typical maintenance items described in the quarterly reports.

4.2 SUMMARY OF OPERATIONAL HISTORY

4.2.1 Original Operational Procedures

The ash pond was designed and operated for reservoir sedimentation and sediment storage of fly ash, bottom ash and boiler slag. Plant process waste water, coal combustion waste, coal pile stormwater runoff, and minimal stormwater runoff around the Ash Pond facility are pumped into the reservoir. Inflow water is treated through gravity settling and deposition. The ash sluice water is re-circulated through the system by a combination of a gravity fed system and pumps.

4.2.2 Significant Changes in Operational Procedures since Original Startup

No significant operating procedures for the ash pond have changed since the original start-up.

4.2.3 Current Operational Procedures

The ash pond was designed and operated for reservoir sedimentation and sediment storage of fly ash, bottom ash and boiler slag. Plant process waste water, coal combustion waste, coal pile stormwater runoff, and minimal stormwater runoff around the Ash Pond facility are pumped into the reservoir. Inflow water is treated through gravity settling and deposition. The ash sluice water is re-circulated through the system by a combination of a gravity fed system and pumps.

4.2.4 Other Notable Events since Original Startup

No additional information was provided.

5.0FIELD OBSERVATIONS

5.1 PROJECT OVERVIEW AND ASSESSMENT

Dewberry personnel Frederic Shmurak, PE and Justin Story, EI performed a site visit on Wednesday, June 30, 2010. The site visit began at 8:00 AM. Weather was hot and cloudy. The overall visual assessment of the Ash Pond was that it is in satisfactory condition and no significant findings were noted. Coal Combustion Dam Inspection Checklists created on June 30, 2030, by the two engineers for the Plant Wansley Ash Pond are provided in Appendix B, Doc 1: 2010.06.28 – Ash Pond Checklist. Photographs from the site visit are provided in Appendix B, Doc 4: Photographs.

5.2 EARTH EMBANKMENT DAM

5.2.1 Crest

The crest was covered by graded aggregate base material and had no signs of any rutting, depressions, tension cracks or other indications of settlement or shear failure, and appeared to be in satisfactory condition.

5.2.2 Upstream Slope

The upstream slope of the separation dike is mostly lined with rip rap and stone. Scarps, sloughs, depressions, bulging or other indications of slope instability or signs of erosion were not observed.



Figure 5.2.2a: Crest and Upstream Slope of ash pond's separation dike.

The upstream slope of the western dike is mostly lined with rip rap and stone. Scarps, sloughs, depressions, bulging or other indications of slope instability or signs of erosion were not observed.



Figure 5.2.2b: Upstream side of western dike

5.2.3 Downstream Slope and Toe

The downstream slope is mostly lined with rip rap and stone. Scarps, sloughs, depressions, bulging or other indications of slope instability or signs of erosion were not observed. Gravel had been placed at a few locations along the embankment as some regular maintenance. The toe of this slope is below the normal pool of the cooling water pond; therefore, visual assessment of seepage could not be performed.



Figure 5.2.3a: Crest and Downstream Slope of ash pond's separation dike.



Figure 5.2.3b: Gravel placed along downstream embankment which appeared to be routine maintenance



Figure 5.2.3c: Crest and Downstream Slope of ash pond's western dike.

5.2.4 Abutments and Groin Areas

The embankment consists of a raised dike system; therefore the earthen embankment does not abut existing hillsides, rock outcrops or other raised topographic features.

5.3 OUTLET STRUCTURES

5.3.1 Overflow Structure

The outlet structure was not in use; however, it visually appeared to be in good condition. Due to circulation and re-use of sluice water, the plant regulates the ash pond water surface elevation below the invert of the outlet works and has no record of the skimmer weir or the spillway ever being used.

5.3.2 Outlet Conduit

The spillway system was not in use at the time of the assessment; however, the visible portion of the outlet conduit had no apparent deterioration.

5.3.3 Emergency Spillway (If Present)

The emergency overflow spillway visually appeared to be in good condition.

5.3.4 Low Level Outlet

No low level outlet is present.

6.0HYDROLOGIC/HYDRAULIC SAFETY

6.1 SUPPORTING TECHNICAL DOCUMENTATION

6.1.1 Floods of Record

No flood of record analysis was provided; however, design flow analyses described below indicate there would be no overtopping during floods. This is supported by the fact that the dikes were not reported to have been overtopped during past hurricanes, tropical storms and depressions.

6.1.2 Inflow Design Flood

According to FEMA Federal Guidelines for Dam Safety, current practice in the design of dams is to use the Inflow Design Flood (IDF) that is deemed appropriate for the hazard potential of the dam and reservoir, and to design spillways and outlet works that are capable of safely accommodating the flood flow without risking the loss of the dam or endangering areas downstream from the dam to flows greater than the inflow. The recommended IDF or spillway design flood for a significant hazard intermediate sized structure (See section 2.2), in accordance with the USACE Recommended Guidelines for Safety Inspection of Dams ER 1110-2-106 criteria is the 100-yr to ½ PMF (See Table 6.1.2).

Table 6.1.2: USACE Hydrologic Evaluation Guidelines Recommended Spillway Design floods							
Hazard	Hazard Size Spillway Design Flood						
	Small	50 to 100-yr frequency					
Low	Intermediate	100-yr to ½ PMF					
	Large	½ PMF to PMF					
	Small	100-yr to ½ PMF					
Significant	Intermediate	½ PMF to PMF					
	Large	PMF					
	Small	½ PMF to PMF					
High	Intermediate	PMF					
	Large	PMF					

The Probable Maximum Precipitation (PMP) is defined by American Meteorological Society as the theoretically greatest depth of precipitation for a given duration that is physically possible over a particular drainage area at a certain time of year. The National Weather Service (NWS) further states that in consideration of our limited knowledge of the complicated processes and interrelationships in storms, PMP values are identified as estimates. The NWS has published application procedures that can be used with PMP estimates to develop spatial and temporal characteristics of a Probable Maximum Storm (PMS). A PMS thus developed can be used with a precipitation-runoff simulation model to calculate a probable maximum flood (PMF) hydrograph.

In a stormwater capacity report provided by Southern Company (See Appendix A, Doc 19: Stormwater Capacity.pdf) the authors determined the Wansley Ash Pond can handle the following:

"A 24 hour rainfall runoff of: 1) 16.9 inches of rainfall runoff, which is 2.13 times the 100 year storm, at the level of the emergency spillway crest, and 2) 23.5 inches of rain which is 2.96 times the 100 year storm, at a level one foot below the crest of the dike."

The Ash Pond is designed to safely pass the design storm corresponding to the ½ PMP and is therefore in compliance with recommended federal guidelines. The 6-hour, 10 square mile PMP is 30.5 inches. Adequate freeboard exists to store the ½ PMP event.

6.1.3 Spillway Rating

No spillway rating was provided. The outlet structure type is unregulated and given little change in the normal pool elevation the resulting discharge rate is expected to be relatively constant.

6.1.4 Downstream Flood Analysis

No downstream flood analysis was provided.

6.2 ADEQUACY OF SUPPORTING TECHNICAL DOCUMENTATION

Supporting technical documentation provided is sufficient.

6.3 ASSESSMENT OF HYDROLOGIC/HYDRAULIC SAFETY

Adequate capacity and freeboard exists to safely pass the design storm.

7.0STRUCTURAL STABILITY

7.1 SUPPORTING TECHNICAL DOCUMENTATION

7.1.1 Stability Analyses and Load Cases Analyzed

A stability analysis report for the Fly Ash Pond, prepared in 1973, by Southern Services, Inc., with Geotechnical Testing performed by Law Engineering Testing Company, provides information on the stability analysis results and is presented in Section 7.1.4 Factors of Safety and Base Stresses. Drawings provided by Georgia Power dated 1973 also contains the critical data for the slop stability analysis. Both steady state (normal) loading and drawdown loading conditions were analyzed. See Appendix A (Doc 08: Separation Dike Analysis (1 of 3).pdf) for the drawing.

7.1.2 Design Properties and Parameters of Materials

Construction drawings for Plant Wansley's separation dike were prepared by the Georgia Power in conjunction with Southern Services, Inc. from 1973. The drawings include documentation of the shear strength design properties for the Fly Ash Pond, which is included in this report and is presented in the following section; see Appendix A (Doc 08: Separation Dike Analysis (1 of 3).pdf) for the drawing.

TOTAL STRESS SOIL CHARACTERISTICS						
TYFE		STRUCT	IN	STEACY S	SEEPAGE &	DRAWDOWN
SUIL	7m LBS/CU.FT	DEGR EE S	LE 5/5Q.FT.	<u> </u>	Ø DEGR EE S	LB5/50.FT.
EMB. FILL (A)	124	26.5	1200	124	19	1400
FILTER (B)	130	40	0	130	40	0
RIP-RAP	130	38	၁	130	38	D
BEDROCK (D)	150	40	3000	150	40	30.00
F51.1 (E)	112	8	550	112	12	700
						<u> </u>

Design Shear Strength

The following items were noted on the Separation Dike Stability Analysis drawings:

- Safety factors shown are the minimum for each condition. Complete computer results available from Southern Services, Inc.
- Safety factors do not include benefit from deposit of ash.
- Soil characteristics from Law Engineering and Testing Co. through Georgia Power.
- Materials recommended by LETCO for embankment fills are: a. Fine to medium sandy silt (west borrow); b. stiff to hard fine to medium sandy micaeous silt (north borrow); partially weathered rock (LETCO report No. 40 1972).
- Embankment fills shall be compacted to at least secure the design strength characteristics used in the analysis of slopes. Field control should ensure the design strength of the materials used in the design.

The above referenced document is provided in Appendix A (Doc 08: Separation Dike Analysis (1 of 3).pdf).

A more recent study was performed in the 2010 and the results are found below. The soil weight and strength parameters used in the study are consistent with soils found in the Piedmont geological province. For the complete report see Appendix A, Doc 17: Slope Stability Analysis.pdf. There is no evidence that the dikes were built of or upon wet ash, slab, or other unsuitable materials.

	Dry Unit Weight (pcf)	(pci)	Effective Stress Parameters		Total Stress Parameters	
			Internal Friction Angle	Cohesion (psf)	Internal Friction Angle	Cohesion (psf)
Embankment Fill	102	123	32	140	29	400
Foundation Soil		112	37	0	24	80
Foundation (Gravel Filter)		130	40	0	40	0
Sluiced Ash		80	10	0	10	0
Rock		150	40	3000	40	3000

7.1.3 Uplift and/or Phreatic Surface Assumptions

The 1973 Separation Dike Stability Analysis drawings, prepared by Georgia Power and Southern Services, Inc., provides information on the phreatic surface as shown in Figure 7.1.3A and the drawings can be found in Appendix A (Doc 08: Separation Dike Analysis (1of3).pdf). A 2010 slope stability analysis was provided that shows the phreatic surface profiles (See Figure 7.1.3B through D). For the complete

report and drawings showing the phreatic surfaces see Appendix A, Doc 17: Slope Stability Analysis.pdf. Piezometric readings indicate that the phreatic surface has overall been stable and is consistent with the assumptions made in the slope stability models.

"The most recent levels for each pond along with the piezometer readings are summarized in the table below.

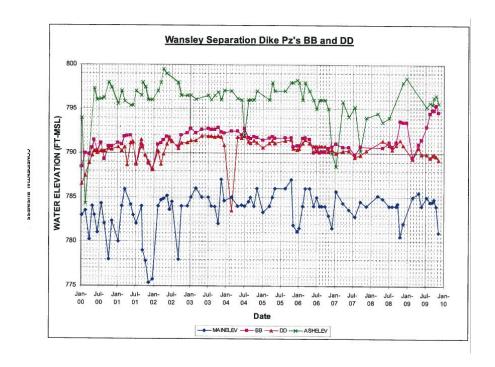


Figure 7.1.3a: Historic Piezometer Readings

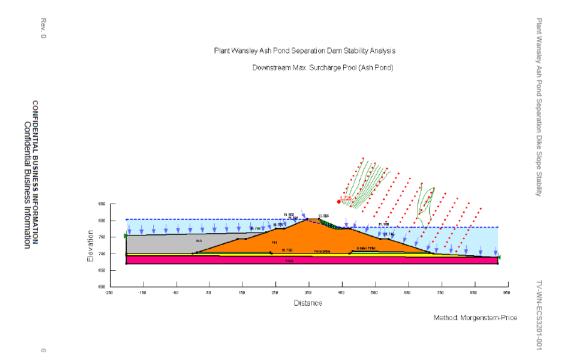


Figure 7.1.3b: Phreatic Surface Profile (Steady State)

The locations of the piezometers are depicted in Figure 9.2.1, within the Instrumentation Plan Section. (See Appendix A Doc 1 through 5 for inspection reports showing historical piezometer data). Piezometer BB was damaged in 2008 and Georgia Power stated it was recently repaired.

The piezometer reading information generally indicates a steady and consistent trend. There appears to be a major drop in piezometer DD reading in 2004 which is unexplained.

The increased elevated readings of piezometer BB starting in 2008 were potentially caused from measurements being made from different reference points when the top of the pipe was broken off.

Internal drainage collection and discharge piping was not located by the dam assessors during the visual site inspection. However, Georgia Power provided documentation on internal drainage collection (drainage blankets) and discharge piping. See Appendix C (Doc 07: Ash Pond Profile.pdf) for the drawing.

7.1.4 Factors of Safety and Base Stresses

A stability analysis drawing for the separation dike prepared in 1973, by Georgia Power, with Southern Services, Inc. provides information on the factors of safety and is presented below. See Appendix A (Doc 08: Separation Dike Analysis (10f3).pdf) for the complete report.

The results of the stability analysis are summarized in the drawings below. The stability analyses were performed on the downstream slope of the separation dike, however, no information was provided for the western dike.

The safety factors presented in the drawings (Steady State = 1.56; Drawdown = 1.27) show that the slopes of the fly ash facility at Plant Wansley have satisfactory safety factors under static and drawdown conditions.

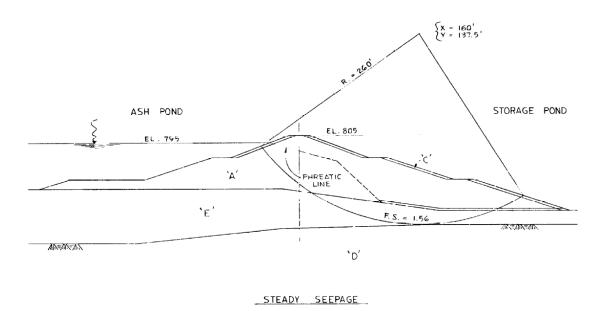


Figure 7.1.4a: Steady Seepage Profile

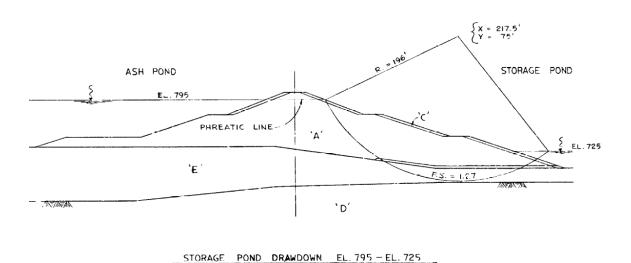


Figure 7.1.4b: Drawdown Profile

A slope stability analysis was performed in 2010 and the results are below. For the complete report please see Appendix A, Doc 17: Slope Stability Analysis.pdf.

Table 7.1.4: Factors of Safety

Failure Conditions	Computed Factor of Safety	Required Minimum Factor of Safety ¹
Downstream Steady State	1.9	1.5
Downstream Seismic	1.2	1.1
Downstream Maximum Surcharge Pool (Ash Pond)	1.7	1.4
Upstream Rapid Drawdown (Ash Pond)	1.9	1.3
Downstream Rapid Drawdown (Storage Pond)	1.4	1.3

¹ US Corps of Engineers Manual EM 1110-2-1902, October 2003

Based on the results of the analyses presented in this report, all the dams and dikes that form the fly ash disposal facility at Plant Wansley were found to have stability safety factors at or above the minimum recommended values. Slope stability analysis were not provided for the western dike; however, foundation preparation, embankment soils and geometry are consistent. Performance of a separate slope stability analysis is not deemed critical.

On this basis, it is believed that the facility is performing as intended in its design. Routine maintenance and inspections should continue to enable the facility to perform as found in this evaluation.

7.1.5 Liquefaction Potential

A 2010 study was provided that showed the soils have a factor of safety of 1.4 against liquefaction. It was stated that from liquefaction potential analysis, that "the separation dike soils are not subject to appreciable strength loss due to earthquake shaking." (See Appendix A, Doc 18: Liquefaction Potential.pdf).

A separate soil report stated the fill material came from native borrow sites and the typical soil in the surrounding area does not have liquefaction problems.

Original foundation soil conditions do not appear susceptible to support liquefaction.

7.1.6 Critical Geological Conditions and Seismicity

No critical geological conditions were noted.

An engineering report for Plant Wansley titled "Earth Embankments Final Report and Appendices" references the geological conditions of the site as follows:

"The site geology and soil profile are typical of much of the Southeastern Piedmont. The plant area and embankments are within the Brevard Zone which is a pronounced geological lineament of deformed rocks extending from Alabama northeastward into North Carolina. The rocks at the site are biotite gneiss and schists, typically striking northeast with a southeast dip. Where sound and unweathered, they are highly competent, but because of the pronounced foliation, they break into flat particles when excavated and crushed. However, this characteristic is not sufficiently detrimental to prevent the rock from being entirely adequate for riprap, bedding material, sub-ballast, and road base. It was not used for concrete aggregate.

Upland soils throughout the site are residual from in-place weathering of underlying parent rock. These soils were used in the construction of dikes and dams, and are generally sandy, micaceous, silts and silty, micaceous, fine sand. The upper layer, however, was more clayey because of advanced weathering of the minerals, and is generally described as red-brown, sandy, silty, clay, sandy, clayey silts.

In the valleys of streams and smaller drainages features, alluvial soils cover the valley floors. These are primarily silts and fine sands, with pockets of soft, organic much soils which required removal before placing embankment. They were not used for embankment fills."

See Appendix A (Doc 06: Earth Embankment Report.pdf) for the complete document.

Based on USGS Seismic-Hazard Maps for the Conterminous United States, dated 2005, the facility is located in an area anticipated to experience a 0.10g acceleration with a 2-percent probability of exceedance in 50-years.

7.2 ADEQUACY OF SUPPORTING TECHNICAL DOCUMENTATION

Structural stability documentation is adequate.

7.3 ASSESSMENT OF STRUCTURAL STABILITY

The structural stability of the ash pond appears to be satisfactory.

Based on the previous assessment reports/inspections provided by Georgia Safe Dams Program and Georgia Power, this assessment of the fly ash pond is generally consistent with historical observations.

8.0MAINTENANCE AND METHODS OF OPERATION

8.1 OPERATIONAL PROCEDURES

Operational procedures are adequate. The facility is operated for reservoir sedimentation and sediment storage; specifically, fly ash, bottom ash, pyrites, boiler slag and flue emission control residuals. Coal combustion process waste water and stormwater runoff from the facility are discharged into the reservoir, inflow water is treated through gravity settling and deposition. The sluice water is re-circulated through the plant.

8.2 MAINTENANCE OF THE DAM AND PROJECT FACILITIES

Maintenance procedures are adequate. Grassed areas are routinely mowed and vegetation is removed from the rip-rap slopes. Spillways and outlets are maintained and debris is removed as needed. Deficiencies as noted in the surveillance & monitoring program are corrected and documented.

8.3 ASSESSMENT OF MAINTENANCE AND METHODS OF OPERATION

8.3.1 Adequacy of Operational Procedures

Operational procedures are adequate.

8.3.2 Adequacy of Maintenance

The maintenance program is adequate.

9.0SURVEILLANCE AND MONITORING PROGRAM

9.1 SURVEILLANCE PROCEDURES

Georgia Power stated they have daily, weekly, monthly, quarterly and annual inspections for the ash pond. Only documentation was provided on the quarterly inspections.

Quarterly Inspections:

A quarterly inspection is conducted by plant personnel and at least one representative of Hydro Services. See Appendix A (Doc 01 through Doc 05) for copies of the 2008 & 2009 quarterly inspection reports.

9.2 INSTRUMENTATION MONITORING

9.2.1 Instrumentation Plan

The following data is based on inspection reports provided by American Electric Power:

An instrumentation plan was not provided, however piezometers have been installed to collect instrumental data. The peizometers are located around the separation dike. For piezometer readings, a water level indicator probe is used, which is lowered within the monitoring well until water is reached, and the distance is recorded. Profiles of the monitoring wells and piezometers are depicted in Figure 9.2.1. Please refer to Appendix A (Doc 11: Separation Dike Profiles.pdf) for piezometer profile drawings.

9.2.2Instrumentation Monitoring Results

Instrumentation monitoring data has been provided and is discussed in Section 7.1.3 Uplift and/or Phreatic Surface Assumptions.

9.2.3Evaluation

The historical data indicates that the embankment dams are performing adequately.

9.3 ASSESSMENT OF SURVEILLANCE AND MONITORING PROGRAM

9.3.1 Adequacy of Inspection Program

Inspection program is adequate.

9.3.2Adequacy of Instrumentation Monitoring Program

The surveillance and monitoring programs should include additional monitoring of the emergency overflow spillway. As indicated previously, the overflow spillway is not in use; however, minor cracking is occurring and the potential for water to undermine the spillway slab exists.

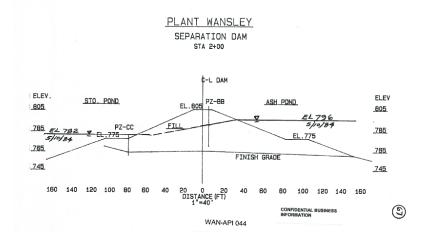


Figure 9.2.1a: Separation Dam Cross Section

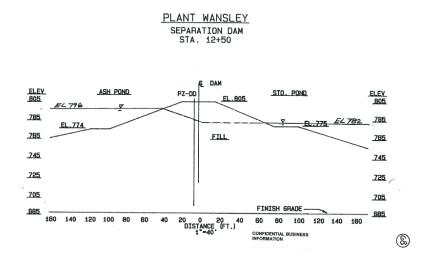


Figure 9.2.1b: Separation Dam Cross Section

Southern Company Generation Bin 10193 241 Ralph McGill Boulevard NE Atlanta, Georgia 30308-3374

Tel 404.506.7033

December 9, 2009



PLANT WANSLEY

Dam Safety Surveillance Quarterly Report REA No. WN-08900

Mr. J. P. Heilbron Plant Manager Georgia Power Co. Plant Wansley

Dear Mr. Heilbron:

Attached is the 4th Quarter - 2009 report on Dam Safety Surveillance for Plant Wansley. The inspection of the dikes at the Main Storage Pond, Ash Pond, Separator Dike, Potable Water Pond, Detention Pond and Gypsum Storage Pond was performed on October 16, 2009 by Hugh Armitage of the SCG Hydro Services Group. A representative from Plant Wansley accompanied Mr. Armitage on the inspections.

This report includes:

- a) A review of the current instrumentation data;
- b) The 4th Quarter 2009 Dam Safety Inspection Report summarizing the current and previous recommendations, field observations, comments. and photographs made during the October 16, 2009, inspections;
- c) A copy of the current instrumentation plots;
- d) A copy of the finalized 1st Qtr report. This report was issued in Draft to plant personnel in April 2009.

The current recommendations from this 4th Quarter Inspection are described on the first page of the attached report. The description and status of recommendations from the previous quarterly inspection are also described on pages 1 and 2 of the attached report.

Should you have any questions, please contact Hugh Armitage at extension 8-506-7109.

Sincerely,

Larry B. Wills

Principal Engineer, Hydro Services

& Will

WAN-API 058

/hha

Attachments

xc: Georgia Power Company

M. A. Leason	(w/ attachment)
N. I. Dean	(w/ attachment)
T. S. Lovvorn	(w/ attachment)
G. C. Moncus	(w/ attachment)
B. Harcrow	(w/ attachment)

Southern Company Services

D. E. Jones	(w/attachment)
E. B. Allison	(w/ attachment)
J. H. Crisler	(w/ attachment)
B. J. Peterson	(w/attachments)
K. Furman	(w/ attachments)

Hydro Service Wansley Notebook

Master File: WN-09900

Hydro Services Correspondence Notebook (w/attachments)

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Instrumentation Data Review 4th Ouarter – 2009

In the 3rd quarter of 2009, it was agreed that the level of the storage pond would be gradually reduced by usage to elevation 780 ft. in an effort to reduce the extent of the wet area on the lower downstream slope at Sta. 37+50. This issue has been discussed since the early 1980's. The pond level taken during the November instrumentation readings was reported to be elevation 781.0 feet.

A current assessment of instrumentation data reviewed up to the most recent readings at Plant Wansley is as follows.

Storage Pond:

Sta. 20+00: Piezometers are generally within their historic range and seem to be tracking the pond level.

Sta 37+50-1: Piezometers PP and TT3 experienced a significant drop in elevation in the spring. It is not known exactly why this occurred but may have been related to a delayed response to a decrease in the pond level late in 2008. The readings since July have returned to their historic levels.

Sta 37+50-2: All of these piezometers appear to be in their historic range. They generally are tracking the pond level.

Sta 37+50 Pipe Flows: These flows appear to be in their historic ranges. These flow rates will continue to be monitored to assess if any trends develop. They seem to be tracking the pond level. The flow rate is decreasing at most of the drains as the pond level is being lowered to elevation 780 ft.

Sta 47+50: The piezometers are within their historic range of measurement.

It is recommended that the flow rate at the toe drain at Station 39+00 be obtained at monthly intervals rather than the current 6 month schedule. This information will be useful when modifications to the drains at 37+50 are carried out.

Sta 58+00: The piezometer levels are Sta. 58+00 are within their historic range. The piezometer LLL was repaired this past summer when the drain valve was observed leaking earlier in the year.

<u>Relief Wells:</u> The relief wells appear to be discharging in their historic range, but also in response to the lower pond level. Dip in flow seems to have occurred when the seep was discovered in July.

Sta 65+00: Piezometric levels at C, E and MM are within their historic ranges.

Sta 70+00: All of these piezometers are reading in their historic ranges.

Weir and Pipe Flows: Weir measurements at Sta. 11+20 and 49+00 indicate a continuing trend to their historic range.

Separation Dike:

These piezometers are generally registering in their historic ranges. Although Piezometer BB continues to exhibit has recently exhibited an elevated water level and generally has followed the change in level of the ash pond. The piezometer was repaired after being damaged in 2008.

Potable Water Pond Dike:

These piezometers were registering in their historic ranges and appear to respond to the water level in the main pond level.

2009 - 4th Quarter Dam Safety Inspection Summary

Date of Inspection:

Rainfail (past 24 hrs):

October 16, 2009

Inspection by: H. Armitage - SCG Hydro Services

Daryl Clayton - Plant Security Officer

Weather: Temperature: Overcast, windy

~56° F

SUMMARY

1. No major dam safety issues that would impact the safety of the structures were observed during this inspection. Recommendations to address current and previous inspection observations are summarized below. Many of the current recommendations are routine, on-going maintenance type activities.

ADDITIONAL COMMENTS

- 1. Plant personnel did a very good job in completing many of the recommendations from previous quarterly inspections.
- 2. A copy of the Plant Wansley instrumentation data and review comments are attached.

CURRENT RECOMMENDATIONS			
No.	Description	Location	Status - Open/Closed
1	Sign at Station 0-00 needs to be repaired & reinstalled.	Storage Pond - East Dike	Open
2	Weeds and grass need to be cut at toe of slope.	Storage Pond - East Dike	Open
3	Localized bare spots on slopes need to be re-grassed to prevent further erosion and silt washing into ditches. A manufactured grass matting product would be preferable to minimize erosion/wash-out during initial grass growth.	Storage Pond - Various locations along East and Southeast dikes (downstream slopes)	Open
4	Numerous rodent/animal burrow holes observed on downstream slopes. All require filling in. (Excerpt from FEMA Publication 473 - "Impact of Animals on Earthen Dams". Copy prev. forwarded to plant personnel, provides options for repair).	Storage Pond - Various locations along East & Southeast dikes (downstream slopes)	Open
5	Numerous ant mounds observed on downstream slopes. Treatment required for ant mounds by fumigants or chemical methods as described in FEMA Publication 473 - "Impact of Animals on Earthen Dams". Copy prev. forwarded to plant personnel, provides options for repair).	Storage Pond - Various locations along East & Southeast dike (downstream slopes)	Open
6	Clean out debris and dirt in concrete lined toe ditches	Storage Pond - East and Southeast Dikes - Various locations	Open
7	Localized holes in concrete need to be repaired (i.e. pressure grout) to prevent undermining of concrete. Water flowing out of holes.	Storage Pond - East Dike, Ash Pond Overflow Channel	Open
8	Mud and debris needs to be removed from behind weir.	Storage Pond - Sta 37+50 - Southeast Dike	Open
9	Truckload of #57 stone required at Emergency Aggregate stockpiles	Storage Pond - Southeast Dike	Open
10	Portion of rip rap spillway washed away should be repaired by replacing with a concrete slab or alternatively use Type 1 rip rap placed on a 16ounce/sq. yard non-woven geotextile. Some excavation will be required with rip-rap option so elevation of existing concrete and rip-rap match existing rip rap.	Ash Pond - Emergency Spillway	Open
11	Repair downstream end of channel using Type #1 rip-rap placed over a 16 oz/sy non-woven geotextile. Area of scour beside the channel should be repaired using type 1 rip rap placed on a 16 oz./sy non-woven geotextile. Alternatively, a concrete slab could be extended from the edge of the concrete channel to prevent future scour.	Ash Pond - Outflow Channel	Open
12	Eroded area of upstream (west side) to be repaired. Recommended repair procedure provided by SCG Hydro Services to Plant personnel. All vegetation and large rocks/boulders need to be removed from upstream approach channel to prevent future blockage of spillway during high flow conditions.	Potable Water Pond	Ореп
13	Fill in several potholes on road at crest of East Dike.	Storage Pond	Open
14	The measurement of flow from the toe drain at Sta. 39+00 should be done monthly and added to monthly instrumentation report that is sent to Hydro Services.	Storage Pond	Open
15	The measurement of flow from the new drain installed in July 2009 near the service water pump house should be done monthly and added to the monthly instrumentation report that is sent to Hydro Services.	Storage Pond	Open

	Plant Wansley		
	2009 - 4th Quarter Dam Safety Inspection Sumi	mary	
011	STATUS OF PREVIOUS RECOMMENDATIONS		
No.	Location, Description & Action Required	Status - Open/Closed	
1	Storage/Potable Water and Detention Pond -Grassed areas on slope should be fertilized to promote healthy & more robust grass growth.	Ongoing	
2	Storage Pond - Localized rutting on east dike from grass cutting equipment. Ruts to be filled in with soil and compacted and re-grassed	Closed - Repaired	
3	Storage Pond - Localized bare spots on slopes need to be re-grassed to prevent further erosion and silt washing into ditches	Closed - Repaired - However new areas observed. See Current Recommendation 3	
4	Storage Pond - Various locations along east dike (downstream) - Numerous rodent/animal burrow holes observed on downstream slopes. All require filling in. (Excerpt from FEMA Publication 473 - "Impact of Animals on Earthen Dams". Copy forwarded to plant personnel, provides options for repair).	Closed-Repaired (New ones since 1st Qtr- See Current Recommendation 4)	
5	Storage Pond - Numerous ant mounds observed on downstream slopes. Treatment required for ant mounds by fumigants or chemical methods as described in FEMA Publication 473 - "Impact of Animals on Earthen Dams". Copy forwarded to plant personnel, provides options for repair)	Closed- Repaired (New ones since 1st Qtr See Current Recommendation 5)	
6	Storage Pond - A truckload of (a) GDOT #10 washed sand and (b) washed #89 stone is required to replace depleted stockpiles.	6(a) - Closed 6(b) - Open	
7	Storage Pond - East Dike - Various locations - Clean out debris and dirt in concrete lined toe ditches	Closed- New debris observed in Ditches (See Current Recommendation 6)	
8	Storage Pond - East Dike -Lower Slope - Storage Pond - Sta 37+00 - Repair undermining at end of concrete lined ditch	Closed - Repaired (See Photo 10)	
9	Storage Pond- East Dike - Sta 45+00 & 49+00, 54+00 and Sta 57+00. Clean out ditch behind weirs and inside of the ends of drainage pipes.	Closed - Completed	
10	Ash Pond Emergency Overflow -Several holes observed in bottom of concrete lined ditch and water spouting out. Repair required to mitigate undermining of slab.	Closed - Replaced by Current Recommendation 7	
11	Ash Pond Emergency Overflow - End of concrete Ilined ditch is undermined. Needs to be repaired per recommendation in 5-28-08 inspection report (page 11 of 20).	Closed - Repaired (Undermined again during recent heavy rains - See Current Recommendation 11)	
12	Storage Pond -Cracks in concrete lined ditches should be cleaned out and caulked - Pending completion	Open	
13	Storage Pond - Downstream Slopes - Sta 19+00 & 22+00 and 37+50D - Drain pipes need to be cleaned out and repaired Pending Completion	Open	
14	Storage Pond - Downstream Slope - Sta 37+50 - Hydro Services investigated wet area 1/29/09 and further options to address will be investigated. Plant personnel needs to monitor this area DAILY for any evidence of distress or unusual events, or movement of slope and contact Hydro Services immediately particularly when pool elev. 782-785 ft. Pending SCG Hydro Services continued monitoring and review. Hydro Services contacted Georgia Safe Dams Program for an extension of time to review possible repair options.	Open - Pending Further Review and Monitoring. Pond level being reduced to elev. 780 ft. to see if this reduces the extent o wet area. Area being monitored daily.	
15	Storage Pond - Downstream Slope - approx. Sta 56+00 - Damaged marker pole for toe drain needs to be repaired.	Open	
16	Storage Pond - Downstream Slope - approx. Sta 62+00 - Damaged concrete ditch needs to be fixed and accumulated silt removed.	Closed - Repaired	
17	Storage Pond - Downstream Slope - Plezometer LLL - Plezometer leak at spigot connection needs to be repaired. Ground surface is wet around piezometer	Closed - Repaired	

	OBSERVATIONS FO	OR 4th QUARTER INSPECTION	
- Storage Pond - I	North Dike - (Road to Recreational Area)		Storage Pond Elev. 782' (10-18-09)
	Observations -	Comments	Photograph No.
1. Upstream Slope			
a. Condition	Grass covered - Overall condition is good. No enear crest that need to be treated with approved Recommendation 5).		n/e
o. Erosion/Sloughing	Yes (X) No ()- Minor localized bare spots. Need to be re-grassed. (See Current Recommendation 3).		n/a
2. Crest		Topic and the second	
a. Condition	Gravel surfaced - No distress or potholes in road surface observed.		n/a
3. Downstream Slope			
a. Condition	Good - Grass covered and length that permits good	od observation.	n/a
o. Seepage/Wet Spots	Yes () No (X)		n/a
c. Erosion/Sloughing	Yes () No (X)	FIDENTIAL BUSINESS DRMATION	n/a

Plant Wansley 2009 - 4th Quarter Dam Safety Inspection Summary II - Storage Pond - East Dike (from North Dike to Spillway) Observations - Comments Photograph No. 1. Upstream Slope a. Condition Rip-rap on upstream face looks satisfactory and no dam safety issues observed. n/a b. Erosion/Sloughing Yes () No (X) n/a 2. Crest Good - Gravel surfaced. Several depressions on road need to be filled potholes in road a. Condition 1 surface observed. (See Current Recommendation 13) 3. Downstream Slope Good - Grass covered and at a length that permitted a good visual inspection. See other a. Condition comments below. 1. Station marker at 0-00 needs to be replaced. (See Current Recommendation 1). 2 2. Weeds and grass near toe of slope needs to be cut down (See Current 3 Recommendation 2). 3. Numerous bare spots on slopes and areas adjacent to concrete lined ditch need to be regrassed (i.e. grass matting product) to prevent further erosion and silt from washing into 4 ditch (i.e. see Photo 4 @ ~Sta 6+10) - (See Current Recommendation 3). 4. Rodent holes observed at Sta. 7+00, 17+40. Need to be treated. (See Current n/a Recommendation 4) 5. Localized ant mounds observed on slopes. Need to be treated. (See Current n/a Recommendations 5) b.Seepage/Wet Spots Yes () No (X) - No seepage or wet spots observed on slope. n/a c. Erosion/Sloughing Yes () No (X) - No evidence of instability. n/a d. Concrete-Lined Concrete in good condition. See other comments below. 5.6 & 7 Drainage Ditch 1. Debris in drain pipe at Sta. 19+00 need to be cleaned out and new sections of pipe required at drains at Sta 19+00 and 22+00 to repair damaged/crushed outlets ends. (See n/a Previous Recommendation 13). 2. Ditch needs to be cleaned out of debris between Sta 2+00 and Sta 19+00. (See Current 5 & 6 Recommendation 6). 3. Hole in concrete ditch needs to be repaired (i.e. pressure grouted). (See Current 7 Recommendation 7). Yes (X) No () - Needs tandem truckload of GDOT #89 stone. (See Previous e. Emergency n/a Aggregate Stockpiles Recommendation 6 (b). III - Storage Pond - Spillway **Observations - Comments** Photograph No. 1. Spillway Abutment/Deck a. Condition Concrete condition is satisfactory. n/a 2. Splliway Floor Concrete satisfactory. Localized areas where caulking required to fill open joints. (See a. Condition n/a Previous Recommendation 12). 3. Spillway Walls a. Condition Concrete satisfactory n/a 4. Splilway Gates a. Condition Looked satisfactory. Gate seals appear okay. n/a 5. Downstream of Spillway (Channel) a. Condition Satisfactory. n/a IV - Storage Pond - Southeast Dike (Splliway to Separator Dike) **Observations - Comments** Photograph No. 1. Upstream Slope a. Condition Rip-rap is satisfactory. No instability or beaching evident. n/a b. Erosion/Sloughing Yes () No (X) n/a 2. Crest a. Condition Satisfactory - Gravel surfaced/Railway tracks - No distress observed along road surface . n/a 3. Downstream Slope 3a - Upper Slope Satisfactory - Grass covered. Rodent hole observed above concrete ditch. Near Sta. a. Condition 8 41+00. (See Current Recommendation 4). b.Seepage/Wet Spots Yes () No (X) n/a c. Erosion/Sloughing Yes () No (X) - No evidence of instability.

	Plant Wansley	
	2009 - 4th Quarter Dam Safety Inspection Sumi	mary
3b - Mid-Slope Road &		Hally Charles Balletine
a. Road Condition	Gravel surfaced - looks good. No distress or potholes in road surface observed.	n/a
b. Concrete-Lined	Concrete in good condition. Cleanout debris in concrete lined ditches. (See Current	
Drainage Ditch	Recommendation 6).	n/a
3c - Middle Slope	Satisfactory. Grass covered. Recent rodent holes observed, near Sta 40+00 at edge of	
a. Condition	concrete ditch. (See Current Recommendation 4).	n/a
b.Seepage/Wet Spots	Yes () No (X)	n/a
c. Erosion/Sloughing	Yes () No (X)	n/a
3d. Lower Road & Dra	inage Ditch	
a. Road Condition	Gravel surfaced - looks good. No distress or potholes in road surface observed.	n/a
b. Concrete-Lined Drainage Ditch	Concrete condition is acceptable	n/a
3e - Lower Slope		
a. Condition	Satisfactory - Grass covered - Grass at a length that permitted a good visual examination. See other comments below.	n/a
	1. Several bare areas require repair. (See Current Recommendation 3)	n/a
	2. Treat ant mounds and rodent holes at various locations. (See Current Recommendations 4 and 5).	n/a
b.Seepage/Wet Spots	Yes (X) No () - See other comments below.	n/a
	1. Ground surface in the area of Sta 37+50 is wet. Operating level of pond is at 782 ft. and is gradually being lowered to Elev. 780 ft. in an effort to "dry up" this area. This area will continued to be monitored.	n/a
	2. Sta 37+50 - Mud and debris needs to be cleaned out from behind weir (See Current Recommendation 8).	9
	3. Large area of ground subsidence/loss was observed near the downslope concrete drainage ditch. This ground loss occurred presumably because of water migrating beside/beneath the concrete ditch as a result of water flowing into a hole in the bottom of a road crossing culvert that discharges into the concrete drainage ditch (Photo 10). Plant personnel mobilized contractor during the inspection to repair the area per instructions provided by Hydro Services. Hole was backfilled with #89 stone and then covered with clayey soil and grass seed and hay mulch placed at ground surface. Plant subsequently mobilized Civil Field Servivces to grout the holes in the corroded culvert and also grout in the area of the repair.	11
	4. Flow measurement at the toe drain at Station 39+00 should be made monthly and sent to Hydro Services (See Current Recommendation 14)	n/a
c. Erosion/Sloughing	Yes () No (X) - No evidence of instability. Some recent bare on slope areas require reseeding. (See Current Recommendation 3).	n/a
d. Concrete Drainage Ditch	Concrete condition is good. Ditches need to be cleaned out of mud and other debris. (See Current Recommendation 6).	n/a
e. Concrete Drainage Channel at 37+50	Concrete condition is good. Rip-rap has been replaced satisfactorily at end on concrete channel.	10
f. Emergency Aggregate Stockpiles	Yes (X) No () - Need one truckload of GDOT washed # 57 stone (See Current Recommendation 9)	n/a
3f - Lower Concrete-Li		
a. Condition	Acceptable	n/a
v - Storage Pond/A	sh Pond - Separator Dike Observations- Comments	Ash Pond Elev. 795 ft
1. Upstream Slope (Sto		Photograph No.
a. Condition	Rip-Rap - Looks satisfactory. No evidence of instability.	n/a
o. Erosion/Sloughing	Yes() No(X)	n/a
2. Crest		
a. Condition	Gravel surfaced and in good condition.	n/a
3. Downstream Slope (· # CE
3a. North End		
a. Condition	Rip-Rap - Looks satisfactory. No evidence of instability	n/e
o. Erosion/Sloughing	Yes () No (X)	n/a
	nger applicable due to Gypsum Pond/berm construction)	7.00
1,070		

	Plant Wansley 2009 - 4th Quarter Dam Safety Inspection Sumn	narv
V - Ash Pond Eme	rgency Overflow & Spillway	Ash Pond Elev. 795 ft
	Observations- Comments	
1. Upstream Siope	Observations- Comments	Photograph No.
a. Condition	Die Ben Leeke estisfesten. Green nedien of also account to be a site of	
b. Erosion/Sloughing	Rip-Rap - Looks satisfactory. Grass portion of slope generally looks satisfactory. Yes (X) No () - Minor erosion at toe of grassed portion of slope. Not serious at this time but condition should be monitored to assess whether condition deteriorate which will require	n/a
2. Crest	repair.	
a. Condition	Gravel surfaced and in good condition.	-6
3. Downstream Slope		n/e
a. Condition	Rip-Rap - Looks satisfactory. No evidence of instability	-6
b. Erosion/Sloughing	Yes () No (X)	n/a
4. Concrete Lined Em	<u> </u>	n/a
a. Condition	Concrete in good condition. Part of the rip rap of spillway was washed away during heavy rainstorms in September. This area should be repaired (See Current Recommendation 10)	n/a
5. Concrete Lined Over	erflow Channel	
a. Condition	Concrete in good condition. See other comments below.	15 & 16
	Several localized areas where hole in concrete have water spouting out. Repair option - pressure grouting. (See Current Recommendation 7).	n/a
	Downstream outlet end of concrete lined ditch is undermined following heavy September rains. Needs to be repaired. (photo 15). (See Current Recommendation 11).	15
	Area adjacent to channel (photo 16), was scoured during heavy September rains and overflow of channel. Area needs to be repaired to prevent further erosion/scour. (See Current Recommendation 11).	16
VII- Potable Water	Pond Pond	Potable Water Pond Elev. 801'
	Observations - Comments	Photograph No.
1. Upstream Dike Slop	e (Potable Water)	
a. Condition	Rip-rap on upstream face looks good.	n/a
b. Erosion/Sloughing	Rip-rap on upstream face looks good. Yes (X) No () - Localized, minor surface erosion.	n/a n/a
b. Erosion/Sloughing c. Concrete Drainage	Yes (X) No () - Localized, minor surface erosion.	
b. Erosion/Sloughing c. Concrete Drainage Ditch		n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed.	n/a n/e
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed.	n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed.	n/e n/e n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. lope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory	n/e n/a n/a n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. lope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope.	n/e n/a n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. lope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory	n/e n/a n/a n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing d. Concrete Drainage	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. Iope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope. Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and	n/a n/a n/a n/a n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing d. Concrete Drainage Ditch	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. lope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope. Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and grass starting to grow. Yes () No (X) - Condition of concrete satisfactory.	n/a n/a n/a n/a n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing d. Concrete Drainage Ditch 4. Spillway Approach	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. lope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope. Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and grass starting to grow. Yes () No (X) - Condition of concrete satisfactory.	n/a n/a n/a n/a n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing d. Concrete Drainage Ditch 4. Spillway Approach a. Condition-General	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. Iope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope. Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and grass starting to grow. Yes () No (X) - Condition of concrete satisfactory. Channel Portion of the upstream approach embankment washed out during heavy rains in September. SCG Hydro Services provided plant with written repair options. Remaining weeds and large rocks/boulders in approach channel need to be removed. (See Current	n/a n/a n/a n/a n/a n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing d. Concrete Drainage Ditch 4. Spillway Approach a. Condition-General b. Condition-Rip-Rap	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. Iope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope. Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and grass starting to grow. Yes () No (X) - Condition of concrete satisfactory. Channel Portion of the upstream approach embankment washed out during heavy rains in September. SCG Hydro Services provided plant with written repair options. Remaining weeds and large rocks/boulders in approach channel need to be removed. (See Current Recommendation 12). Good. No evidence of instability.	n/a n/a n/a n/a n/a n/a n/a n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing d. Concrete Drainage Ditch 4. Spillway Approach a. Condition-General b. Condition-Rip-Rap c. Condition-Concrete	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. Iope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope. Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and grass starting to grow. Yes () No (X) - Condition of concrete satisfactory. Channel Portion of the upstream approach embankment washed out during heavy rains in September. SCG Hydro Services provided plant with written repair options. Remaining weeds and large rocks/boulders in approach channel need to be removed. (See Current Recommendation 12). Good. No evidence of instability.	n/a n/a n/a n/a n/a n/a n/a n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing d. Concrete Drainage Ditch 4. Spillway Approach a. Condition-General b. Condition-Rip-Rap c. Condition-Concrete 5. Spillway Structure -	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. Iope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope. Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and grass starting to grow. Yes () No (X) - Condition of concrete satisfactory. Channel Portion of the upstream approach embankment washed out during heavy rains in September. SCG Hydro Services provided plant with written repair options. Remaining weeds and large rocks/boulders in approach channel need to be removed. (See Current Recommendation 12). Good. No evidence of instability. Good. Abutments/Deck	n/e n/a n/e n/e n/e n/e n/e n/e 17 & 18 n/e n/e
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing d. Concrete Drainage Ditch 4. Spillway Approach a. Condition-General b. Condition-Rip-Rap c. Condition-Concrete 5. Spillway Structure a. Condition	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. Iope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope. Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and grass starting to grow. Yes () No (X) - Condition of concrete satisfactory. Channel Portion of the upstream approach embankment washed out during heavy rains in September. SCG Hydro Services provided plant with written repair options. Remaining weeds and large rocks/boulders in approach channel need to be removed. (See Current Recommendation 12). Good. No evidence of instability. Good. Abutments/Deck Concrete in good condition.	n/a n/a n/a n/a n/a n/a n/a n/a
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing d. Concrete Drainage Ditch 4. Spillway Approach a. Condition-General b. Condition-General c. Condition-Concrete 5. Spillway Structure a. Condition 6. Spillway Structure	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. Jope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope. Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and grass starting to grow. Yes () No (X) - Condition of concrete satisfactory. Channel Portion of the upstream approach embankment washed out during heavy rains in September. SCG Hydro Services provided plant with written repair options. Remaining weeds and large rocks/boulders in approach channel need to be removed. (See Current Recommendation 12). Good. No evidence of instability. Good. Abutments/Deck Concrete in good condition.	n/e
b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing d. Concrete Drainage Ditch 4. Spillway Approach (a. Condition-General b. Condition-General c. Condition-Concrete 5. Spillway Structure a. Condition 6. Spillway Structure 5. Spillway Structure 6. Spillway Structure	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. Iope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope. Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and grass starting to grow. Yes () No (X) - Condition of concrete satisfactory. Channel Portion of the upstream approach embankment washed out during heavy rains in September. SCG Hydro Services provided plant with written repair options. Remaining weeds and large rocks/boulders in approach channel need to be removed. (See Current Recommendation 12). Good. No evidence of instability. Good. Abutments/Deck Concrete in good condition. Floor Concrete in good condition	n/e n/a n/e n/e n/e n/e n/e n/e 17 & 18 n/e n/e
a. Condition b. Erosion/Sloughing c. Concrete Drainage Ditch 2. Crest a. Condition 3. Downstream Dike S a. Condition b. Seepage/Wet Spots c. Erosion/Sloughing d. Concrete Drainage Ditch 4. Spillway Approach (a. Condition-General b. Condition-General c. Condition-Concrete 5. Spillway Structure a. Condition 6. Spillway Structure a. Condition 7. Spillway Structure a. Condition 7. Spillway Structure a. Condition	Yes (X) No () - Localized, minor surface erosion. Concrete in good condition. No obstructions in channel observed. Satisfactory - Gravel surfaced. No distress or potholes in road surface observed. Iope (Storage Pond) Grass covered portion- Satisfactory. Rip Rap covered portion - Satisfactory Yes () No (X) - No seepage or wet spots observed on slope. Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and grass starting to grow. Yes () No (X) - Condition of concrete satisfactory. Channel Portion of the upstream approach embankment washed out during heavy rains in September. SCG Hydro Services provided plant with written repair options. Remaining weeds and large rocks/boulders in approach channel need to be removed. (See Current Recommendation 12). Good. No evidence of instability. Good. Abutments/Deck Concrete in good condition. Floor Concrete in good condition	n/a n/a n/a n/a n/a n/a n/a n/a

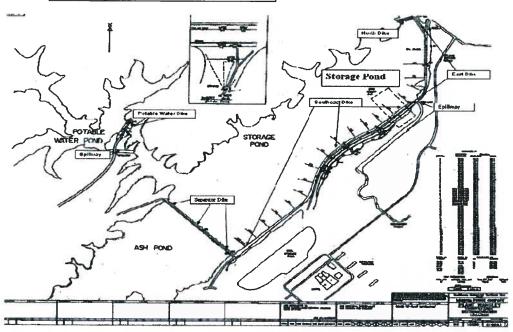
Plant Wansley 2009 - 4th Quarter Dam Safety Inspection Summary

VIII - Detention Por	nd	
	Observations- Comments	Photograph No.
1. Upstream Dike Slop	<u>e</u>	
a. Condition	Rip-rap in good condition.	n/a
b. Erosion/Sloughing	Yes () No (X) - Slope looks satisfactory, no visible instability observed	n/a
2. Crest		
a. Condition	Gravel surfaced and in good condition. No distress or potholes observed.	n/a
3. Downstream Dike S	lope	
a. Condition	Satisfactory Grass-covered. Needs cutting. No visible evidence of instability observed	n/a
b.Seepage/Wet Spots	Yes (x) No () - Localized, milnor wet water beyond toe of slope. Will continue to be	n/a
c. Erosion/Sloughing	Yes () No (X)	n/a
4. Concrete Spiliway C	hannel	<u> </u>
a. Concrete Condition	Concrete is in good condition	n/a
5.Spiilway Outlet Char	nel	
a. Condition	Rip-rap at outfall and outlet channel is in good condition. No issues observed.	n/a
VIII - Gypsum Pond	Dikes (No fluid in ponds)	
	Observations- Comments	Photograph No.
I. Upstream Dike Slop		
a. Condition	Good.	n/a
o. Erosion/Sloughing	Yes () No (X)	n/a
2. Crest		
a. Condition	Gravel surfaced and in good condition.	n/e
. Downstream Dike Si	оре	
a. Condition	Grass-covered. Good condition.	n/a
.Seepage/Wet Spots	Yes () No (X)	n/a
. Erosion/Sloughing	Yes () No (X)	n/a
		<u> </u>

anything

Hugh H. Armitage - Sr. Engineer
SCG - Hydro Services

Location Plan
Storage Pond and Potable Water Pond



2009 - 4th Quarter Inspection Photographs - October 16, 2009

(See accompanying report attached)

Photo No.	Description	
1	Storage Pond - East Dike - Crest - Localized depressions at crest should be filled with Graded Aggregate Base (GAB).	
2	Storage Pond - East Dike - Marker pole at Sta 0- 00 needs to be re-installed. Has been broken off at ground level	0000
3	Storage Pond - East Dike - Cut grass/brush down along toe of dike.	Cut grass/small brush at too of slope
4	Storage Pond - East Dike - Bare areas need to be re-grassed. (Use of a grass mat type product will minimize erosion/wash out during initial growth of grass)	

2009 - 4th Quarter Inspection Photographs - October 16, 2009

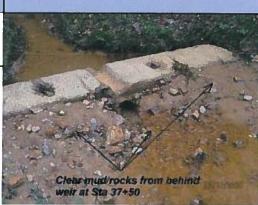
(See accompanying report attached)

Photo	Description	
No.		
5	Storage Pond - East Dike - Remove mud/debris from drainage ditches, along east dike.	
6	Storage Pond - East Dike -Toe Ditch - Remove overgrowth from drainage ditch	
7	Storage Pond - East Dike -Toe Ditch - Localized holes in concrete ditch need to be grouted to prevent water flow beneath concrete and potential ground loss	
8	Storage Pond- Southeast Dike - Rodent holes on slope need to be filled in.	Bedons Halo
		10/10/2009

2009 - 4th Quarter Inspection Photographs - October 16, 2009

(See accompanying report attached)

No.	Description
9	Storage Pond - Southeast Dike - Lower Slope - Sta 37+50. Clear mud/rocks from behind weir.



Storage Pond - Southeast Dike - Lower Slope Sta 37+00 - Undermined area at end of concrete
lined ditch fixed satisfactorily.



Lower Slope -Sta 37+50 - Ground loss resulting from defective/leaking (corroded) culvert allowing water to flow beneath and beside concrete ditch. Both items have been repaired since inspection on 10-16-09.



Storage Pond- Southeast Dike - Mid-slope
Ditch - Mud and vegetation needs to be removed from ditches.



12

2009 - 4th Quarter Inspection Photographs - October 16, 2009

(See accompanying report attached)

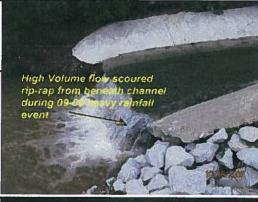
No.	Description
13	Storage Pond- Southeast Dike - Lower Slope - Toe Ditch - Mud needs to be removed from ditches.



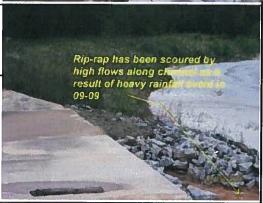
Ash Pond- Emergency Overflow - Downstream
 Scour adjacent to concrete channel which occurred during Sept-09 heavy rainfall at site.
 Requires repair.



Ash Pond- Emergency Overflow - End of Concrete Channel - Rip-rap has been scoured from beneath end of concrete channel following 09-09 heavy rainfall at site.



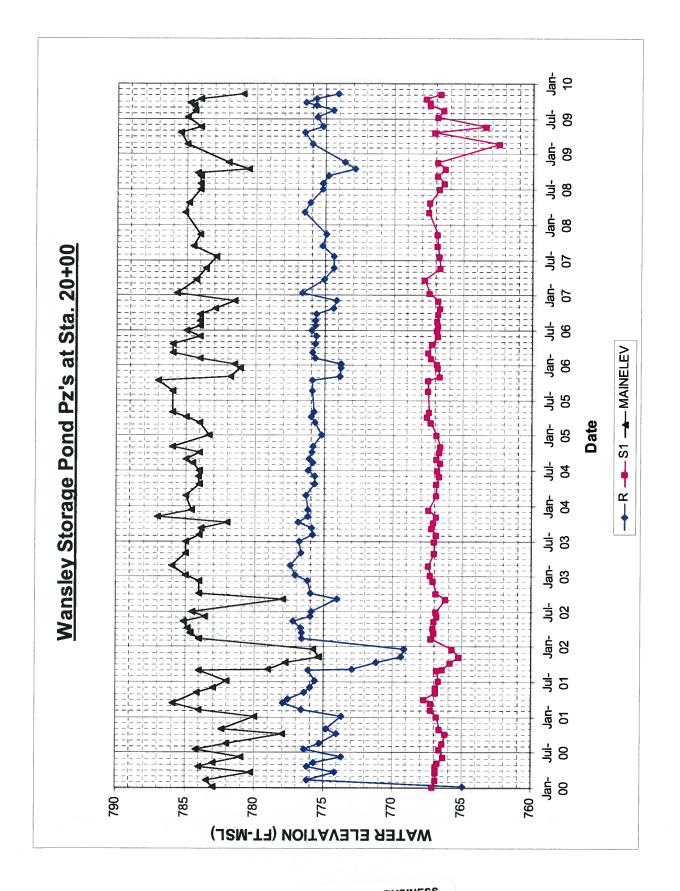
Ash Pond- Emergency Overflow - Spillway Rip-rap was scoured due to high flows in adjacent concrete channel following 09-09 rain event

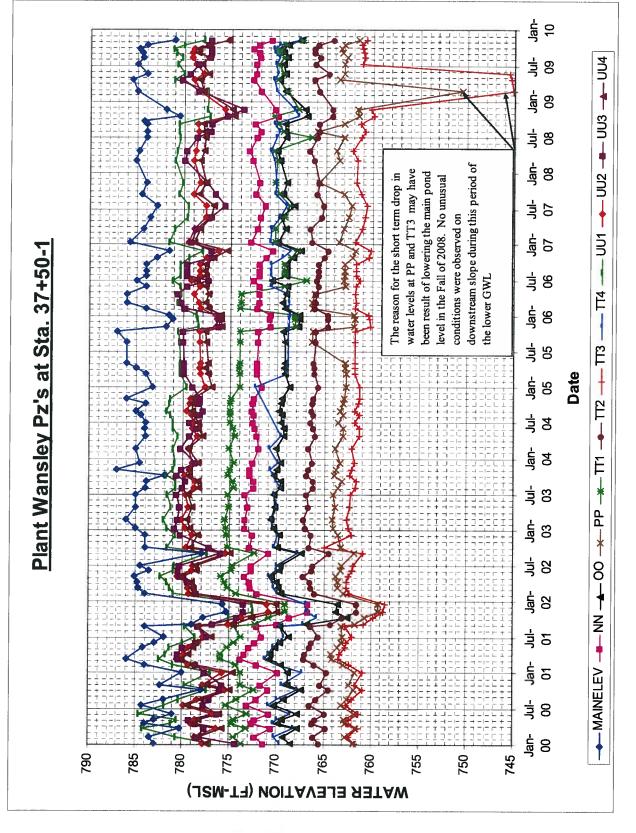


2009 - 4th Quarter Inspection Photographs - October 16, 2009

(See accompanying report attached)

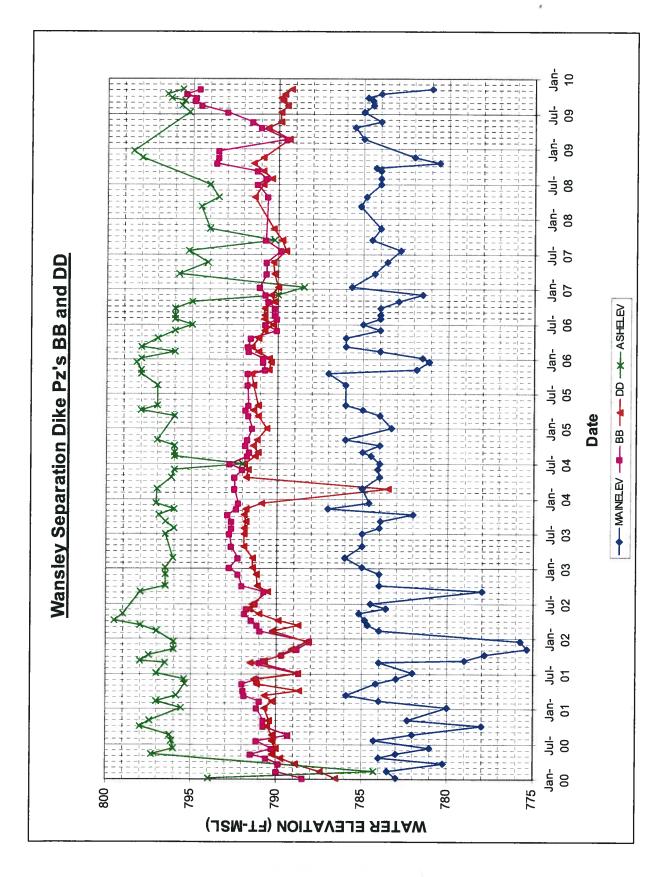
Photo No.	Description	
17	Potable Water Pond- Spillway - Upstream - Left, upstream spillway approach embankment needs to be repaired. Hydro Services has provided plant personnel with repair procedure.	
18	Potable Water Pond - Spillway (Upstream) - Rip-rap was scoured due to high flows in adjacent concrete channel following Sept-09 rain event.	

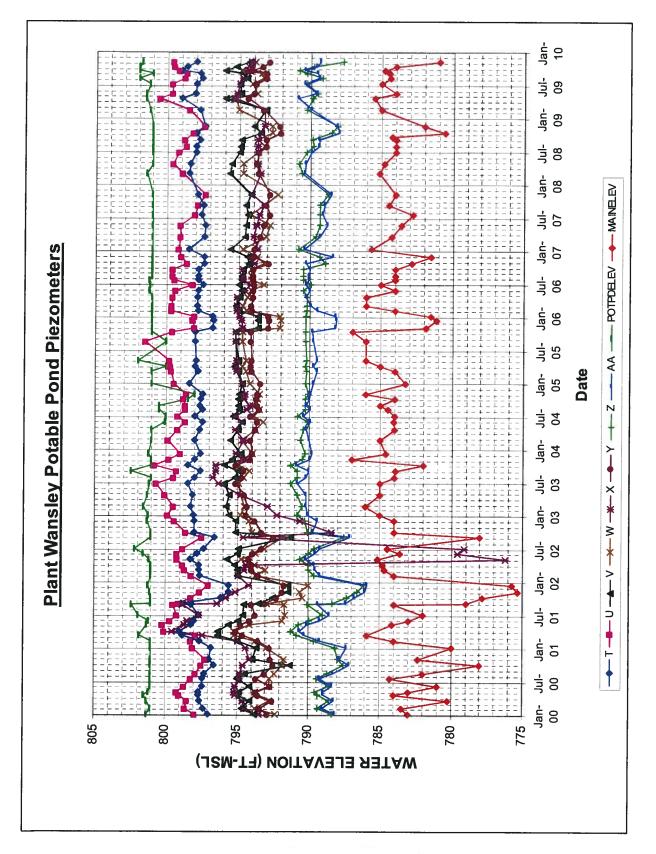




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Southern Company Generation Bin 10193 241 Ralph McGill Boulevard NE Atlanta, Georgia 30308-3374

Tel 404.506.7033



August 14, 2009

PLANT WANSLEY

Dam Safety Surveillance Quarterly Report REA No. WN-08900

Mr. J. P. Heilbron Plant Manager Georgia Power Co. Plant Wansley

Dear Mr. Heilbron:

Attached is the 1st Quarter 2009 report on Dam Safety Surveillance for Plant Wansley. The inspection of the Main Storage Pond, Ash Pond Separator Dike, Potable Water Pond and Detention Pond was performed on March 17 and 30, 2009 by Hugh Armitage of the SCG Hydro Services Group. Representatives for Plant McDonough accompanied Mr. Armitage on the inspections.

This report includes:

- a) A review of the current instrumentation data;
- b) The 1st Quarter 2009 Dam Safety Inspection Report summarizing the field observations and comments made during the March 17 and 30, 2009 inspections, and;
- c) A copy of the current instrumentation plots.

The current recommendations from the 1st Quarter Inspection are described on the first page of the attached report. The description and status of recommendations from the previous quarterly inspection are also described on pages 1 and 2 of the attached report.

Should you have any questions, please contact Hugh Armitage at extension 8-506-7109.

WAN-API 057

Sincerely,

Joel Galt

Hydro Services Supervisor

/hha

Attachments

xc: Georgia Power Company

S. J. Winston (w/ attachment)
N. I. Dean (w/ attachment)
B. Harcrow (w/ attachment)

Southern Company Services

D. E. Jones	(w/attachment)
E. B. Allison	(w/ attachment)
J. H. Crisler	(w/ attachment)
B. J. Peterson	(w/attachments)
K. Friedel	(w/ attachments)

Hydro Service Wansley Notebook

Master File: WN-09900

Hydro Services Correspondence Notebook (w/attachments)

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Instrumentation Data Review 1st Quarter – 2009

A current assessment of instrumentation data reviewed up to the most recent readings of, at Plant Wansley is as follows.

Storage Pond:

Sta. 20+00: Piezometers are generally within their historic range and seem to be tracking the pond level.

Sta 37+50-1: Piezometers PP and TT3 experienced a significant drop in elevation in the spring. It is not known why this occurred, but the readings for July have returned to their historic levels. All of these piezometers appear to be in their historic range. All generally are tracking the pond level.

Sta 37+50-2: All of these piezometers appear to be in their historic range. They generally are tracking the pond level although most have not yet responded to the recent, modest (November, 2008) increase in elevation

Sta 37+50 Pipe Flows: These flows appear to be in their historic ranges. These flow rates will continue to be monitored to assess if any trends develop. They seem to be tracking the pond level.

Sta 47+50: The piezometers are within their historic range of measurement.

It is recommended that the flow rate at the toe drain at Station 39+00 be obtained at monthly intervals rather than the current 6 month schedule. This information will be useful when modifications to the drains at 37+50 are carried out.

Sta 58+00: The level at piezometer LLL has dropped to more a more consistent historic level, which may be in part related to the decrease in pond level. Repair of a leaking valve, as noted in the attached quarterly report, is required for piezometer LLL.

<u>Relief Wells:</u> The relief wells appear to be discharging in their historic range, but also in response to the lower pond level.

Sta 65+00: Piezometric levels at C, E and MM are within their historic ranges.

Sta 70+00: All of these piezometers are reading in their historic ranges.

Weir and Pipe Flows: Weir measurements at Sta. 11+20 and 49+00 indicate a return to their historic range since the last quarterly inspection report.

Separation Dike:

These piezometers are generally registering in their historic ranges. Piezometer BB has recently exhibited an elevated water level. In discussions with plant personnel, we understand that piezometer BB was covered over during construction of the gypsum ponds this year. The piezometer pipe appears to have been broken off at some point when it was uncovered. This may explain the "apparent" elevated water level,

particularly if the measurements have been made from a lower reference elevation than previous measurements. Plant personnel are investigating and will advise Hydro Services.

Potable Water Pond Dike:

These piezometers were registering in their historic ranges and appear to respond to the decrease in the main pond level.

2009 - 1st Quarter Dam Safety Inspection Summary

Date of inspection:

March 17 & March 30, 2009

Inspection by: H. Armitage

Weather:

Temperature:

3-17-09 - Sunny, light breeze

3-17-09 - '50° F

3-30-09 - Sunny

3-30-09 - 40° F

Tracy Duke (Fluor - 3-17-09)

Brandon Harcrow (GPC-3-30-09)

Russell Poole (Flour- 3-30-09)

Rainfali (past 24 hrs):

SUMMARY

1. No major dam safety issues that would impact the safety of the structures were observed during this inspection. Recommendations to address current and previous inspection observations are summarized below. Many of the current and previous recommendations are routine, on-going maintenance type activities.

ADDITIONAL COMMENTS

- 1. Plant personnel did a very good job in completing most of the recommendations from previous quarterly inspections.
- 2. A copy of the Plant Wansley instrumentation data and review comments are attached.

CURRENT RECOMMENDATIONS					
No.	Description	Location	Status - Open/Closed		
1	Grassed areas on slope should be fertilized to promote healthy & more robust grass growth.	Storage/Potable Water and Detention Pond	Open		
2	Localized rutting on east dike from grass cutting equipment. Ruts to be filled in with soil and compacted and re-grassed	Storage Pond	Open		
3	Localized bare spots on slopes need to be re-grassed to prevent further erosion and silt washing into ditches	Storage Pond	Open		
4	Numerous rodent/animal burrow holes observed on downstream slopes (upstream slope at Detention Pond I). All require filling in. (Excerpt from FEMA Publication 473 - "Impact of Animals on Earthen Dams". Copy forwarded to plant personnel, provides options for repair).	Storage Pond - Various locations along east dike (downstream)	Open		
5	Numerous ant mounds observed on downstream slopes. Treatment required for ant mounds by fumigants or chemical methods as described in FEMA Publication 473 - "Impact of Animals on Earthen Dams". Copy forwarded to plant personnel, provides options for repair)	Storage Pond	Open		
6	A truckload of a). GDOT #10 washed sand and b). washed #89 stone is required to replace depleted stockpiles.	Storage Pond	Open		
7	Clean out debris and dirt in concrete lined toe ditches	Storage Pond - East Dike - Various locations	Open		
8	Storage Pond - Sta 37+00 - Repair undermining at end of concrete lined ditch	Storage Pond - East Dike -Lower Slope	Open		
9	Sta 45+00 & 49+00, 54+00 and Sta 57+00. Clean out ditch behind weirs and inside of the ends of drainage pipes.	Storage Pond- East Dike	Open		
10	Several holes observed in bottom of concrete lined ditch and water spouting out. Should be investigated further. Repair may be required to mitigate undermining of slab.	Ash Pond Emergency Overflow	Open		
11	End of concrete llined ditch is undermined. Needs to be repaired per recommendation in 5-28-08 inspection report (page 11 of 20).	Ash Pond Emergency Overflow	Open		
	STATUS OF PREVIOUS RECOMMENDATIONS				
No.	Location, Description & Action Required		Status - Open/Closed		
1	Storage Pond - Spillway - Downstream of end of spillway requires trees and bushes to be cut down so that flows are not restricted during flow. Completed Satisfactorily		Closed		
2	Detention Pond - Downstream Slope - Small bushes and trees on downstream slope need to be cut down. Completed satisfactorily		Closed		
3	Storage Pond -Cracks in concrete lined ditches should be cleaned out and caulked - Pending completion		Open		
4	Storage Pond - Downstream Slopes - Rodent holes to be filled and fire treated. (See Current Recommendations 4 & 5)	Closed			
5	Storage Pond - Downstream Siopes - Sta 19+00 & 22+00 and 37+50 need to be cleaned out and repaired Pending Completion	Open			

Plant Wansley 2009 - 1st Quarter Dam Safety Inspection Summary STATUS OF PREVIOUS RECOMMENDATIONS (con't) No. Location, Description & Action Required Status - Open/Closed Storage Pond - Downstream Slope - Sta 37+50 - Hydro Services investigated wet area 1/29/09 and further options to address will be investigated. Plant personnel needs to monitor this area DAILY for any evidence of distress or unusual events, or movement of Open slope and contact Hydro Services immediately particularly when pool elev. 782-785 ft. Pending Completion. Hydro Services has contacted Georgia Safe Dams Program to request an extension of time to review possible repair options. Storage Pond - Downstream Slope - approx. Sta 56+00 - Damaged marker pole for toe 7 Open drain needs to be repaired. Storage Pond - Downstream Slope - approx. Sta 62+00 - Damaged concrete ditch 8 Open needs to be fixed and accumulated silt removed. Storage Pond - Downstream Slope - Piezometer LLL - Piezometer leak at spigot 9 connection needs to be repaired. Ground surface is wet around piezometer - Pending Open Completion. **OBSERVATIONS FOR 1st QUARTER INSPECTION** i - Storage Pond - North Dike - (Road to Recreational Area) Storage Pond Elev. 785' (3-30-09) **Observations - Comments** Photograph No. 1. Upstream Slope a. Condition Grass covered - Overall condition is good. No evidence of instability. n/a b. Erosion/Sloughing Yes () No (X) n/a 2. Crest a. Condition Gravel surfaced - No distress or potholes in road surface observed. n/a 3. Downstream Slope a. Condition Grass covered - Overall condition is good. No evidence of instability. n/a b. Seepage/Wet Spots Yes () No (X) - No seepage or wet spots observed on slope. n/a c. Erosion/Sloughing Yes () No (X) n/a II - Storage Pond - East Dike (North Dike to Spiliway) **Observations - Comments** Photograph No. 1. Upstream Slope a. Condition Rip-rap on upstream face looks satisfactory and no dam safety issues observed. b. Erosion/Sloughing Yes () No (X) - No evidence of instability observed n/a 2. Crest Gravel surfaced - No distress or potholes in road surface observed. a. Condition n/a 3. Downstream Slope Grass covered and at a length that permitted a good visual inspection. Ruts from grass cutting equipment need to be repaired i.e. see Photo 1 @ Sta 0+00). Numerous bare spots on slopes and areas adjacent to concrete lined ditch need to be re-grassed to prevent further erosion and silt from washing into ditch (i.e. see Photo 2 @ ~Sta 6+10). a. Condition 1, 2 and 3 Place soil (where necessary), compact and re-establish grass. Rodent holes observed at Sta. 7+00, 17+40 (i.e. see Photo 3 @ Sta 7+00). Localized ant mounds observed on slopes. Need to be treated . All flagged for repair by plant personnel. (See Recommendations 1, 2, 3, 4 and 5) b. Seepage/Wet Spots Yes () No (X) - No seepage or wet spots observed on slope. n/a c. Erosion/Sloughing Yes () No (X) - No evidence of instability. n/a Concrete in good condition. Drain pipe at Sta. 19+00 need to be cleaned out still. New d. Concrete-Lined sections drain pipe required at drains at 19+00 and 22+00 to repair damaged/crushed n/a Drainage Ditch outlets ends. (See Previous Recommendation 7) Yes (X) No () - Needs tandem truckload of GDOT #10 washed sand and #89 stone. e. Emergency n/a (See Current Recommendations 6a) and 6b)) Aggregate Stockpiles iii - Storage Pond - Spiliway **Observations - Comments** Photograph No. 1. Spillway Abutment/Deck a Condition Concrete condition is satisfactory. 2. Spillway Floor a. Condition Concrete satisfactory n/a 3. Spillway Walls a. Condition Concrete satisfactory n/a 4. Spillway Gates Looked satisfactory. Slight water flow on RHS of RHS gate at bottom. Possible a leaky a. Condition 4 seal. Plant to investigate when gates opened. 5. Downstream of Spillway (Channel) a. Condition Vegetation downstream of spillway has been cleaned out satisfactorily. n/a

	<u>Plant Wañsley</u>	
	2009 - 1st Quarter Dam Safety Inspection Sum	<u>mary</u>
IV - Storage Pond -	Southeast Dike (Spillway to Separator Dike)	
	Observations - Comments	Photograph No.
1. Upstream Slope		
a. Condition	Rip-rap looks acceptable. No instability or beaching evident.	n/a
b. Erosion/Sloughing	Yes () No (X)	n/a
2. Crest		
a. Condition	Gravel surfaced/Railway tracks - No distress observed along road surface .	n/a
3. Downstream Slope		
3a - Upper Slope		
a. Condition	Grass covered - looks satisfactory.	n/a
. Seepage/Wet Spots	Yes () No (X)	n/a
: Erosion/Sloughing	Yes () No (X) - No evidence of instability.	n/a
Bb - Mid-Slope Road &		1
a. Road Condition	Gravel surfaced - looks good. No distress or potholes in road surface observed.	n/a
o. Concrete-Lined Orainage Ditch	Concrete in good condition. Cleanout debris in concrete lined ditches.	5
lc - Middle Slope		
a. Condition	Grass covered. Satisfactory. No visual evidence of instability. Rodent hole near head wall in 4th Qtr 2008 has been repaired.	n/a
. Seepage/Wet Spots	Yes () No (X)	n/a
. Erosion/Sloughing	Yes () No (X)	n/a
d. Lower Road & Drai	nage Ditch	
a. Road Condition	Gravel surfaced - looks good. No distress or potholes in road surface observed.	n/a
o. Concrete-Lined Orainage Ditch	Concrete condition is acceptable	n/a
le - Lower Slope		
a. Condition	Grass covered - Grass at a length that permitted a good visual examination. Several bare areas where require repair and treat ant mound and rodent holes at various locations. (See Current Recommendations 1, 3, 4 and 5)	6
o. Seepage/Wet Spots	Yes (X) No () 1. Ground surface in the area of Sta 37+50 is wet. No change from 4th Qtr visit. Safe Dams requested a report and repairs be done to address this area by 6-30-09. SCG Hydro Services to request extension to complete investigation and repairs (See photo 7). 2. Rodent hole flagged by plant personnel for repair near Sta 37+50. 3. The area beneath Drain 37+50D was repaired - okay. 4. The area beneath the end of the concrete drainage ditch was repaired but is undermined again. Needs to be repaired per recommendation in 5-28-08 inspection report (page 11 of 20) (See photo 8). 5. The marker sign for the toe drain near approx Sta 56+00 needs to be fixed. 6. Area around Piezometer LL is wet. Appears that the valve is leaking and needs to be fixed. (See Previous Recommendations 6, 7 and 9 and Current Recommendations 7 and 8)	7 and 8
c. Erosion or Sloughing	Yes () No (X) - No evidence of instability. Some of the localized bare spots in grass cover have been re-seeded, however several other bare areas require same repair.	n/a
. Concrete Drainage Ditch	Concrete condition is good. Sta 45+00, 49+00, 54+00 and 57+00 - Clean-out of debris/leaves in drain pipe, ditch and behind weir is required. (See Current Recommendations 9)	n/a
. Emergency Aggregate Stockpiles	Yes (X) No () - Need one truckload of GDOT washed #89 stone (See Current Recommendation 6b).)	n/a
f - Lower Concrete-Lin		
ı. Condition	Sta 62+00 - Portion of concrete channel is broken and needs to be repaired. Localized portion of concrete ditch needs to be cleaned out of silt and sandy material - (See photo 9 and Previous Recommendation 8)	9

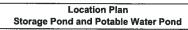
	Plant Wansley 2009 - 1st Quarter Dam Safety Inspection Summ	nary
V - Storage Pond/A	sh Pond - Separator Dike	Ash Pond Elev. Not measured
	Observations- Comments	Photograph No.
1. Upstream Slope (St	The state of the s	-
a. Condition	Rip-Rap - Looks satisfactory. No evidence of instability.	n/a
b. Erosion/Sloughing	Yes () No (X)	n/a
2. Crest		n = ***********************************
a. Condition	Gravel surfaced and in good condition. A few minor ruts that should be monitored and repaired if condition worsens	n/a
3. Downstream Slope	(Ash Pond)	
3a. North End		
a. Condition	Rip-Rap - Looks satisfactory. No evidence of instability	n/a
b. Erosion/Sloughing	Yes () No (X)	n/a
3b. South End - (No lo	nger applicable due to Gypsum Pond/berm construction)	
a. Condition	Concrete generally in good condition. Several localized areas where water flowing out of lioints and holes in concrete.	n/a
V - Ash Pond Emer	gency Overflow	Ash Pond Elev. 799.5'
1. Upstream Slope	Observations- Comments	Photograph No.
a. Condition	Rip-Rap - Looks satisfactory. Grass portion of slope generally looks satisfactory,	n/a
b. Erosion/Sloughing	Yes (X) No () - Minor erosion at toe of grassed portion of slope. Not serious at this time but condition should be monitored to assess whether condition deteriorate whiich will require repair.	10
2. Crest		
a. Condition	Gravel surfaced and in good condition.	-4-
3. Downstream Slope	oraver danaged and in good containor.	n/a
a. Condition	Rip-Rap - Looks satisfactory. No evidence of instability	n/a
b. Erosion/Sloughing	Yes () No (X)	n/a
4. Concrete Lined Eme		
a. Condition	Concrete in good condition.	n/a
5. Concrete Lined Ove	rflow Ditch	
a. Condition	Concrete in good condition. Several localized areas where hole in concrete have water spouting out (See photo 11). See Current Recommendation 10. Downstream outlet end of concrete lined ditch is undermined. Needs to be repaired per recommendation in 5-28-08 inspection report (page 11 of 20) (See photo 12). See Current Recommendation 11.	11 & 12
VII- Potable Water P	Pond	Potable Water Pond Elev. 801'
	Observations - Comments	Photograph No.
I. Upstream Dike Slope	e (Potable Water)	
a. Condition	Rip-rap on upstream face looks good.	n/a
. Erosion or Sloughing	Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching has been done tand grass is growing.	n/a
c. Concrete Drainage Ditch	Concrete in good condition. No obstructions in channel observed.	n/a
. Crest		
. Condition	Gravel surfaced - looks good. No distress or potholes in road surface observed.	n/a
. Downstream Dike Si	ope (Storage Pond)	
. Condition	Grass covered - Overall - looks good. No evidence of instability	n/a
. Seepage/Wet Spots	Yes () No (X) - No seepage or wet spots observed on slope.	n/a
. Erosion or Sloughing	Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching done and grass starting to grow.	n/a
I. Concrete Drainage Ditch	Yes () No (X) - Condition of concrete satisfactory.	n/a
. Spillway Approach C	<u>hannel</u>	
. Condition - General	Small bushes/trees removed.	n/a
. Condition - Rip-Rap	Good. No evidence of instability.	n/a
. Condition - Concrete	Good.	n/a
i. Spillway Structure - /	Abutments/Deck	
. Condition	Concrete in good condition.	n/a
. Spillway Structure -	Floor	
	Concrete in good condition	n/a
. Spiilway Structure -	The state of the s	
. Condition	Concrete - Good CONFIDENTIAL BUSINESS	n/a
	INFORMATION	

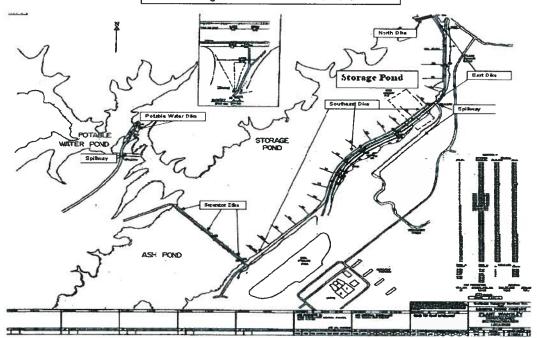
2009 - 1st Quarter Dam Safety Inspection Summary

VIII	- D	etei	ntio	n Po	ond

	Observations- Comments	Photograph No.
1. Upstream Dike Slo	<u>De</u>	· · · · · · · · · · · · · · · · · · ·
a. Condition	Rip-rap in good condition.	n/a
b. Erosion/Sloughing	Yes () No (X) - Slope looks satisfactory, no visible instability observed	n/a
2. Crest		
a. Condition	Gravel surfaced and in good condition. No distress or potholes observed.	n/a
3. Downstream Dike S	llope	
a. Condition	Grass-covered. Small bushes/trees have been cut down and grass has been cut. Looks satisfactory. No visible evidence or instability observed	n/a
b. Visible Seepage or Wet Spots	Yes (X) No () - Localized, minor ponded water beyond toe of slope	n/a
c. Erosion/Sloughing	Yes () No (X)	n/a
4. Concrete Spillway (Channel	
a. Concrete Condition	Concrete is in good condition	n/a
5.Spillway Outlet Cha	nnel .	
a. Condition	Rip-rap at outfall and outlet channel is in good condition. No issues observed.	n/a

Hugh H. Armitage - Sr. Engineer
SCG - Hydro Services





2009 - 1st Quarter Inspection Photographs - March 17 and 30, 2009

(See accompanying report attached)

Photo No.	Description	
1	Storage Pond - East Dike - Localized wheel ruts/erosion need to be repaired on slope.	



Storage Pond - East Dike - Localized eriosion
 and bare areas on dike slope and adjacent to toe ditch. Re-establish grass cover.



3 Storage Pond - East Dike - Localized rodent holes observed on slope need to be filled in.



Storage Pond - Spillway Gates - Slight gap
 between wall and edge of gate allowing water through. Plant to investigate.



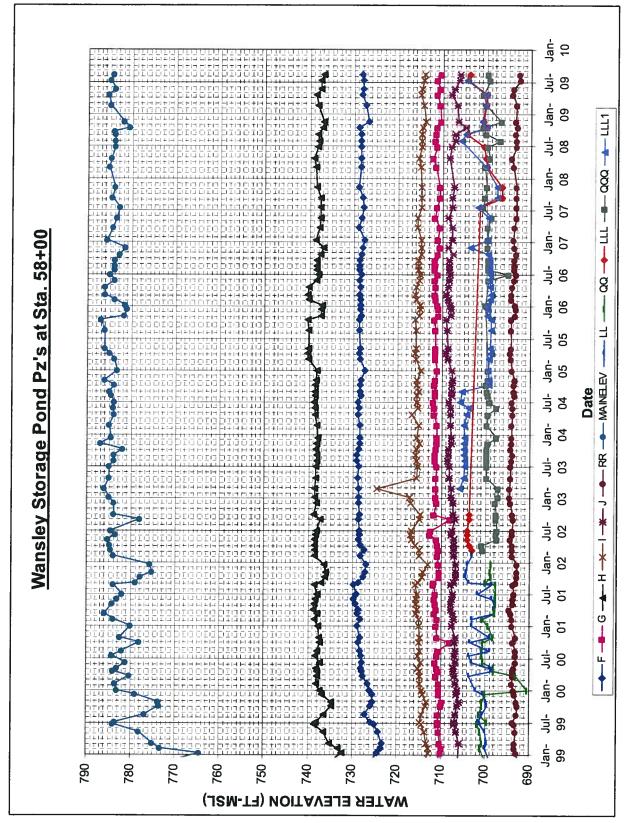
2009 - 1st Quarter Inspection Photographs - March 17 and 30, 2009

Photo No.	Description	
5	Storage Pond - East Dike -Toe Ditch - Mid-slope - Bare spots on slope need to be re-graseed.	
		03/30/2004
6	Storage Pond - East Dike -Lower Slope - Bare spots on slope need to be re-graseed.	
		DOTAGE OF THE PARTY OF THE PART
7	Storage Pond - East Dike - Lower Slope - Sta 37+50. Condition unchanged from previous 4th Qtr visit.	A Company of the Comp
		G/MASS
8	Storage Pond - East Dike -Lower Slope - Sta 37+00 - Repair undermining at end of concrete lined ditch	

2009 - 1st Quarter Inspection Photographs - March 17 and 30, 2009

	(See accompanying report	attached)
Photo No.	Description	
9	Storage Pond - Southeast Dike - Downstream-Lower Slope -Sta 62+00 - Damaged concrete needs repair and removal of silt removal at bottom of ditch.	Silitor be remayed
10	Ash Pond- Emergency Overflow - Upstream - Minor erosion at toe of slope at waterline. Shoud be monitored for further deterioration that will require repair	Minor Erosion at toe
11	Ash Pond- Emergency Overflow - Bottom of Concrete Ditch - Several holes in concrete and water spouting from hole. Needs to be investigated and repaired.	Assignment of the second of th
		Holes in concrete. Appears that water spouting out from beneath slab. Salibilities.
12	Ash Pond- Emergency Overflow - Downstream End - Erosion at end of ditch resulting in underminig of concrete. Needs to be repaired.	

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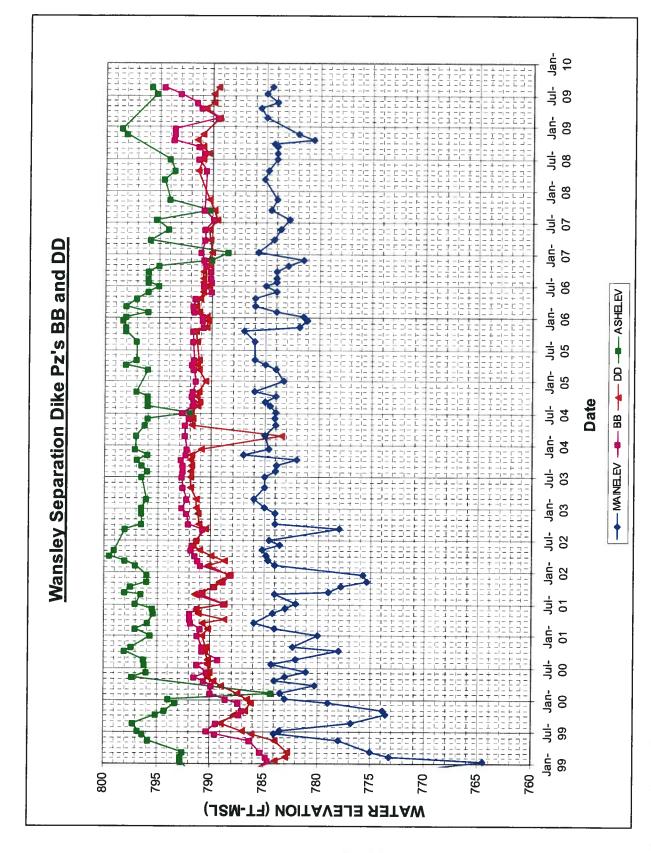


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CONFIDENTIAL BUSINESS

INFORMATION

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Southern Company Generation Bin 10193 241 Ralph McGill Boulevard NE Atlanta, Georgia 30308-3374

Tel 404.506.7033



January 12, 2009

PLANT WANSLEY

Dam Safety Surveillance Quarterly Report REA No. WN-08900

Mr. J. P. Heilbron Plant Manager Georgia Power Co. Plant Wansley

Dear Mr. Heilbron:

Attached is the 4th Quarter 2008 report on Dam Safety Surveillance for Plant Wansley. The inspection of the Main Storage Pond, Ash Pond Separator Dike, Potable Water Pond and Detention Pond was performed on November 12, 2008 by Hugh Armitage of the SCG Hydro Services Group. This inspection coincided with the annual inspection of the Storage Pond Main Dike by the Georgia Department of Natural Resources, Environmental Protection Division, Safe Dams Program (SDP). The inspections were coordinated with Mr. T. E. Wilson of Plant Wansley.

This report includes:

CONFIDENTIAL BUSINESS INFORMATION

- a) A review of the current instrumentation data;
- b) A table summarizing visual observations made during the November 12, 2008 inspections. This report is supplemented with relevant site photographs; and
- c) A copy of the current instrumentation data.

As a result of the 4th quarterly inspection, the following recommendations have been discussed with plant personnel and are shown in Table 1. The status for corrective action for previous 2008 recommendations are noted in Table 2.

TABLE -1 Recommendations from 4th Quarter Inspections

WAN-API 056

No.	Location and Description
1	Storage Pond - Downstream Slopes - Rodent holes to be filled and fire ant mounds to be treated. Locations flagged in field by plant personnel
2	Storage Pond - Downstream Slopes - Sta 19+00 & 22+00 and 37+50D - Drain pipes need to be cleaned out and repaired. Undermined concrete ditch to be repaired (per Hydro Services 5-28-08 letter).

No.	Location and Description
3	Storage & Potable Water Ponds - Downstream Slopes - Sta 62+00 & other localized areas on slopes. This is an on-going maintenance item.
4	Storage Pond - Downstream Slope - Sta 37+50 - Hydro Services to investigate wet area in 1st Qtr 2009. Plant personnel needs to monitor this area WEEKLY for any evidence of movement of slope and contact Hydro Services immediately if observed.
5	Storage Pond - Downstream Slope - approx. Sta 56+00 - Damaged marker pole for toe drain needs to be repaired.
6	Storage Pond - Downstream Slope - approx. Sta 62+00 - Damaged concrete ditch needs to be fixed and accumulated silt removed.
7	Storage Pond - Downstream Slope - Piezometer LLL - Piezometer leak at spigot connection needs to be repaired. Ground surface is wet around piezometer
9	Detention Pond - Downstream Slope - Small bushes and trees on downstream slope need to be cut down. Grass on slope needs to be cut. Source of ponded water downstream of toe needs to be investigated.

Table 2 - Recommendations from Previous 2008 Quarterly Inspections

No.	Location, Description & Action	Status Open/Closed
1	Storage Pond - Spillway - Downstream of end of spillway requires trees and bushes to be cut down so that flows are not restricted during flow Pending Completion	Open
2	Detention Pond - Downstream Slope - Small bushes and trees on downstream slope need to be cut down. Grass on slope needs to be cut - Pending Completion	Open
3	Various Locations -Cracks in concrete lined ditches should be cleaned out and caulked - Pending completion	Open

Should you have any questions, please contact Hugh Armitage at extension 8-506-7109.

Sincerely,

CONFIDENTIAL BUSINESS INFORMATION

Joel Galt

Hydro Services Supervisor

/hha

Attachments

xc: Georgia Power Company

T. E. Wilson (w/ attachment)
N. I. Dean (w/ attachment)

Southern Company Services

E. B. Allison (w/ attachment)
J. H. Crisler (w/ attachment)
F. J. Pryor (w/ attachment)

Hydro Service Wansley Notebook Master File: WN-08900

Hydro Services Correspondence Notebook (w/attachments)

T:\Quarterly Reports\Fossil Plants\2008\Wansley\08-4th Qtr\1 - 08-Qtr 4 - Cover Letter.DOC

Instrumentation Data Review 4th Quarter – 2008

A current assessment of instrumentation data reviewed up to the most recent readings of November 18, 2008, at Plant Wansley is as follows.

Storage Pond:

Sta. 20+00: Piezometers are generally within their historic range and seem to be tracking the pond level.

Sta 37+50-1: All of these piezometers appear to be in their historic range. All generally are tracking the pond level.

Sta 37+50-2: All of these piezometers appear to be in their historic range. They generally are tracking the pond level although most have not yet responded to the recent, modest (November, 2008) increase in elevation

Sta 37+50 Pipe Flows: These flows appear to be in their historic ranges. These flow rates will continue to be monitored to assess if any trends develop. They seem to be tracking the pond level.

Sta 47+50: The piezometers are within their historic range of measurement.

It is recommended that the flow rate at the toe drain at Station 39+00 be obtained at monthly intervals rather than the current 6 month schedule. This information will be useful when modifications to the drains at 37+50 are carried out.

Sta 58+00: The level at piezometer LLL has dropped to more a more consistent historic level, which may be in part related to the decrease in pond level. Repair of a leaking valve, as noted in the attached quarterly report, is required for piezometer LLL.

<u>Relief Wells:</u> The relief wells appear to be discharging in their historic range, but also in response to the lower pond level.

Sta 65+00: Piezometric levels at C, E and MM are within their historic ranges.

Sta 70+00: All of these piezometers are reading in their historic ranges.

Weir and Pipe Flows: Weir measurements at Sta. 11+20 and 49+00 indicate a return to their historic range since the last quarterly inspection report.

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Separation Dike:

These piezometers are generally registering in their historic ranges. Piezometer BB has recently exhibited an elevated water level. In discussions with plant personnel, we understand that piezometer BB was covered over during construction of the gypsum ponds this year. The piezometer pipe appears to have been broken off at some point when it was uncovered. This may explain the "apparent" elevated water level, particularly if the measurements have been made from a lower reference elevation than

previous measurements. Plant personnel are investigating and will advise Hydro Services.

Potable Water Pond Dike:

These piezometers were registering in their historic ranges and appear to respond to the decrease in the main pond level.

2008 - 4th Quarter Inspection Summary Dam Safety Surveillance

Date of Inspection:

November 12, 2008

Inspection by: H. Armitage

Weather:

Cloudy

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(Storage Pond T. Wilson - GPC Inspection)

Temperature: ~48° to 64° F INFORMATION

T. Tucker - Fluor

Rainfall (past 24 hrs): 0

A. Murray, S. Madichetty - Ga SDP

SUMMARY

1. No major dam safety issues that would impact the safety of the structures were observed during this inspection. Recommendations to address current and previous inspection observations are summarized below. Many of the current and previous recommendations are routine, on-going maintenance type activities.

ADDITIONAL COMMENTS

- 1. Plant personnel did a very good job in completing most of the recommendations from previous quarterly inspections.
- 2. 4th Quarter Inspection done in conjunction with GA EPD Safe Dams Program 2009 Annual Inspection
- A representative from the plant accompanied the inspectors and Hydro Services on the Storage Pond surveillance. The representative placed small flags where items that need to be corrected.
- 4. A copy of the Plant Wansley instrumentation data and review comments are attached.

	CURRENT RECOMMENDATIONS - 4th Quarter Inspection	
No.	Location and Description	Photo No.
1	Storage Pond - Downstream Slopes - Rodent holes to be filled and fire ant mounds to be treated. Locations flagged in field by plant personnel	3, 9 & 13
2	Storage Pond - Downstream Slopes - Sta 19+00 & 22+00 and 37+50D - Drain pipes need to be cleaned out and repaired. Undermined concrete ditch to be repaired (per Hydro Services 5-28-08 letter)	5, 6, 14 & 1
3	Storage & Potable Water Ponds - Downstream Slopes - Sta 62+00 & other localized areas on slopes. This is an on-going maintenance item.	11 & 22
4	Storage Pond - Downstream Slope - Sta 37+50 - Hydro Services to investigate wet area in 1st Qtr 2009. Plant personnel needs to monitor this area WEEKLY for any evidence of movement of slope and contact Hydro Services immediately if observed.	12
5	Storage Pond - Downstream Slope - approx. Sta 56+00 - Damaged marker pole for toe drain needs to be repaired.	16
6	Storage Pond - Downstream Slope - approx. Sta 62+00 - Damaged concrete ditch needs to be fixed and accumulated silt removed.	17
7	Storage Pond - Downstream Slope - Piezometer LLL - Piezometer leak at spigot connection needs to be repaired. Ground surface is wet around piezometer	18
9	Detention Pond - Downstream Slope - Small bushes and trees on downstream slope need to be cut down. Grass on slope needs to be cut. Source of ponded water downstream of toe needs to be investigated.	23 & 24
	STATUS OF PREVIOUS RECOMMENDATIONS	
No.	Location, Description & Action	Status Open/Closed
1	Storage Pond - Spillway - Downstream of end of spillway requires trees and bushes to be cut down so that flows are not restricted during flow Pending Completion	Open
2	Detention Pond - Downstream Slope - Small bushes and trees on downstream slope need to be cut down. Grass on slope needs to be cut - Pending Completion	Open
3	Various Locations -Cracks in concrete lined ditches should be cleaned out and caulked - Pending completion	Open
	Storage Pond - North Dike - Warning sign needs to be re-mounted on pole - Fixed -	Closed

2008 - 4th Quarter Inspection Summary

STATUS OF PREVIOUS RECOMMENDATIONS (continued)

No.	Location, Description & Action	Status Open/Closed
5	Storage Pond & Potable Water Pond - General - Localized bare spots on grass covered slopes need repair. Re-seeding and mulching required to prevent further surface erosion. Many areas have been reseeded. This is an on-going maintenance item.	Closed
6	Storage Pond - East and SE Dike - Concrete Lined Ditches & Toe Drains - Localized vegetation growth and debris needs to be removed from ditches so that flow isn't restricted from approx. Sta.1+60 to 3+50 and Sta. 5+00 to 23+50. Drains at Sta 73+00 & 74+00 need to be cleaned of silt and debris Completed	Closed
7	Storage Pond - SE Dike (Southwest End) and Potable Water Pond (U/S Slope) - Trees and bushes need to be cut down at toe of slopes to mitigate root growth (seepage path) into embankment - Corrected	Closed
8	Storage Pond - SE Dike- Lower Slope - Rodent holes on lower slope near Sta. 67+60 and 68+40 need to be filled in Repaired	Closed
9	Separator Dike - Upstream & Downstream Slopes - Localized erosion rills/gullies need to be repaired to mitigate further erosion Fixed	Closed
10	Potable Water Pond Spillway - Upstream end of spillway. (Approach channel) requires trees and bushes to be cut down and cleared so that flows are not restricted - Larger trees/bushes have been cut down	Closed
11	General Comment & Sta 1+00 to 8+00 - Remove accumulated vegetation from within concrete lined channels -Completed	Closed
12	Sta. 75+00, 76+00 & 77+00 - Clean out end of finger drains - Completed	Closed
13	Multiple Locations on slopes -Fill in animal burrows - Rodent holes observed in 1st Qtr filled or could not be found during current inspection - See Current Reco. 6 above	Closed
14	Sta 73+00 & 74+00 - Bare spots on slope need to be re-seeded Some area have become grown over with grass cover satisfactorily. See Current Reco. 2 above	Closed
15	Sta. 66+00, 70+00 & 72+50 - Repair end of damaged toe drains - The ends have been fixed.	Closed
16	Sta 62+00 - Clean Silt behind Weir - Cleaned out	Closed
17	Separator Dike - Runoff erosion at crest of upstream slope repaired	Closed
18	Localized bare spots on slope to be re-seeded (or covered with small rip-rap).	Closed
19	Upstream Toe of Slope- Vegetation needs to be cut down - Bushes and trees cut down - ok	Closed

Plant Wansley 2008 - 4th Quarter Inspection Summary **OBSERVATIONS FOR 4th QUARTER INSPECTION** Storage Pond - Storage Pond - North Dike - (Road to Recreational Area) Elev. 778.1' **Observations - Comments** Photograph No. 1. Upstream Slope a. Condition Grass covered - Overall condition is good. No evidence of instability. n/a b. Erosion/Sloughing Yes () No (X) n/a 2. Crest Gravel surfaced - No distress or potholes in road surface observed. Sign has been rea. Condition 1 mounted on post 3. Downstream Slope Grass covered - Overall condition is good. No evidence of instability. a. Condition n/a b. Seepage/Wet Spots Yes () No (X) - No seepage or wet spots observed on slope. n/a c. Erosion/Sloughing Yes () No (X) n/a II - Storage Pond - East Dike (North Dike to Spillway) **Observations - Comments** Photograph No. 1. Upstream Slope a. Condition Rip-rap on upstream face looks satisfactory and no dam safety issues observed. n/a b. Erosion/Sloughing Yes () No (X) - No instability observed n/a 2. Crest a. Condition Gravel surfaced - No distress or potholes in road surface observed. n/a 3. Downstream Slope Grass covered - Overall - Grass has been cut. Localized bare spots noted on previous inspections have been re-seeded & straw placed over seed. Grass starting to grow in a. Condition 2, 3 some places. (On-going maintenance issue). Rodent hole at Sta. 7+00 (photo 3), localized fire ant mounds. All flagged for repair by plant personnel. b. Seepage/Wet Spots Yes () No (X) - No seepage or wet spots observed on slope. n/a c. Erosion/Sloughing Yes () No (X) - No evidence of instability. n/a Concrete in good condition. Localized vegetation growth and debris has been removed d. Concrete-Lined from ditches (photo 4). Good job done. Drain pipes at Sta. 19+00 and 22+00 need to 4,5 & 6 Drainage Ditch be cleaned out and repaired (photos 4 and 5) e. Emergency Yes (X) No () Aggregate Stockpiles III - Storage Pond - Spillway **Observations - Comments** Photograph No. 1. Spillway Abutment/Deck Concrete condition is satisfactory. Per Ga SDP letter 4-3-08, spall at left abutment has a. Condition 7 been repaired. 2. Spillway Floor Concrete satisfactory a. Condition n/a 3. Spillway Walls a. Condition Concrete satisfactory n/a 4. Downstream of Spillway (Channel) Vegetation downstream of spillway still needs to be cleared to prevent a. Condition n/a

CONFIDENTIAL BUSINESS INFORMATION

blockage/restriction of flow capacity of channel - Pending activity.

Plant Wansley
2008 - 4th Quarter Inspection Summary

IV - Storage Pond -	IV - Storage Pond - Southeast Dike (Spillway to Separator Dike)	
	Observations - Comments	Photograph No.
1. Upstream Slope		·
a. Condition	Rip-rap (size varies) on upstream slope looks acceptable. No instability or beaching evident. (SCG sent response to GA EPD-SDP letter 4/3/08. SCG will continue ongoing monitoring of rip-rap). Bushes and trees at toe of slope adjacent to separator dike have been cut down satisfactorily (See Photo 8).	8
b. Erosion/Sloughing	Yes () No (X)	n/a
2. Crest		<u>L </u>
a. Condition	Gravel surfaced/Railway tracks - No distress observed along road surface .	n/a
3. Downstream Slope		
3a - Upper Slope		
a. Condition	Grass covered - looks satisfactory. Grass recently cut so good visual examination possible.	n/a
b. Seepage/Wet Spots	Yes () No (X)	n/a
c. Erosion/Sloughing	Yes () No (X) - No evidence of instability.	n/a
3b - Mid-Slope Road &	Drainage Ditch	
a. Road Condition	Gravel surfaced - looks good. No distress or potholes in road surface observed.	n/a
b. Concrete-Lined	Concrete in good condition. Vegetation in ditch/around drain at Sta. 73+00 has been	
Drainage Ditch	removed satisfactorily.	n/a
3c - Middle Slope		
a. Condition	Grass covered. Satisfactory. No visual evidence of instability. New rodent hole encountered near head wall and was flagged by plant personnel.	9
b. Seepage/Wet Spots	Yes () No (X)	n/a
c. Erosion/Sloughing	Yes () No (X)	n/a
3d. Lower Road & Drai		
a. Road Condition	Gravel surfaced - looks good. No distress or potholes in road surface observed.	n/a
b. Concrete-Lined Drainage Ditch	Concrete condition is acceptable	n/a
3e - Lower Slope		
a. Condition	Grass covered - Grass has recently been cut. Overall good coverage. Localized reseeding has been carried out to fix bare spots. Several additional areas require repair.	10 & 11
b. Seepage/Wet Spots	Yes (X) No () 1. Ground surface in the area of Sta 37+50 is wet. Minor and localized visible water ponded. (photo 12). EPD inspector stated that corrective action be taken to address wet area by 6-2009. This issue was addressed in response letter to EPD dated 10-28-08. 2. Rodent hole flagged by plant personnel for repair near Sta 37+50 (photo 13). 3. The area beneath Drain D is undermined and needs to be repaired (photo 14). 4. The area beneath the end of the concrete drainage ditch is undermined and needs to be repaired (photo 15). 5. The marker sign for the toe drain near approx Sta 56+00 needs to be fixed. Has been damaged. (photo 16). 6. Area around Piezometer LL is wet. Appears that the valve is leaking and needs to be fixed (photo 17).	12, 13, 14 15, 16 and 17
c. Erosion or Sloughing	Yes () No (X) - No evidence of instability. Some of the localized bare spots in grass cover have been re-seeded, however several other bare areas require same repair.	n/a
d. Concrete Drainage Ditch	Concrete condition is good. Sta 49+00 - Clean-out of debris/leaves in drain pipe, ditch and behind weir is required.	n/a
e. Emergency Aggregate Stockpiles	Yes (X) No ()	n/a
3f - Lower Concrete-Lir	ned Drainage Ditch	
a. Condition	Sta 62+00 - Portion of concrete channel is broken and needs to be repaired. Localized portion of concrete ditch needs to be cleaned out of silt and sandy material	18

Plant Wansley 2008 - 4th Quarter Inspection Summary		
		Ash Pond Elev. 799.5'
	Observations- Comments	Photograph No.
1. Upstream Slope (St		
a. Condition	Rip-Rap - Looks satisfactory. Size of stone on surface of slope varies. No evidence of instability.	n/a
b. Erosion or Sloughing	Yes (X) No () - Localized, shallow erosion rills/gullies have been repaired (filled in with stone/rip-rap).	19
2. Crest		
a. Condition	Gravel surfaced and in good condition. New gravel has been placed on roadway.	n/a
3. Downstream Slope	(Ash Pond)	
3a. North End		
a. Condition	Rip-Rap - Looks satisfactory. No evidence of instability	n/a
b. Erosion or Sloughing	Yes (X) No () - Localized, shallow erosion rills/gullies have been repaired (filled in with stone/rip-rap).	n/a
3b. South End - (No lo	nger applicable due to Gypsum Pond/berm construction)	
a. Condition	N/A - Recently constructed gypsum storage pond dike now abuts the south end of the separator dike.	n/a
VII- Potable Water Pond		Potable Water Pond Elev. 801'
	Observations - Comments	Photograph No.
1. Upstream Dike Slop	e (Potable Water)	
a. Condition	Rip-rap on upstream face looks good. Small trees and bushes at toe have been cut down.	20
b. Erosion or Sloughing	Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching has been done to reestablish grass growth in bare areas.	21
c. Concrete Drainage Ditch	Concrete in good condition. No obstructions in channel observed.	n/a
2. Crest		
a. Condition	Gravel surfaced - looks good. No distress or potholes in road surface observed.	n/a
3. Downstream Dike SI	ope (Storage Pond)	
a. Condition	Grass covered - Overall - looks good. No evidence of instability	n/a
b. Seepage/Wet Spots	Yes () No (X) - No seepage or wet spots observed on slope.	n/a
c. Erosion or Sloughing	Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching required to re-establish vegetative growth.	22
d. Concrete Drainage Ditch	Yes () No (X) - Condition of concrete satisfactory.	n/a
4. Spillway Approach C	<u>channel</u>	
a. Condition - General	Small bushes/trees removed.	n/a
b. Condition - Rip-Rap	Good. No evidence of instability.	n/a
c. Condition - Concrete	Good.	n/a
5. Spillway Structure -	Abutments/Deck	
a. Condition	Concrete in good condition.	n/a
6 . Spillway Structure -		
a. Condition	Concrete in good condition	n/a
7 . Spillway Structure -		
a. Condition	Concrete - Good	n/a

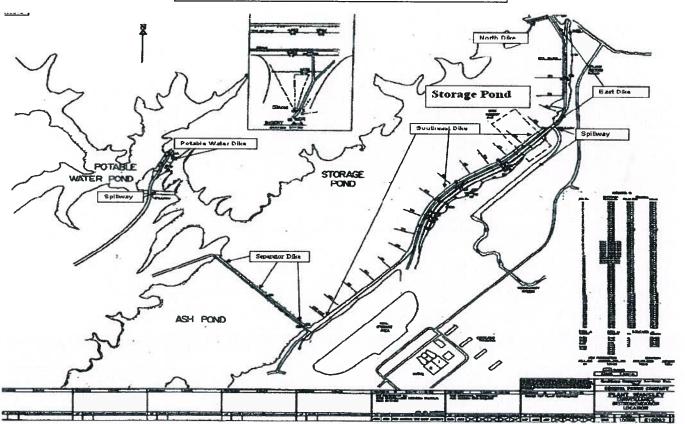
2008 - 4th Quarter Inspection Summary

VIII - Detention Pon	<u>id</u> i kan kala kala kala kala kala kala kala	
	Observations- Comments	Photograph No
1. Upstream Dike Slop	<u>e</u>	
a. Condition	Rip-rap in good condition.	n/a
b. Erosion/Sloughing	Yes () No (X) - Slope looks satisfactory, no visible instability observed	n/a
2. Crest		
a. Condition	Gravel surfaced and in good condition. No distress or potholes observed.	n/a
3. Downstream Dike Si	ope_	
a. Condition	Grass-covered. Central area of slope has small bushes/trees that need to be removed. Grass requires cutting. No visible evidence or instability observed	23
b. Visible Seepage <i>or</i> Wet Spots	Yes (X) No () - Localized, minor ponded water beyond toe of slope	24
c. Erosion or Sloughing	Yes () No (X)	n/a
4. Concrete Spillway C	hannel	<u> </u>
a. Concrete Condition	Concrete is in good condition	n/a
5.Spillway Outlet Chan	nel	
a. Condition	Rip-rap at outfall and outlet channel is in good condition. No issues observed.	n/a

CONFIDENTIAL BUSINESS INFORMATION

Hugh H. Armitage - Sr. Engineer
SCG - Hydro Services

Location Plan
Storage Pond and Potable Water Pond



2008 - 4th Quarter Inspection Photographs - November 12, 2008 (See accompanying report attached)

No.	Description	
1	Storage Pond - N Dike - Near Rec. Center Entrance - Notice sign re-mounted on post	

Photo



Storage Pond - East Dike - Localized areas have
 been re-seeded to re-establish grass on former bare areas



Storage Pond - East Dike - Localized rodent holes to be filled in and fire ant mounds to be treated. The rodent holes and fire ant mounds were flagged by plant personnel



4 Storage Pond - East Dike - Ditches cleaned out of debris/silt



2008 - 4th Quarter Inspection Photographs - November 12, 2008 (See accompanying report attached)

No.	Description
5	Storage Pond - East Dike - Drain pipe at Station 19+00 needs to be cleaned out. Drain needs to be extended to prevent future clogging



Storage Pond - East Dike - Drain pipe at Station 22+00 needs to be fixed. Crushed end to be replaced/repaired

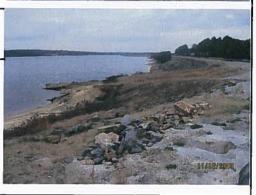


Storage Pond - Spillway - Concrete spall on downstream wall has been fixed.



Storage Pond - Southeast Dike - Upstream - Bushes and trees have been removed at toe of slope.





2008 - 4th Quarter Inspection Photographs - November 12, 2008 (See accompanying report attached)

Photo No.	Description
9	Storage Pond- Southeast Dike - Downstream - Sta 64+00 - Rodent Hole to be repaired.



Storage Pond- Southeast Dike - Downstream Sta 64+00 - Areas of slope have been repaired by re-seeding to re-establish grass cover.



Storage Pond- Southeast Dike - Downstream Near Sta 62+00 - Bare area on slope to be seeded to re-establish grass cover.



Storage Pond - Southeast Dike - Downstream-Lower Slope at Sta 37+ 50. - EPD Safe Dams Program stated that a repair of wet area required by June 2009



2008 - 4th Quarter Inspection Photographs - November 12, 2008

(See accompanying report attached)

Photo No.	Description
13	Storage Pond - Southeast Dike - Downstream- Lower Slope at Sta 37+ 50. Rodent hole to be repaired. Flagged by field personnel.



Storage Pond - Southeast Dike - Downstream-Lower Slope at Sta 37+ 50 Drain D -Undermining at end of concrete drainage pipe requires repair.



Storage Pond - Southeast Dike - Downstream-Lower Slope at Sta 37+ 50 - Undermining at end of concrete drainage ditch requires repair. (See repair procedure in letter of May 28, 2008)



Storage Pond - Southeast Dike - Downstream - Lower Slope at Sta 56+00 - Damaged drain marker to be replaced

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16

2008 - 4th Quarter Inspection Photographs - November 12, 2008

(See accompanying report attached)

Photo No.	Description
17	Storage Pond - Southeast Dike - Downstream - Lower Slope - Wet area around Piezometer LLL. Appears that drain valve is leaking. Needs to be repaired.



Storage Pond - Southeast Dike - Downstream-Lower Slope -Sta 62+00 - Damaged concrete needs repair and removal of silt removal at bottom of ditch.



Storage Pond - Spearator Dike- Upstream and Downstream - Erosion gullies have been repaired with stone/rip-rap



Potable Water Pond- Upstream Slope - Small trees and bushes have been cut down at downstream toe.



2008 - 4th Quarter Inspection Photographs - November 12, 2008 (See accompanying report attached)

Photo No.	Description
21	Potable Water Pond- Upstream Slope - Localized bare areas have been re-seeded and grass is starting to grow.



Potable Water Pond- Downstream Slope Localized bare areas need to re-seeded and grass reestablished

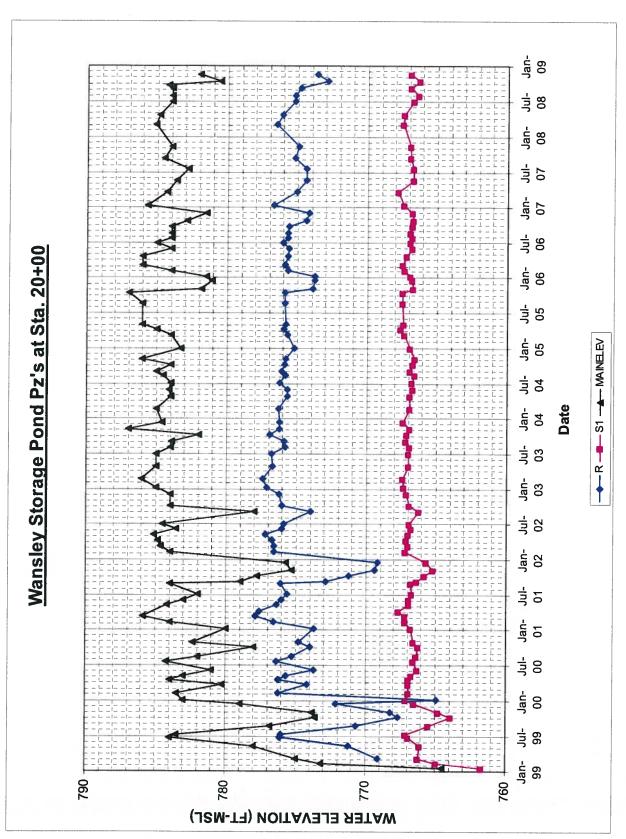


Detention Pond - Downstream Slope. Small trees to be removed. Grass and weeds to be cut.

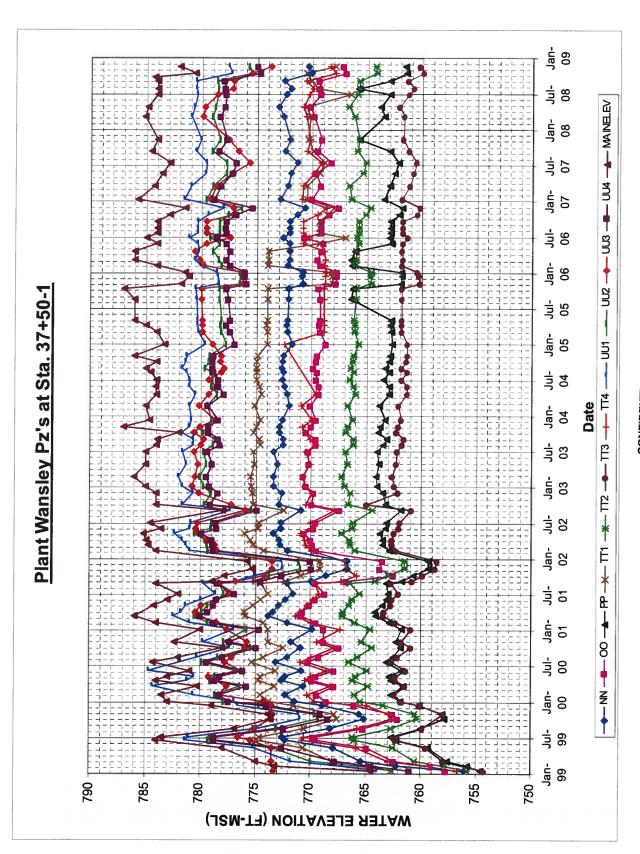


24 Detention Pond- Downstream - Localized wet areas downsteam of toe of slope. Need to investigate source of water. Plant to coordinate with Hydro Services.

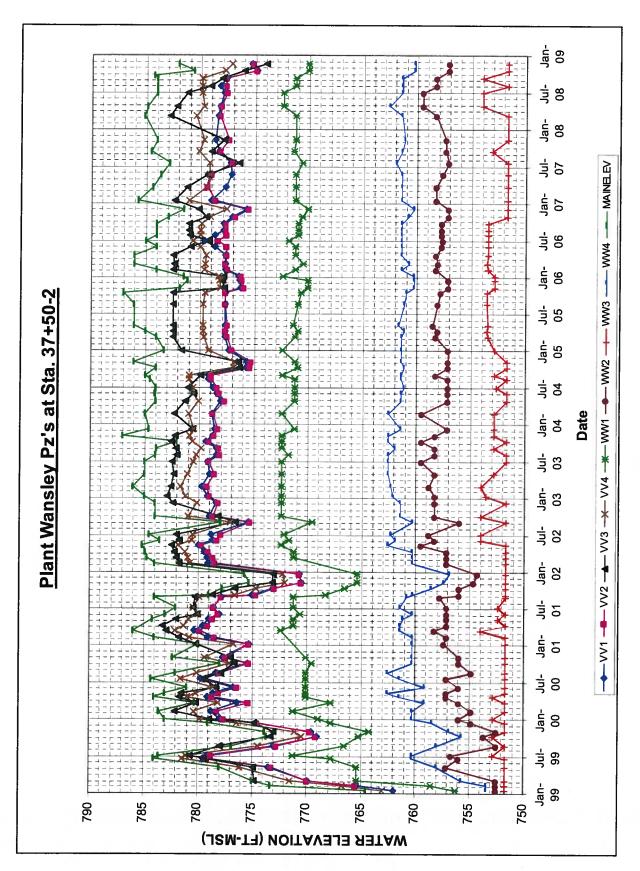




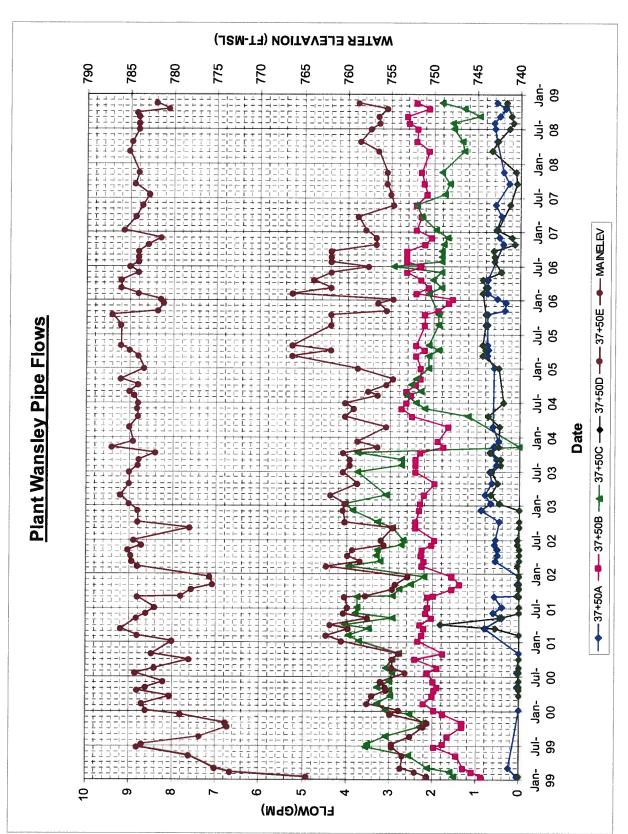
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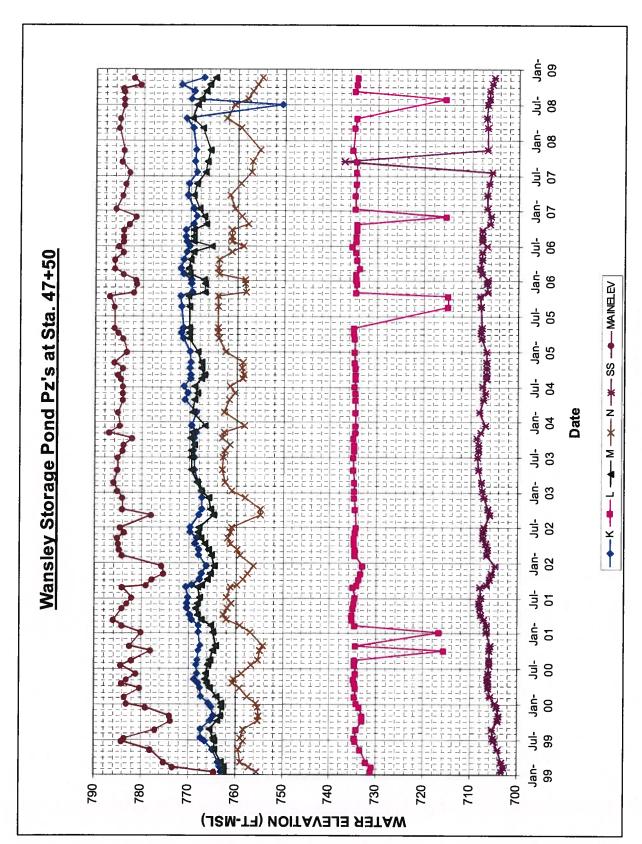
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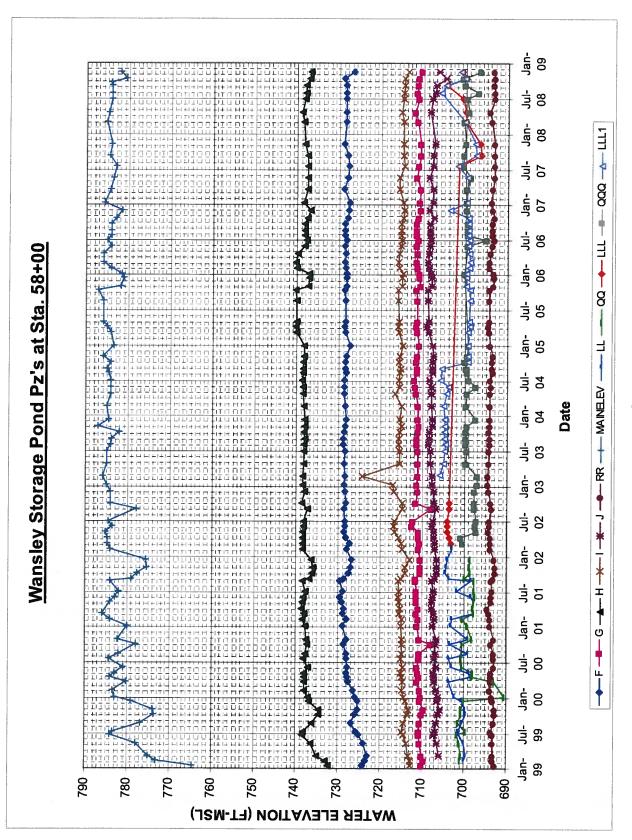
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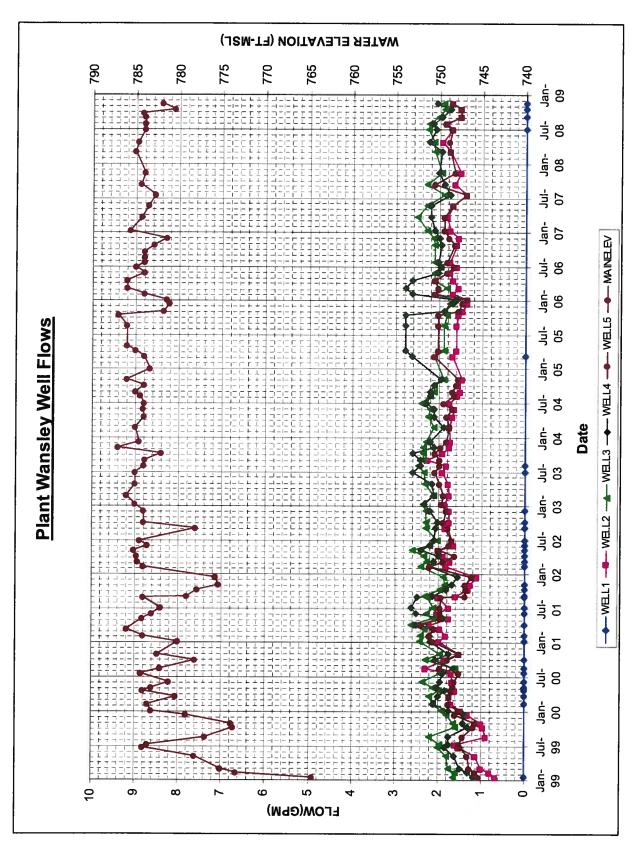
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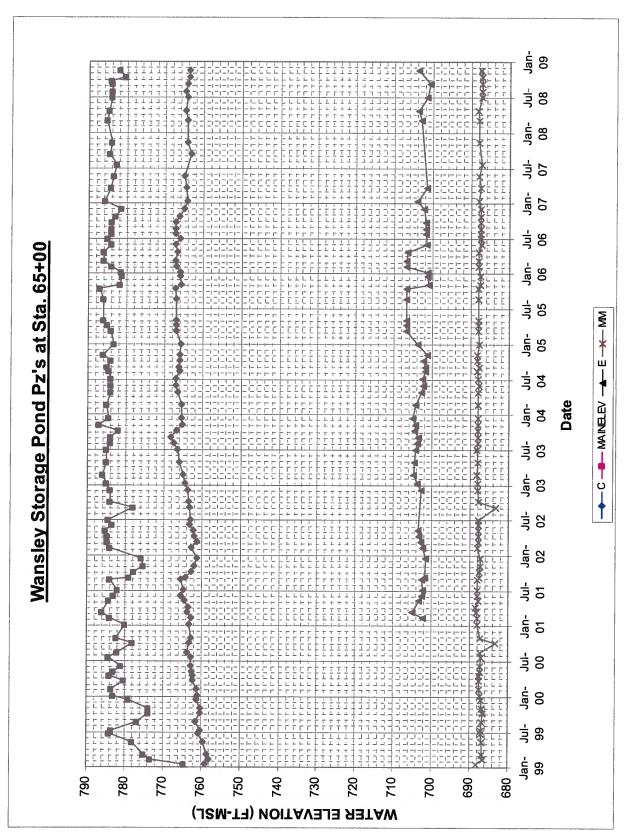
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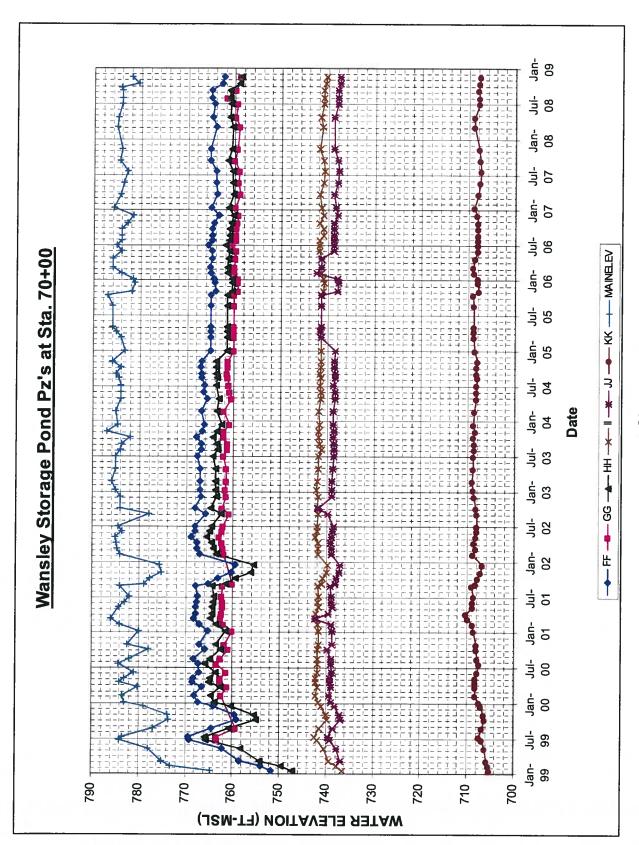
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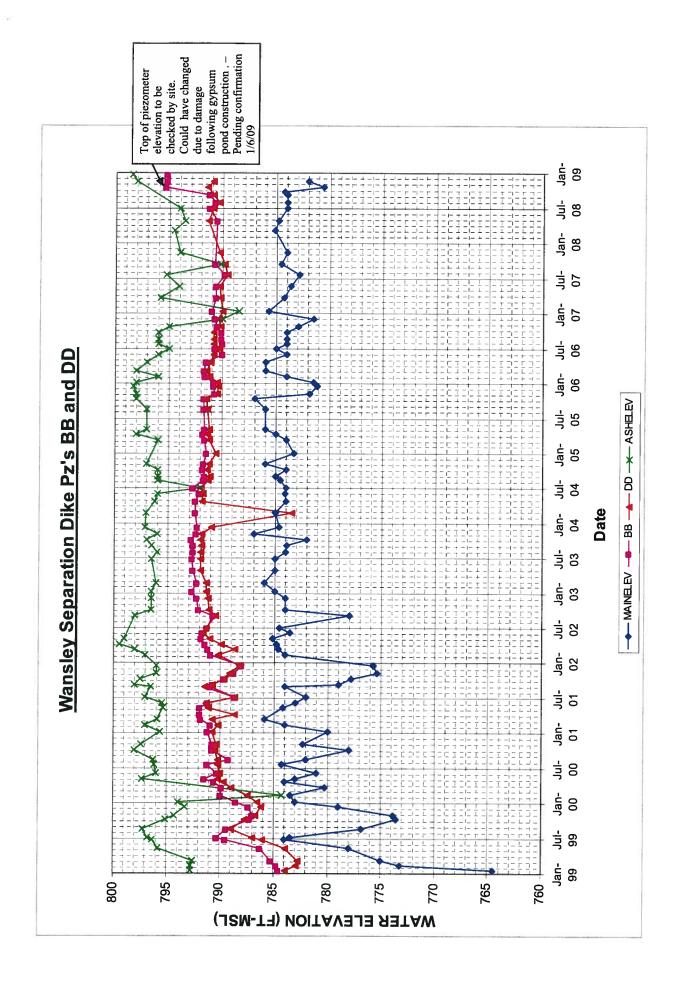
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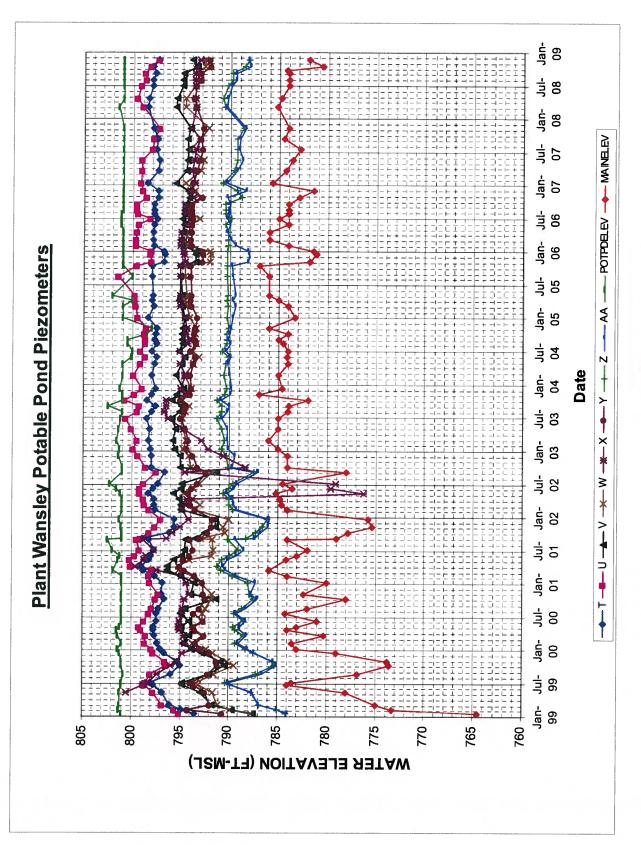
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CONFIDENTIAL BUSINESS INFORMATION

Southern Company Generation Bin 10193 241 Ralph McGill Boulevard NE Atlanta, Georgia 30308-3374

Tel 404.506.7033



September 9, 2008

PLANT WANSLEY

Dam Safety Surveillance Quarterly Report REA No. WN-08900

Mr. J. P. Heilbron Plant Manager Georgia Power Co. Plant Wansley

Dear Mr. Heilbron:

Attached is the 3rd quarter 2008 report on Dam Safety Surveillance for Plant Wansley. The inspection of the main Storage Pond, Ash Pond Separator Dike, Potable Water Pond and Detention Pond was performed on July 22, 2008 by Hugh Armitage of the SCG Hydro Services Group. The inspections were coordinated with Mr. T. E. Wilson and Mr. N. I. Dean of Plant Wansley.

This report includes:

- a) A review of the current instrumentation data;
- b) A table summarizing visual observations made during the July 22, 2008 inspections. The table includes current recommendations and a status update for recommendations contained in the previous quarterly report. This report is supplemented with relevant site photographs; and
- c) A copy of the current instrumentation data.

Should you have any questions, please contact Hugh Armitage at extension 8-506-7109.

Sincerely,

Joel Galt

Hydro Services Supervisor

WAN-API 055

CONFIDENTIAL BUSINESS INFORMATION

/hha

Attachments

xc: Georgia Power Company

T. E. Wilson (w/ attachment)
N. I. Dean (w/ attachment)

Southern Company Services

D. E. Jones (w/attachment)
E. B. Allison (w/ attachment)
J. H. Crisler (w/ attachment)
F. J. Pryor (w/ attachment)

Hydro Service Wansley Notebook

Master File: WN-08900

Hydro Services Correspondence Notebook (w/attachments)

T:\Quarterly Reports\Fossil Plants\2008\Wansley\08-3rd Qtr\1-08 Qtr 3 - Cover Letter .DOC

Instrumentation Data Review 3rd Quarter – 2008

A current assessment of instrumentation data reviewed up to the most recent readings of July 31, 2008 at Plant Wansley, is as follows.

Storage Pond:

Sta. 20+00: Piezometers are within their historic range. Both seem to track the pond level.

Sta 37+50: All of these piezometers appear to be in their historic range. All generally seem to track the pond level.

Sta 37+50 Pipe Flows: These flows appear to be in their historic ranges. These flow rates will continue to be monitored to assess if any trends develop.

Sta 47+50: With the exception of L, the other piezometers are within their historic range of measurement. Piezometer L experienced a sharp decline at the end of July 2008. However, a reading taken on September 8, 2008, indicated that the level is back to within it's historic range. Piezometer K re-bounded to its more historic value in July 2008.

It is recommended that the flow rate at the toe drain at Station 39+00 be obtained at monthly intervals rather than the current 6 month schedule. This information will be useful when modifications to the drains at 37+50 are carried out.

Sta 58+00: LLL1 has continued to display an increasing rise in the piezometric level in July 2008. This trend will continue to be monitored on a monthly basis. The other piezometers are generally within their historic range.

Relief Wells: The relief wells appear to be discharging in their historic range.

Sta 65+00: Piezometric levels at C, E and MM are within their historic ranges.

Sta 70+00: All of these piezometers are reading in their historic ranges.

<u>Weir and Pipe Flows</u>: Weir measurements at Sta. 49+00 have indicated a higher than normal flow the past two measurements (July 31 and September 8, 2008). It is recommended that site personnel continue to measure/monitor the flow rate at this weir on a weekly basis to see if this rate is sustained or reverts back to its historic range. Please keep SCG Hydro Services advised.

Separation Dike:

These piezometers are registering in their historic ranges. They exhibit a muted relationship with the storage pond elevation but little relationship to the ash pond elevation.

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Potable Water Pond Dike:

These piezometers were registering in their historic ranges.

2008 - 3rd Quarter Inspection Summary Dam Safety Surveillance

Date of Inspection:

July 22, 2008

Inspection by: H. Armitage

Weather:

Sunny to Mostly Cloudy

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Temperature:

~75° to 91° F

INFORMATION

Rainfali (past 24 hrs): 0.74"

SUMMARY

1. No major dam safety issues that would impact the safety of the structures were observed during this inspection. Recommendations to address current and previous inspection observations are summarized below. Generally, these recommendations are routine, on-going maintenance type activities.

ADDITIONAL COMMENTS

- 1. Plant personnel do a very good job of maintaining the embankment slopes at the different ponds at the site.
- 2. A copy of the Plant Wansley instrumentation data and review comments are attached.

CURRENT RECOMMENDATIONS - 3rd Quarter Inspection		
No.	Location and Description	Photo No.
1	Storage Pond - North Dike - Warning sign needs to be re-mounted on pole	1
2	Storage Pond & Potable Water Pond - General - Localized bare spots on grass covered slopes need repair. Re-seeding and mulching required to prevent further surface erosion.	
3	Storage Pond - East and SE Dike - Concrete Lined Ditches & Toe Drains - Localized vegetation growth and debris needs to be removed from ditches so that flow isn't restricted from approx. Sta.1+60 to 3+50 and Sta. 5+00 to 23+50. Vegetation removal required at outlet of Drain at Sta. 19+00. Drains at Sta 73+00 & 74+00 need to be cleaned of silt and debris.	
4	Storage Pond Spillway - Downstream of end of spillway requires trees and bushes to be cut down and cleared so that flows are not restricted during flow.	8
5	Storage Pond - SE Dike (South End) and Potable Water Pond (U/S Slope) - Trees and bushes need to be cut down at toe of slopes to mitigate root growth (seepage path) into embankment. Storage Pond - SE Dike- Lower Slope - Rodent holes on lower slope near Sta. 67+60 and 68+40 need to be filled in. Storage Pond - SE Dike- Lower Slope - Sta 37+50 - Wet area and small scarp on slope needs to be monitored for any signs of movement on a weekly basis. Storage Pond - SE Dike- Lower Slope - Sta 39+00. Increase the flow measurement frequency at the drain at Sta 39+00 to monthly rather than the current 6 month schedule. This information will be useful when modifications to the drains at 37+50 are carried out.	
6		
7		
8		
9	Separator Dike - Upstream & Downstream Slopes - Localized erosion rills/gullies need to be repaired to mitigate further erosion.	
10	Potable Water Pond Spillway - Upstream end of spillway. Approach channel) requires trees and bushes to be cut down and cleared so that flows are not restricted.	
11	Detention Pond - Downstream Slope - Small bushes and trees on slope need to be cut down. Grass on slope needs to be cut.	24

PREVIOUS RECOMMENDATIONS - 1st Quarter Inspection			
No.	Description & Action	Location	Status Open/Closed
1	General Comment & Sta 1+00 to 8+00 - Remove accumulated vegetation from within concrete lined channels - Pending Completion - See Current Reco. 3 above - See photos 4, 5 & 12).	Storage Pond	Open
2	Various Locations -Cracks in concrete lined ditches should be cleaned out and caulked - Pending completion	Storage Pond	Open
3	Sta. 75+00, 76+00 & 77+00 - Clean out end of finger drains - Completed.	Storage Pond	Closed
4	Multiple Locations on slopes -Fill in animal burrows - Rodent holes observed in 1st Qtr filled or could not be found during current inspection - See Current Reco. 6 above - See Photo 14).	Storage Pond	Closed

Plant Wansley 2008 - 3rd Quarter Inspection Summary

PREVIOUS RECOMMENDATIONS - 1st Quarter Inspection - Continued

No.	Description & Action	Location	Status Open/Closed
5	Sta 73+00 & 74+00 - Bare spots on slope need to be re-seeded Some area have become grown over with grass cover satisfactorily. See Current Reco. 2 above. (photos 2, 6, 16 and 20)		Closed
6	Sta. 66+00, 70+00 & 72+50 - Repair end of damaged toe drains - The ends have been fixed.	Storage Pond	Closed
7	7 Sta. 62+50 - near toe of slope - Bare spots on slope need to be reseeded sand mulched. See Current Reco. 2 above (Photos 2, 6, 16, & 20)		Open
8	Sta 62+00 - Clean Silt behind Weir - Cleaned out	Storage Pond	Closed
9 Sta 19+00 - Clean out vegetation of outlet of toe drain - Pending completion St		Storage Pond	Open
10	Sta 37+00 - Undermining at end of concrete channel - Pending completion Separator Dike - Runoff erosion at crest of upstream slope Pending completion - See Current Rec. 9 - See photos 17 & 18) Localized bare spots on slope to be re-seeded (or covered with small riprap). Pending completion (See Current Reco. 2 above) Upstream Toe of Slope- Vegetation needs to be cut down - Pending completion - See Current Reco. 5 above - See photos 5 & 10)		Open
11			Open
12			Open
13			Open

Plant Wansley 2008 - 3rd Quarter Inspection Summary **OBSERVATIONS FOR 3rd QUARTER INSPECTION** Storage Pond - Storage Pond - North Dike - (Road to Recreational Area) Elev. 783.6' **Observations - Comments** Photograph No. 1. Upstream Slope a. Condition Grass covered - Overall condition is good. No evidence of instability. n/a b. Erosion/Sloughing Yes () No (X) - No beaching observed. n/a 2. Crest Gravel surfaced - looks good. No distress or potholes in road surface observed. a. Condition 1 Warning sign needs to be re-mounted onto pole. 3. Downstream Slope Grass covered - Overall condition is good. No evidence of instability. Localized bare a. Condition 2 spots on slope where re-seeding/mulching required to prevent further erosion. b. Visible Seepage or Yes () No (X) - No seepage or wet spots observed on slope. n/a Wet Spots Yes () No (X) c. Erosion or Sloughing n/a II - Storage Pond - East Dike (North Dike to Spillway) **Observations - Comments** Photograph No. 1. Upstream Slope Rip-rap on upstream face looks satisfactory and no dam safety issues observed. - No a. Condition n/a beaching observed. Rip Rap varies in size. b. Erosion/Sloughing Yes () No (X) n/a 2. Crest a. Condition Gravel surfaced - looks good. No distress or potholes in road surface observed. n/a 3. Downstream Slope Grass covered - Overall - looks good. Grass cutting in progress. Localized bare spots a. Condition 3 & 6 need to be re-seeded & mulched (photo 6 - Sta 11+20) b. Visible Seepage or Yes () No (X) - No seepage or wet spots observed on slope. n/a Wet Spots c. Erosion or Sloughing Yes () No (X) n/a Concrete in good condition. Localized vegetation growth and debris needs to be d. Concrete-Lined removed from ditches so that flow isn't restricted from approx. Sta.1+60 to 3+50 and 4 & 5 Drainage Ditch Sta. 5+00 to 23+50. e. Emergency Yes (X) No () 7 Aggregate Stockpiles III - Storage Pond - Spillway **Observations - Comments** Photograph No. 1. Spillway Abutment/Deck Concrete condition is satisfactory. Per Ga SDP letter 4-3-08, crack in left abutment a. Condition needs to be repaired, just downstream of the gate needs to be fixed. SCG Hydro n/a Services to review during next site inspection. 2. Spillway Floor a. Condition Concrete condition is satisfactory n/a

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Vegetation downstream of spillway needs to be cleared to prevent blockage/restriction

4. Downstream of Spillway (Channel)

Concrete condition is satisfactory

of flow capacity of channel

3. Spillway Walls
a. Condition

a. Condition

n/a

8

	2008 - 3rd Quarter Inspection Summary	
IV - Storage Pond -	Southeast Dike (Spillway to Separator Dike)	
	Observations - Comments	Photograph No.
1. Upstream Slope		
a. Condition	Rip-rap (size varies) on upstream slope looks acceptable. No instability or beaching evident. (SCG to assess condition per GA EPD-SDP letter 4/3/08). Bushes and trees at toe of slope need to be cut down to prevent potential instability issues at toe of slope (see Photo 10)	9, 10
b. Erosion or Sloughing	Yes () No (X)	n/a
2. Crest		
a. Condition	Gravel surfaced/Railway tracks - No distress or potholes observed in road surface .	n/a
3. Downstream Slope		
3a - Upper Slope		
a. Condition	Grass covered - looks satisfactory. No evidence of instability observed.	11
b. Visible Seepage or Wet Spots	Yes () No (X) - No seepage or wet spots observed on slope.	n/a
c. Erosion or Sloughing	Yes () No (X) - No evidence of instability.	n/a
3b - Mid-Slope Road &	Drainage Ditch	
a. Road Condition	Gravel surfaced - looks good. No distress or potholes in road surface observed.	n/a
b. Concrete-Lined Drainage Ditch	Concrete in good condition. Vegetation in ditch/around drain at Sta. 73+00 needs to be removed so that flow isn't restricted. Localized areas where grass growing and silt has built up on bottom of drainage ditch. Removal of vegetation and silt required	12
3c - Middle Slope		
a. Condition	Grass covered. Satisfactory. No visual evidence of instability. Previous rodent hole filled in.	13
b. Visible Seepage or Wet Spots	Yes () No (X)	n/a
c. Erosion or Sloughing	Yes () No (X)	n/a
3d. Lower Road & Drail	nage Ditch	<u> </u>
a. Road Condition	Gravel surfaced - looks good. No distress or potholes in road surface observed.	n/a
b. Concrete-Lined Drainage Ditch	Concrete condition is acceptable	n/a
<u>3e - Lower Slope</u>		
a. Condition	Grass covered - Overall - looks good. Grass requires cutting but this did not affect satisfactory visual assessment. Several rodent holes at Stations 67+60 (near headwall) and 68+40 - see photo 14.	14
b. Visible Seepage or Wet Spots	Yes (X) No () - Wet spot observed at ground surface on lower slope at Sta 37+50. A localized area where a minor slough was observed. Plant personnel were advised of observation and they will routinely monitor condition.	15
c. Erosion or Sloughing	Yes () No (X) - No evidence of instability. Localized bare spots in grass cover adjacent to drainage ditch and other localized other areas.	16
d. Concrete Drainage Ditch	Concrete condition is good. Vegetation in ditch at Sta. 73+00 needs to be removed so that flow ditch is not restricted.	n/a
e. Emergency Aggregate Stockpiles	Yes (X) No ()	n/a
3f - Lower Concrete-Lin		
a. Condition	Condition is acceptable	n/a

	Plant Wansley 2008 - 3rd Quarter Inspection Summary		
V - Storage Pond/Ash Pond - Separator Dike			
	Observations- Comments	783.6' Photograph No.	
1. Upstream Slope (St		, motographi ito.	
a. Condition	Rip-Rap - Looks satisfactory. Size of stone on surface of slope varies. No evidence of instability.	n/a	
b. Erosion or Sloughing	Yes (X) No () - Localized, shallow erosion rills/gullies need to be repaired (filled in with stone/rip-rap).	17	
2. Crest			
a. Condition	Gravel surfaced and in good condition. No distress or potholes observed on road surface. Clean up of mud on road from construction vehicles required.	n/a	
3. Downstream Slope	(Ash Pond)		
3a. North End			
a. Condition	Rip-Rap - Looks satisfactory. Size of stone on surface of slope varies. No evidence of instability	n/a	
b. Erosion or Sloughing	Yes (X) No () - Localized, shallow erosion rills/gullies need to be repaired (filled in with stone/rip-rap).	18	
3b. South End - (No Io	nger applicable due to Gypsum Pond/berm construction)		
a. Condition	N/A - Recently constructed gypsum storage pond dike now abuts the south end of the separator dike.	19	
VI- Potable Water P	<u>ond</u>	Potable Water Pond Elev. 806'	
	Observations - Comments	Photograph No.	
1. Upstream Dike Slope	e (Potable Water)		
a. Condition	Rip-rap on upstream face looks good. Small trees and bushes at toe need to be cut down to maintain integrity of embankment toe.	20	
b. Erosion or Sloughing	Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching to reestablish vegetative growth.	21	
c. Concrete Drainage Ditch	Concrete in good condition. No obstructions in channel observed.	n/a	
2. Crest			
a. Condition	Gravel surfaced - looks good. No distress or potholes in road surface observed.	n/a	
3. Downstream Dike SI	ope (Storage Pond)		
a. Condition	Grass covered - Overall - looks good. Localized, minor surface erosion. Re-seeding and mulching to reestablish vegetative growth.	n/a	
b. Visible Seepage or Wet Spots	Yes () No (X) - No seepage or wet spots observed on slope.	n/a	
c. Erosion or Sloughing	Yes (X) No () - Localized, minor surface erosion. Re-seeding and mulching to reestablish vegetative growth.	n/a	
d. Concrete Drainage Ditch	Yes () No (X) - Condition of concrete satisfactory, and no further erosion noticed & are stable at this time.	n/a	
4. Spillway Approach C	hannel		
a. Condition - General	Small bushes/trees to be removed to prevent restriction of flow in channel.	22	
b. Condition - Rip-Rap	Good. No evidence of instability.	n/a	
c. Condition - Concrete	Good.	n/a	
5. Spillway Structure - A			
a. Condition	Concrete in good condition.	n/a	
6 . Spillway Structure -			
	Concrete in good condition	n/a	
7 . Spillway Structure -			
a. Condition	Concrete - Good	n/a	

Plant Wansley 2008 - 3rd Quarter Inspection Summary

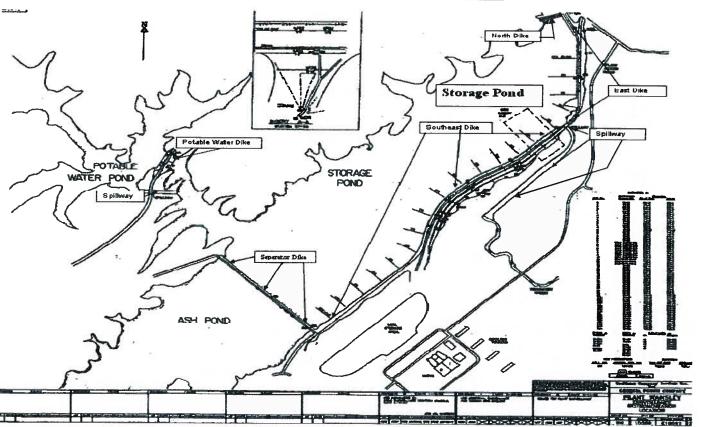
VII - Detention Pond			
	Observations- Comments	Photograph No.	
1. Upstream Dike Slope			
a. Condition	Rip-rap in good condition. Pond being dredged during visit. Pond level below normal level.	23	
b. Erosion or Sloughing	Yes () No (X) - Slope looks satisfactory, no visible instability observed	n/a	
2. Crest		<u> </u>	
a. Condition	Gravel surfaced and in good condition. No distress or potholes observed.	n/a	
3. Downstream Dike SI	ope_		
a. Condition	Well grass-covered. Needs cutting but this did not prevent good visual examination. Small bushes/trees need to be removed. No visible evidence or instability observed	24	
b. Visible Seepage or Wet Spots	Yes () No (X)	n/a	
c. Erosion or Sloughing	Yes () No (X)	n/a	
4. Concrete Spillway C	hannel_		
a. Concrete Condition	Concrete is in good condition	n/a	
5.Spillway Outlet Chan	nel		
a. Condition	Rip-rap at outfall and outlet channel is in good condition. No issues observed.	25	

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Hugh H Armitage St Engineer

Hugh H. Armitage - Sr. Engineer SCG - Hydro Services

Location Plan
Storage Pond and Potable Water Pond



2008 - 3rd Quarter Inspection Photographs - July 22, 2008

(See accompanying report attached)

Photo No.	Description	
1	Storage Pond - N Dike - Near Rec. Center Entrance - Warning sign needs to be re-mounted.	



Storage Pond - N. Dike -Localized bare spots in grass cover. Re-establish grass to prevent erosion.



Storage Pond - E. Dike - Downstream slope in good condition



Storage Pond - E. Dike - D/S slope - Toe Ditch - Localized vegetation and debris needs removal so that flow isn't restricted. (Sta.1+60 to 3+50).



2008 - 3rd Quarter Inspection Photographs - July 22, 2008

(See accompanying report attached)

Photo No.	Description	
5	Storage Pond - East Dike - Concrete lined toe ditches - Vegetation/Debris needs to be cleaned out of ditch (Approx Sta. 5+00 to 23+50)	



'Storage Pond - East Dike - Bare spots on slope to be re-seeded to prevent further erosion. Sta 11+20



Storage Pond - East Dike - Emergency Stockpiles of coarse and fine aggregate



Storage Pond -Spillway - Vegetation downstream of end of spillway needs to be cleared.



2008 - 3rd Quarter Inspection Photographs - July 22, 2008

(See accompanying report attached)

Photo No.	Description
9	Storage Pond - Southeast Dike - Upstream Slope - Riprap varies in size from gravel size to 18"-24", occasiona 24" to 36" nominal size. Per GA Safe Dams letter of 4-308, this will be addressed by SCG Hydro Services.



Storage Pond - Southeast Dike - South end near Separator Dike. Bushes and trees need to be removed at toe of slope.



Storage Pond - Southeast Dike - Downstream (D/S) - Upper Slope looks satisfactory



Storage Pond - Southeast Dike- D/S -Silt and debris removal required at outlet of Drain at Sta. 73+00 and 74+00



2008 - 3rd Quarter Inspection Photographs - July 22, 2008 (See accompanying report attached)

Photo No.	Description
42	Storage Pond - Southeast Dike D/S - Previous

rodent hole has been filled in.

13

15

16



Storage Pond - Southeast Dike D/S - Rodent Holes on lower slope at approx. Sta 67+60 (near headwall) and Sta 68+40 need to be filled



Storage Pond - Southeast Dike D/S - Sta. 37+50 - Ground surface was damp/wet. A localized area was observed where minor scarp indicating surface movement may have occurred. This area to be monitored routinely by plant for evident of on-going movement.



Storage Pond - Southeast Dike D/S - Localized bare spots on lower slope that need to be reseeded/mulched to prevent further erosion.



2008 - 3rd Quarter Inspection Photographs - July 22, 2008

(See accompanying report attached)

Photo No.	llecrintion	
17	Storage/Ash Pond - Separator Dike - Upstream Slope. No evidence of instabilty or distress. Minor erosion rills/gullies need to be filled in.	



Storage/Ash Pond - Separator Dike - Downstream
Slope - Localized erosion rills/gullies need to be repaired to prevent further erosion.



Storage/Ash Pond - Separator Dike - South End.
Gypsum Storage pond berm on right side of photo.



Cut bushes/trees

Potable Water Pond- Upstream Slope. Small trees/bushes at toe need to be removed.



2008 - 3rd Quarter Inspection Photographs - July 22, 2008

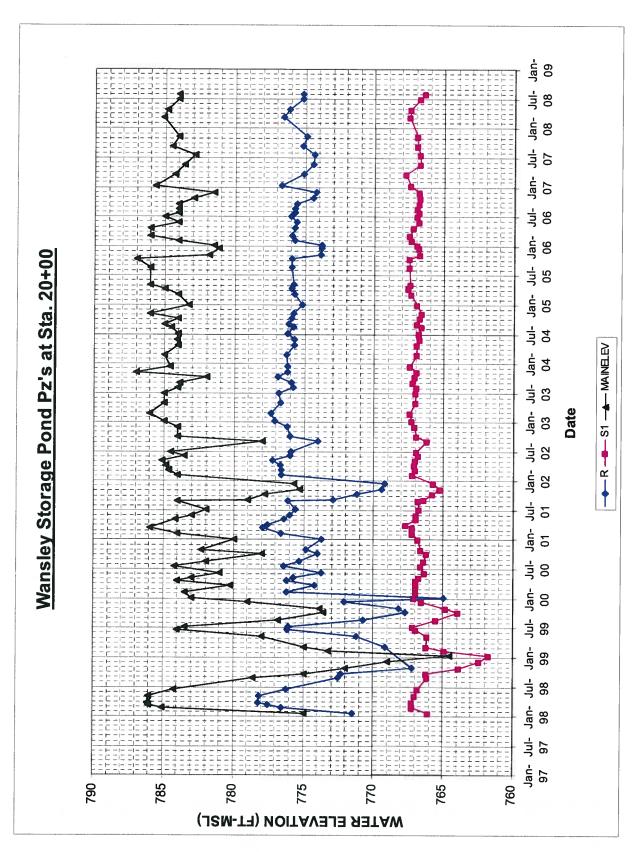
	(See accompanying report	attacheuj
Photo No.	Description	
21	Potable Water Pond- Upstream Slope. Localized bare spots on slope. Re-establish vegetation to prevent further erosion	
		Bare spots on Slope
22	Potable Water Pond-Spillway Approach Channel-Remove small trees/bushed within channel.	
		Bushes and trees
23	Detention Pond - Upstream Slope - Rip-rap in good condition, no visible evidence of instabilty.	
24	Detention Pond - Downstream Slope - Good grass cover, but needs to be cut. Small trees/bushes on slope need to be cut down.	
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2008 - 3rd Quarter Inspection Photographs - July 22, 2008

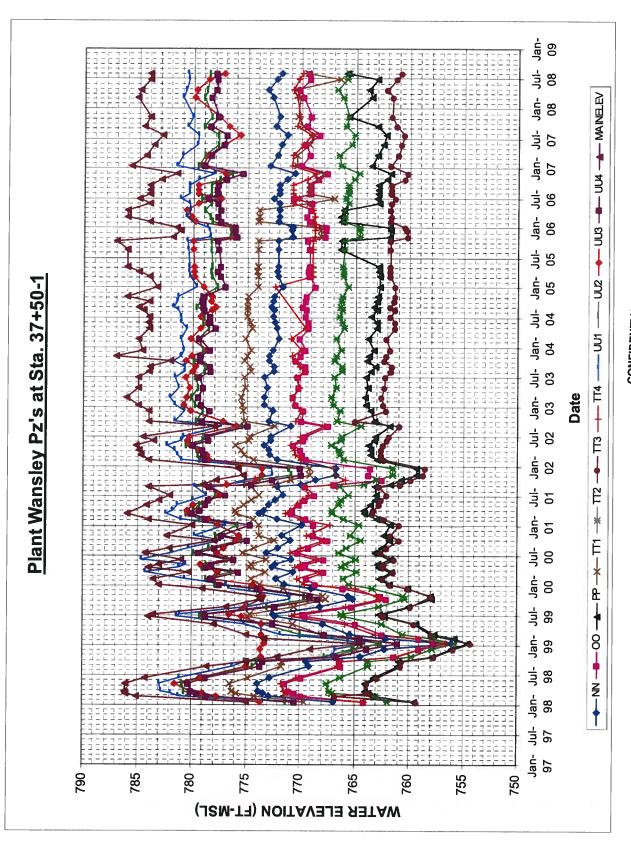
(See accompanying report attached)

Photo No.	Description
25	Detention Pond - Outfall Channel - Rip-rap and channel are in good condition. No erosion or instability observed.

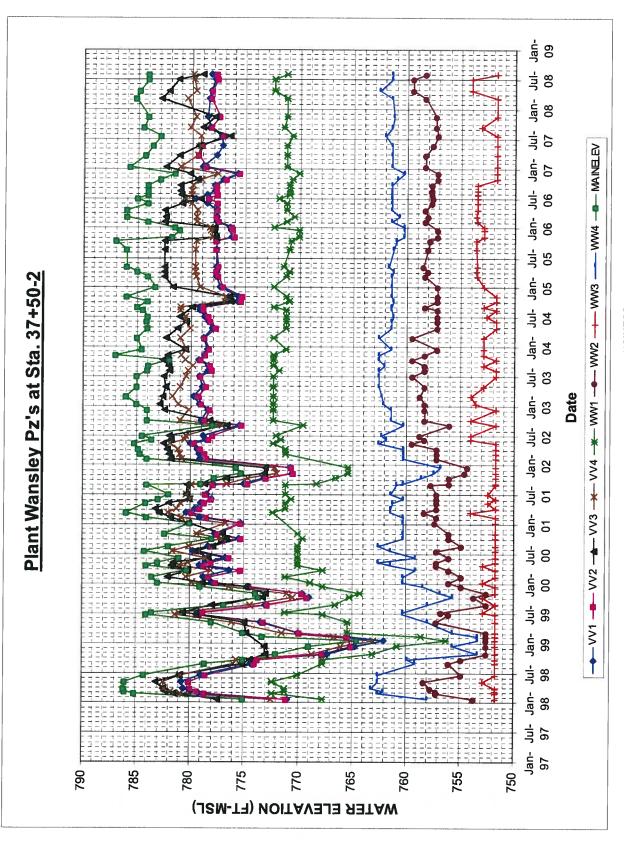




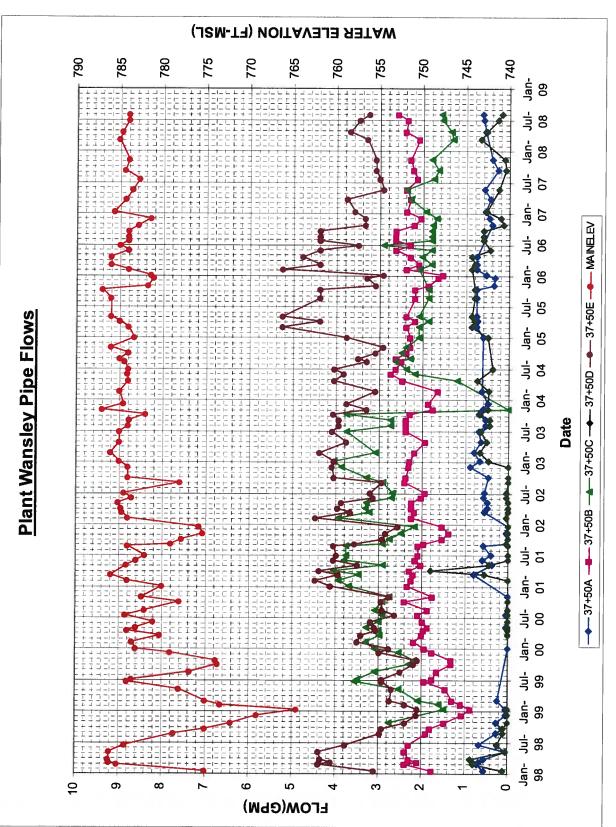
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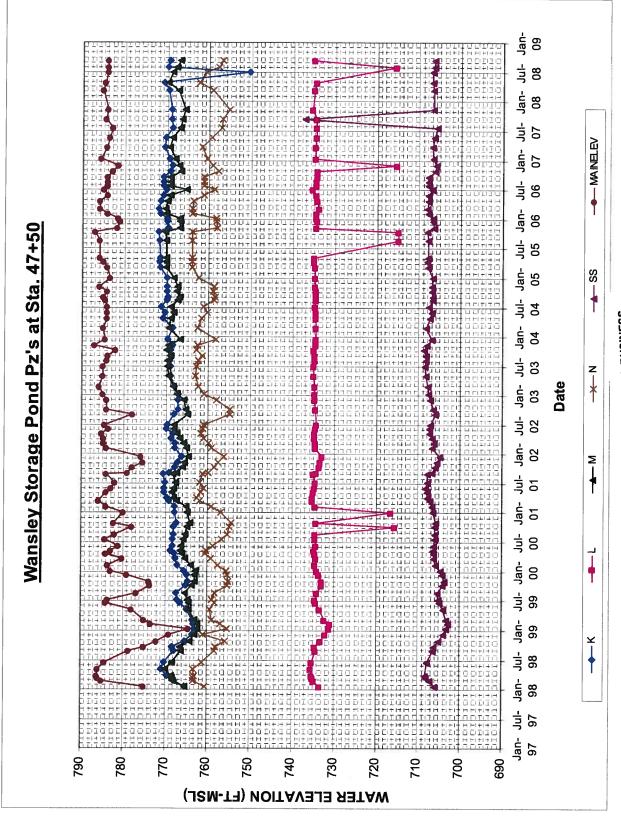
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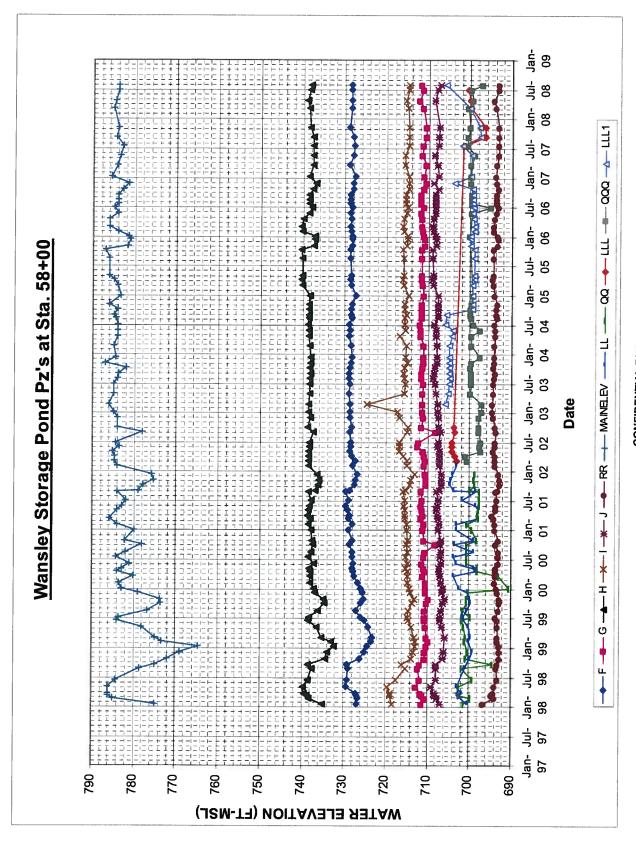
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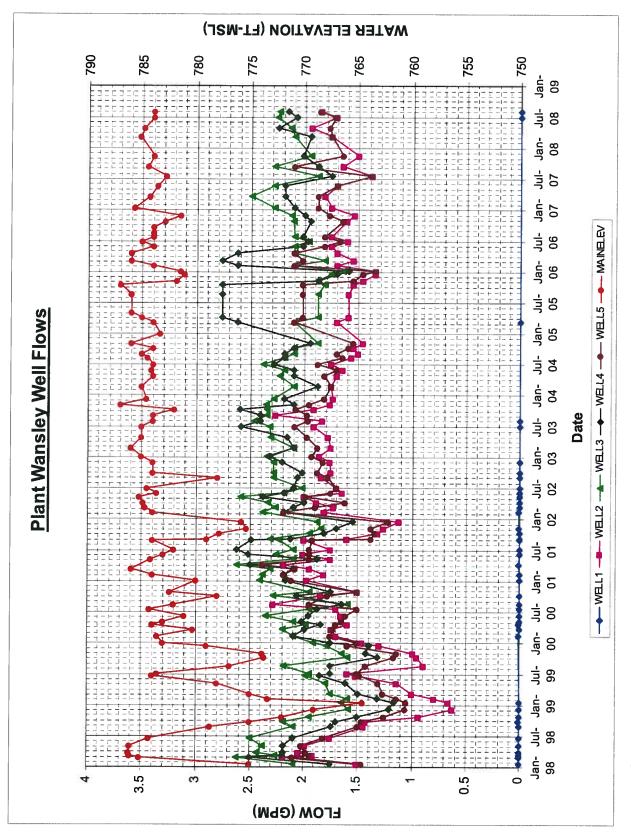
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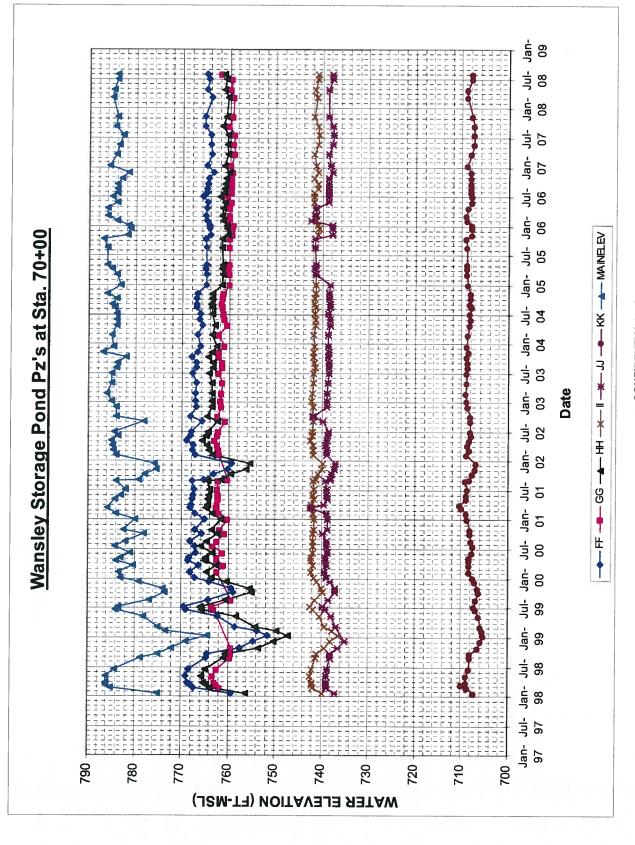
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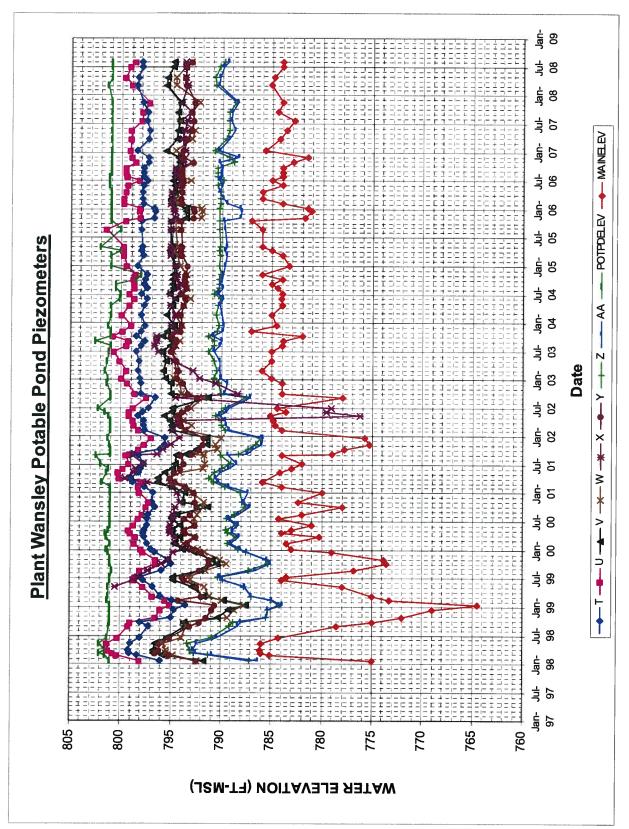


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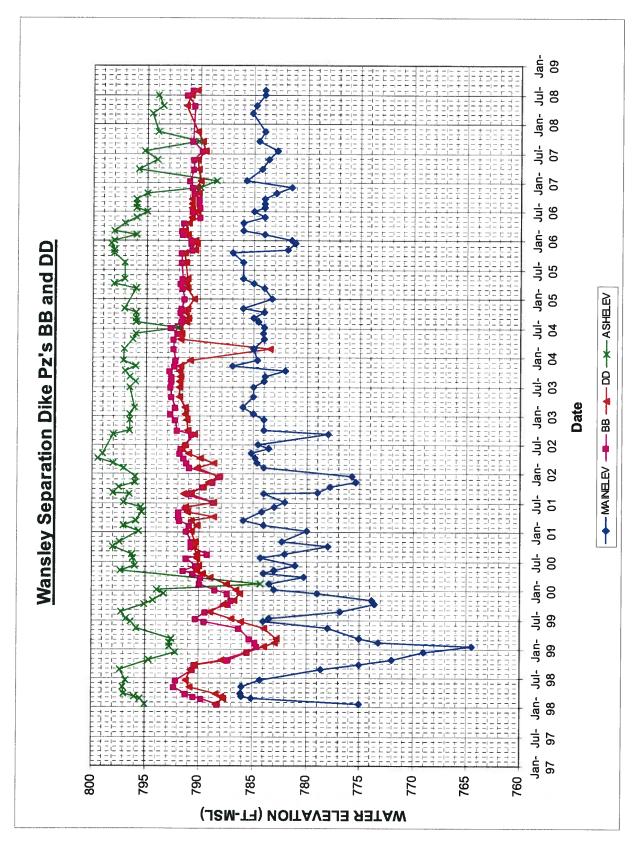


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Southern Company Generation Bin 10193 241 Ralph McGill Boulevard NE Atlanta, Georgia 30308-3374

Tel 404.506.7033



May 28, 2008

Plant Wansley

Dam Safety Surveillance Quarterly Report REA No. WN-08900

Mr. J. P. Heilbron Plant Manager Georgia Power Co. Plant Wansley

Dear Mr. Heilbron:

Attached is the first 2008 quarterly report on Dam Safety Surveillance for Plant Wansley, prepared by the SCG Hydro Services Group. The report includes observations of site conditions made during our inspection, plots of surveillance data, and our interpretation of this data. The inspection of the Storage Pond Main Dike was carried out on March 10, 2008 in conjunction with the Georgia Department of Natural Resources, Environmental Protection Division, Safe Dams Program (SDP) annual inspection.

No major dam safety issues that would immediately impact the safety of the structures were discovered during this inspection. However, there are a number of maintenance items that should be addressed soon to assure the continued safety and performance of the structures. These items are identified in the report.

Should you have any questions, please do not hesitate to contact me at extension 8-506-7033.

Sincerely,

Joel Galt

Hydro Services Supervisor

Joel Falx

WAN-API 054

Attachments

CONFIDENTIAL BUSINESS INFORMATION

/hha

Georgia Power Company xc:

T. E. Wilson (w/ attachment) (w/ attachment) N. I. Dean

Southern Company Services
E. B. Allison (w/ attachme (w/ attachment) J. H. Crisler (w/ attachment) (w/ attachment) F. J. Pryor

Hydro Service Wansley Notebook

Master File: WN-08900

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INSTRUMENTATION

Following are observations from the review of the dam safety instrumentation at Plant Wansley.

Storage Pond:

Sta. 20+00: Piezometers are within their historic range. Both seem to track the pond level.

Sta 37+50: All of these piezometers appear to be in their historic range. All seem to track pond level.

Sta 37+50 Pipe Flows: These flows appear to be in their historic ranges. The flow at 37+50C seems to be in a recent declining phase, and is at its lowest flow rate in the past four years. These flow rates will continue to be monitored to assess if any trends develop.

Sta 47+50: The relatively high reading measured in September 2007 on SS, appears to have been an anomaly since subsequent readings up to April 2008 have been within the historic range of past measurements. The other piezometers are within their historic range of measurement.

Sta 58+00: LLL1 and LLL have increased slightly since the 3rd quarter 2007 report and are generally within their historic range as is the case with the other piezometers.

Relief Wells: The relief wells appear to be discharging in their historic range. The flow appears to vary with the pond elevation with the exception of well 1.

Sta 65+00: Measurement of the piezometric level at E has resumed since the 3rd Quarter report. C, E and M are within their historic ranges.

Sta 70+00: All of these piezometers are reading in their historic ranges.

Weir and Pipe Flows: With the recent exception of the measurements weir at 49+00 and 11+20 (March/April 2008), both weirs appeared to be flowing in their historic ranges with the exception of the past 2 months During the inspection it was noted that the weirs at Sta. 62+00 and 57+00 were silted up. These weirs should be cleaned out and allowed to stabilize before each reading. Reading them when they are silted up produces bad data.

Separation Dike:

These piezometers are registering in their historic ranges. They exhibit a muted relationship with the storage pond elevation but little relationship to the ash pond elevation.

Potable Water Pond Dike:

These piezometers were registering in their historic ranges.

INSPECTIONS

On March 10, 2008, Joel Galt and Hugh Armitage of Southern Company Generation (SCG) Hydro Services conducted a dam safety inspection of the dikes at Plant Wansley. The inspection of the Storage Pond Main Dike was carried out in conjunction with the Georgia Department of Natural Resources, Environmental Protection Division, Safe Dams Program (SDP) annual inspection. Mr. Terry Wilson of Georgia Power Company accompanied SCG and SDP personnel on the inspection of the Storage Pond Main Dike. The weather was clear and warm. Approximately 2 to 3 inches of rain had fallen in the area the week preceding the inspection, but conditions were generally dry on the day of the inspection.

No major dam safety issues that would immediately impact the safety of the structures were discovered during this inspection. There are a number of maintenance items that should be addressed soon to assure the continued safety and performance of the structures. If not addressed in a timely manner, these things can become more costly to fix. The actions suggested are in **bold text**. These issues can be addressed as time and resources permit, but should be addressed within the next three months.

The site personnel that have the best opportunity to thoroughly inspect the dikes are the mowing crew. During the next mowing of the dikes, it would be a good idea to have the mowing crew flag any ant mounds and animal burrows that they come across. Then the mounds could be poisoned and the burrows filled in. However, safety considerations should be foremost.

The upstream slopes of the main dike, the east dike and separator dikes were examined from a boat. The downstream slopes and the remainder of the upstream slopes at the other ponds were inspected by traversing along the downstream slope and/or making observations from the crest of the dikes.

During this inspection at the Main Storage Dam, we made use of a GPS unit to capture the locations, using waypoints, of items described in the text. The waypoints are indicated by red flags and numbers on an aerial photo of the subject structure. The paragraph describing this location begins with the waypoint number. In some cases, these flags and numbers have tended to overlap, obscuring some of the numbers. Some waypoints were taken for reference, so that all flags on the aerial photos do not necessarily indicate the location of an item described in the text. With experience, we hope that the method can be refined and will result in a report that is more valuable to the user.

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Storage Pond - Main Dike

The text for this segment is arranged as if the reader is moving from southwest to northeast along the main storage dike and then to the spillway and then back along the toe of the slope towards the southwest.

LOWER MAIN DIKE



General Comment – Remove all accumulated vegetation within concrete lined channels



Pt. 130 – Cracks in concrete lined channel. Cracks like this should be cleaned out of dirt and caulked (using a appropriate caulking material). This will prevent water infiltration and progressive undermining beneath the concrete,



Pt. 131 – Sta. 77+00 – Localized build-up of grass, silt and debris. Requires clean out of the outlet end of the finger drain.



Sta. 76+60 – Animal Burrow needs to be filled in.



Pt. 132 – Sta. 76+00 - Localized build-up of grass, silt and debris. Requires clean out of the outlet end of the finger drain.



CONFIDENTIAL BUSINESS INFORMATION

Page 5 of 20

Sta. 75+00 – Localized build-up of grass, silt and debris. Requires clean out of the outlet end of the toe drain.



Sta. 73+00 to 74+00 – Bare spots on slope. Re-seed and mulch areas to re-establish grass cover to mitigate surface erosion.



Pt. 133 (Approx. 75+50) Pt. 134 (Sta. 73+00) Pt. 135, 136 (pictured at right) and 137 (~Sta 66+67)

Animal burrows need to be filled in.



Sta. 72+50 and 70+00 (pictured) and 66+00 – End of toe and finger drains damaged by mowers. Repair ends of drains (i.e. add new section of pipe or beat back into shape). Provide markers to prevent re-occurrence



Pt. 138 (Sta 62+00) – Silted up behind weir. Clean out dirt from around weir



Pt. 139 (~Sta. 62+50) – Bare spots on slope. Re-seed and mulch areas to re-establish grass cover to mitigate surface erosion.



UPPER MAIN DIKE



Pt. 140 (~Sta. 57+00) – Build-up of silt at weir. Clean out dirt around weir.



Pt 141 (~Sta 53+00) – Animal burrow needs to be filled in.



Pt. 142 (~Sta. 45+00) – Three animal burrows at headwall need to be filled in.



Pt. 143 (approx Sta. 40+00) – Rip-Rap on upstream slope of main dike.



CONFIDENTIAL BUSINESS INFORMATION

Page 8 of 20

Pt 144 - Rip-Rap on upstream slope of separator dike.



STORAGE POND - EAST DIKE



Pt. 145 - Sta. 1+00 to 8+00 - Clear debrisfrom finger drains at Sta. 1+00 & Sta. 7+00 and clean out bottom of concrete lined channel of silt/sand and miscellaneous.

(Note: Pt. 146 - not shown on plan - same comment as Pt. 147 below)



Pt. 147 – Minor seepage into concrete channel (~ Sta. 12+50).



Pt. 148 (~Sta. 15+00) Finger drain dry, but localized seepage into channel. Localized grass in channel needs to be removed.



Pt. 149 – (Sta 19+00) – Finger drain blocked at this location and needs to be cleaned out.



Pt. 150 – Build-up of grass and debris in channel, (restriction of flow through channel). **Debris/grass needs to be cleaned out of channel**



Storage Pond Main Dike/Spillway

Pt. 151 – Localized minor vegetation observed at floor of spillway. Remove vegetation from weephole and other various locations in the spillway floor.



Sta. 37+50 – Progressive undermining of concrete paving. Requires repair to mitigate further ground loss and future damage to pavement. (See attached Sketch No. 1 below for proposed repair option)



Pt 152 (Sta. 68+00) —Bare spots in slope Localized re-seeding to re-establish grass cover.



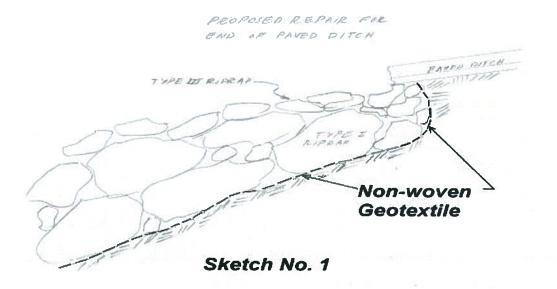
Pt 153 – Animal burrow just upslope from concrete drainage channel (approx Sta. 68+00). Requires repair/filled in.



Page 11 of 20

Pt 154 – Stockpiles of emergency coarse and fine aggregate drainage materials looks good





Separator Dike

Upstream slope generally looks satisfactory although surface runoff is causing localized erosion near top of slope.

Localized re-grading required to flatten over steepened upper portion of slope and to provide positive drainage to minimize over-slope flows (which will result in progressive erosion along slope)



Page 12 of 20

Crest of Upstream Slope - Localized erosion rill needs to be repaired with #57 stone.



Potable Water Pond Dike

Localized bare spots on slope. Repair either by re-seeding to re-establish grass cover or place small rip-rap/stone).

(See Updated Status from Third Quarter Report –Location 092, for items to be completed)



Vegetation growth at toe of upstream slope. Cut down and remove vegetation (grass/bushes)

(See Updated Status from Third Quarter Report – Location 089, for items to be completed).



Ash Pond Emergency Overflow

Repair of downstream outfall completed

(See Updated Status from Third Quarter Report – 094).



Rip-rap on upstream and downstream slopes overall looks good. No evidence of recent erosion or instability. Brush has been cleared since 3rd Quarter 2007.

(See Updated Status from Third Quarter Report – Location 095, for items to be completed).



Detention Pond

Vegetation on downstream slope.

Remove/cut down

vegetation/brush/small trees for a

distance of 30 ft. out from toe of slope.

(See Updated Status from Third Quarter Report – Location 099, for items to be completed).



Tree and brush adjacent to spillway should be cleared.

(See Updated Status from Third Quarter Report – Location 100, for items to be completed).



<u>UPDATED STATUS (Reference - 3rd Quarter Report dated October 27, 2007)</u>

073 The cleanout pipes for the subsurface drain at Sta 69+00 do not have covers on them. These pipes should be covered.
3-10-08 - Pipes capped



075 The stockpile of granular material west of the makeup water pump station appears to have gotten smaller than it should be. It would be a good idea to add one load of sand, one load of #89 stone and two loads of #57 stone.

3-10-08 - No Change



062 At the paved ditch at the toe of the maximum section of the storage pond dike, the slopes of the ditch have become overgrown with tall weeds and woody brush. **This vegetation should be cleared or poisoned.**

3-10-08 - Brush cleared



062 The brush growing up between the steps has also made walking on the stairs to the relief well flow measurement platform hazardous. This brush should be removed. The weir in the paved ditch at this location is clogged with silt and should be cleaned out.

3-10-08 - Brush cleared



066 An animal burrow was noted next to the paved ditch at Sta 45+00. There is some woody brush in this area that should also be cut. Brush and tall grass provide cover for burrowing animals and therefore encourage their burrowing. The animal should be trapped or gassed and the burrow should be packed with gravel and/or foamed. The brush should be cleared.



3-10-08 - Brush cleared but burrow still observed

068 The toe of the dike at Sta 37+50 was very boggy as usual. In May, Hydro Services had Dr. Rene Rodriguez conduct geophysical testing of this area to try to determine the flow path of the water. Dr. Rodriguez' results and a drawing of the finger drain system correlate to indicate that the source of the water may be a buried finger drain with no outlet to the surface. Hydro Services will work with plant management to plan an exploration of this area with the aim of locating the buried drain and installing a pipe to provide relief to this groundwater.

069 The weir at Sta 37+50 is silted up and should be cleared so that it can be read properly. 3-10-08 – Weir cleared of silt

070 The emergency granular material stockpiles at Sta 37+50 are overgrown with weeds and brush. This vegetation might hinder access to the materials in an emergency. The stockpiles should be sprayed to kill the weeds.





077 The stoplogs for the spillway are stored on the dike adjacent to the spillway sitting on some timbers. The skin side of the stoplogs is down, allowing rainwater to stand in the stoplog. This causes corrosion and shortens the life of the stoplogs. It would be a good idea to examine the stoplogs for loss of section due to corrosion, repair and paint them if necessary, and store them skin side up to reduce the potential for corrosion.

3-10-08 - Unchanged



Page 16 of 20

Storage Pond - East Dike

081 There are some small trees and woody brush growing at the heel (upstream) of the east dike at this point. This vegetation should be cut or poisoned. (about Sta 7+00?)

3-10-08 - Vegetation cleared



082 Tall grass and woody brush are growing on both sides of the paved ditch at the toe of the east dike from Sta 5+00 to Sta 20+00. **This vegetation should be cut down.**

3-10-08 - Vegetation cleared



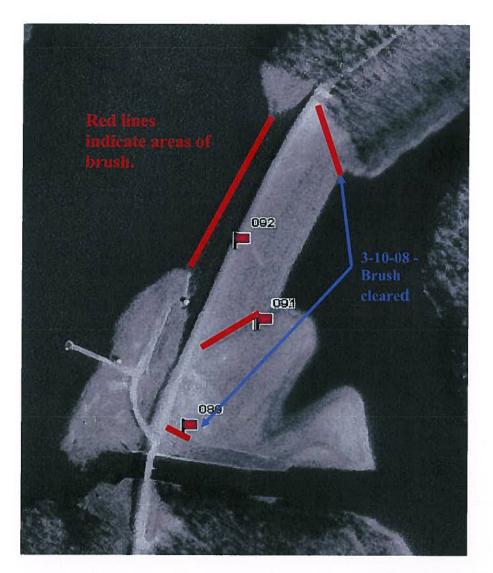
083 The granular material stockpiles downstream of the east dike have become overgrown with weeds. The sand will not be useable due to roots. The stockpile should be sprayed with poison and a load of sand added. (about Sta 10+00)

3-10-08 - Vegetation removed and stone added to stockpile



Potable Water Pond Dike

3-10-08 - Brush cleared downstream of the dike but brush needs to be cleared on upstream toe of slope



089 An eroded area was noted on the downstream slope just east of the spillway. This appears to be the result of runoff from the roadway passing through the riprap and running down this area instead of into the adjacent paved ditch. It would be a good idea to add some rock to the eroded area to protect it from further erosion and to add some rock to the riprap at the top of the slope to try to direct the runoff into the existing paved ditch. There are also some small trees growing in this area. These should be cut or poisoned.

3-10-08 — Trees and bushes removed and gravel placed in rip rap. Some additional rock required to cover bare spots



091 On the downstream side of the dike there is a growth of tall grass and woody brush along the east and west contacts of the dike with the natural ground. This vegetation should be cut or poisoned. 3-10-08 – Brush cleared at both locations



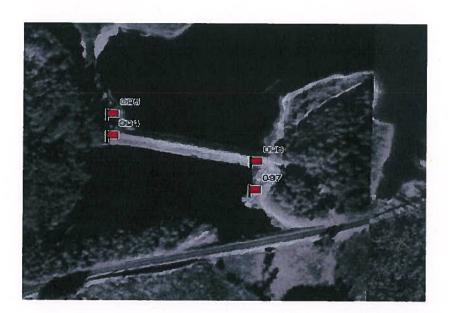


092 The upstream slope of the dike is rip rapped. There is a fair amount of grass growing in the riprap. This grass should be poisoned. There is a stand of brush along the upstream edge of the dike. This brush should be removed.

3-10-08 - No work yet.



Ash Pond Emergency Overflow



094 The downstream outfall for the paved ditch has been undermined. Storm flows from the ditch undermine the ditch liner and will eventually remove the soil supporting the end of the ditch liner causing the liner to fail. This area should be reinforced by placing Type I riprap in the pond at the outfall and choking that with Type III riprap. See sketch below.

3-10-08 - Work completed

095 The upstream spillway of the paved ditch has been repaired with Type III riprap, but the riprap needs to be dressed to match the contours of the paved ditch. The current layout could result in uneven flows over the riprap and subsequent erosion 3-10-08 - No change



096 There is woody brush growing on the east end of the dike on the downstream side. This brush should be cut or poisoned.
3-10-08 - Brush has been cut



There is a bare spot adjacent to paved ditch near the location noted above. This area should be armored with Type III riprap on a 4" bed of #57 stone. 3-10-08 - Still required



098 There is woody brush growing in various locations around the perimeter of the pond on the downstream side of the skimmer wall. This brush could make recovery efforts difficult in the event of an oil spill to the ash pond. This brush should be cut or



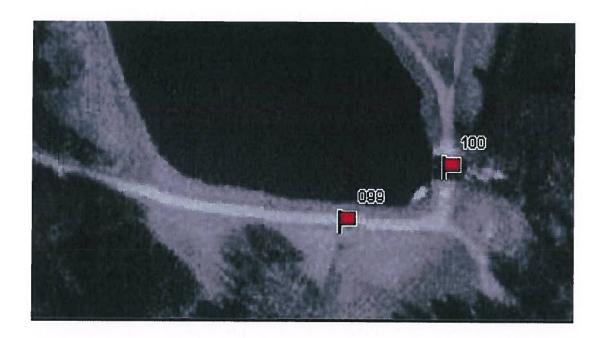


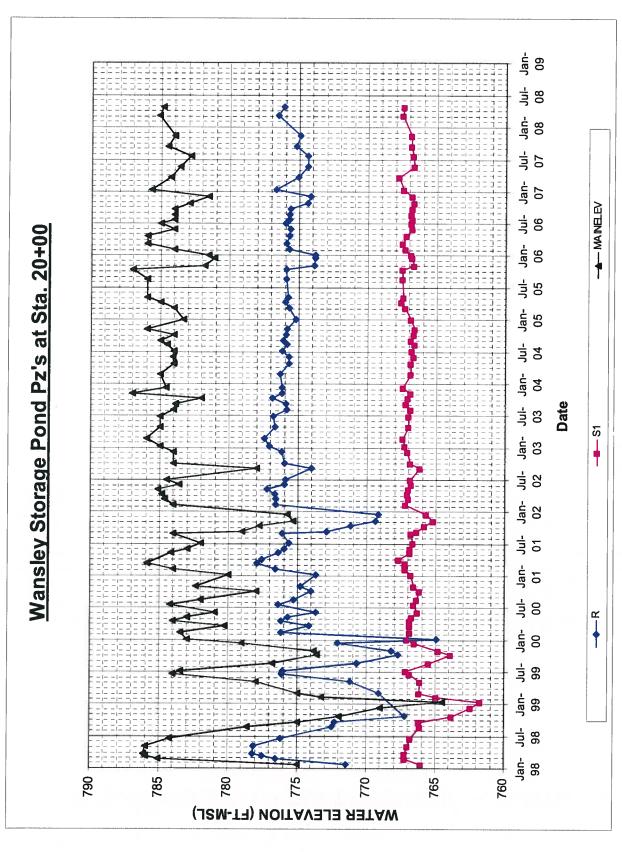


Detention Pond

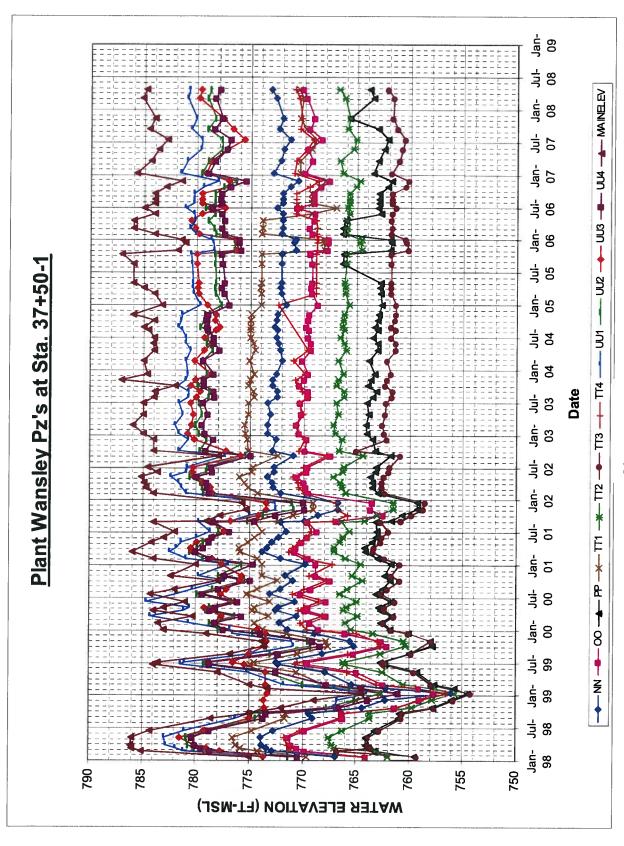
099 The tall grass and small trees growing on the downstream side of the dike should be cut for a distance of 30 feet out from the toe. 3-10-08 - No work yet.

100 The tree and brush growing adjacent to the edge of the spillway should be cleared. 3-10-08 - No work yet.

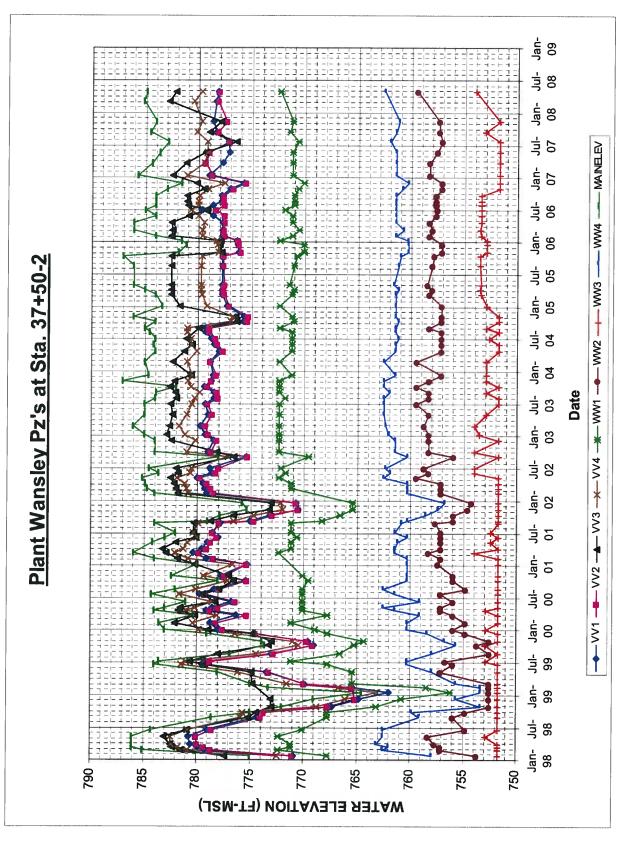




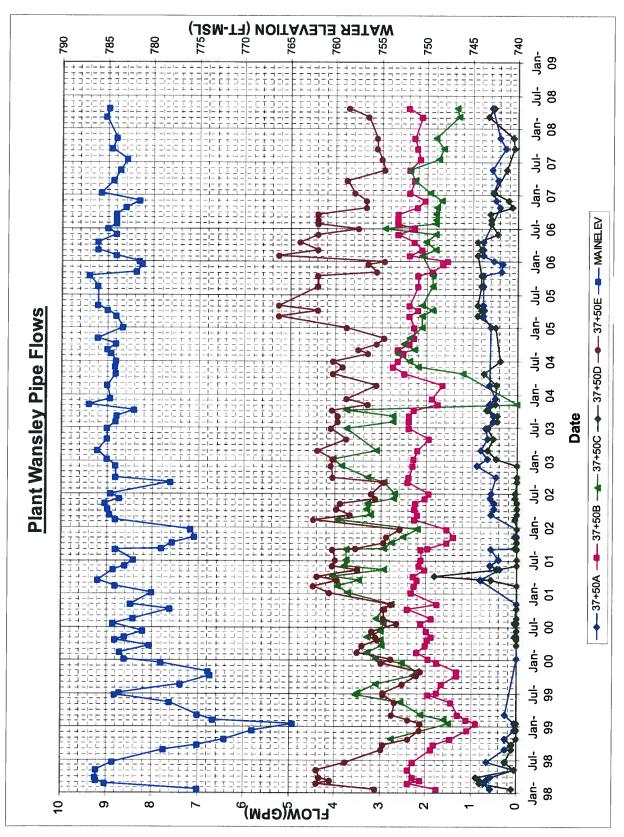
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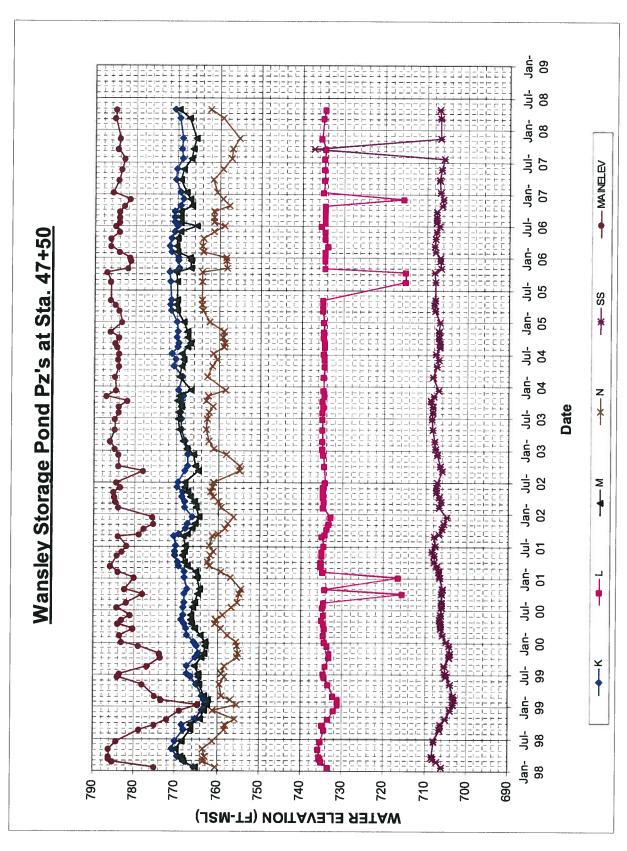
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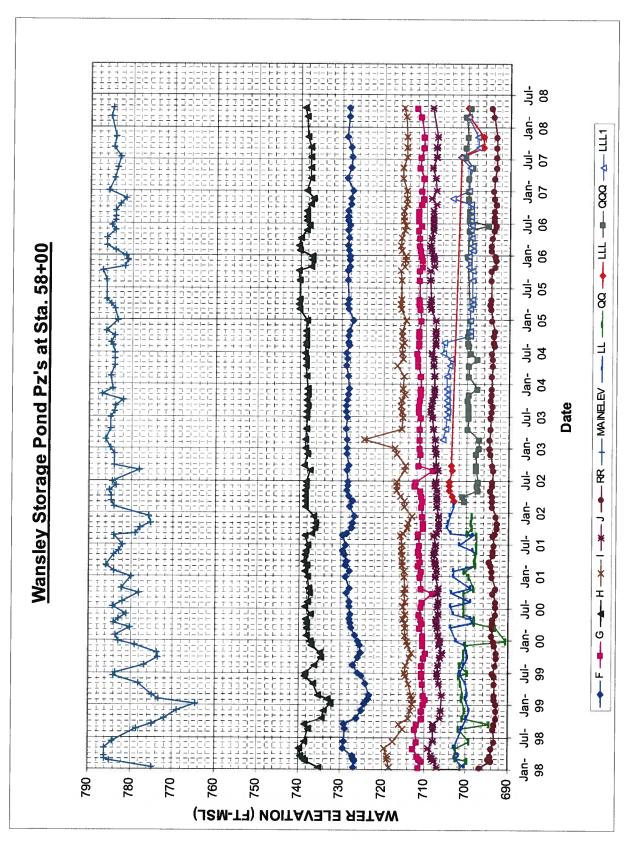
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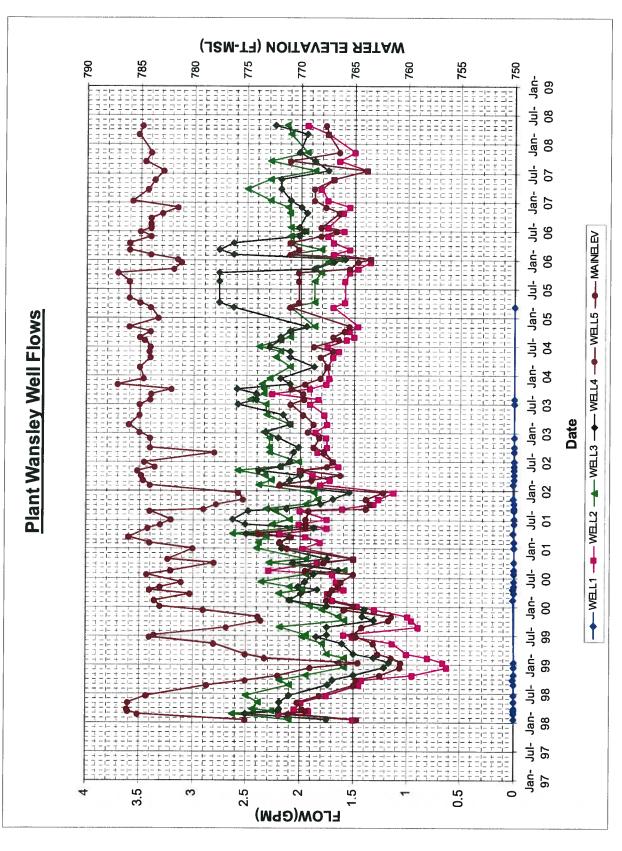
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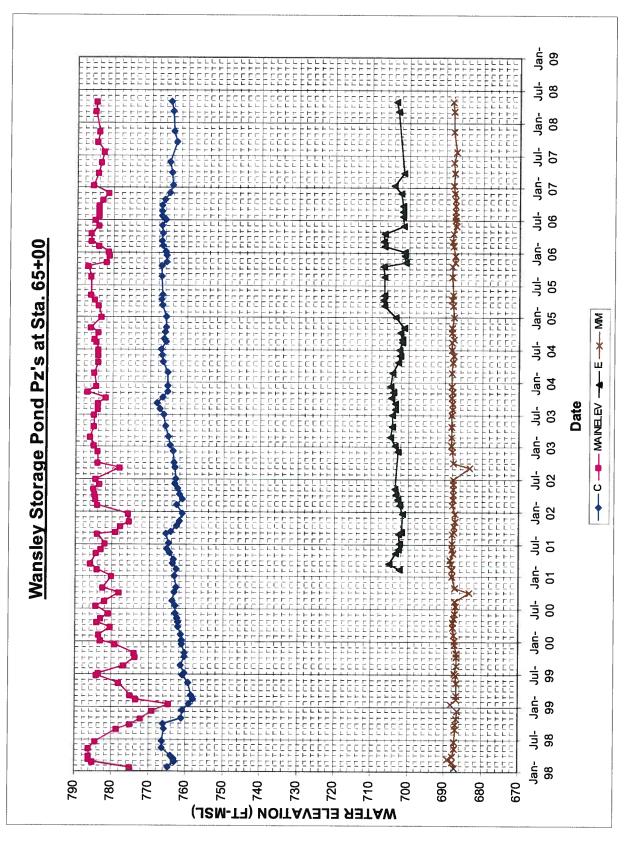
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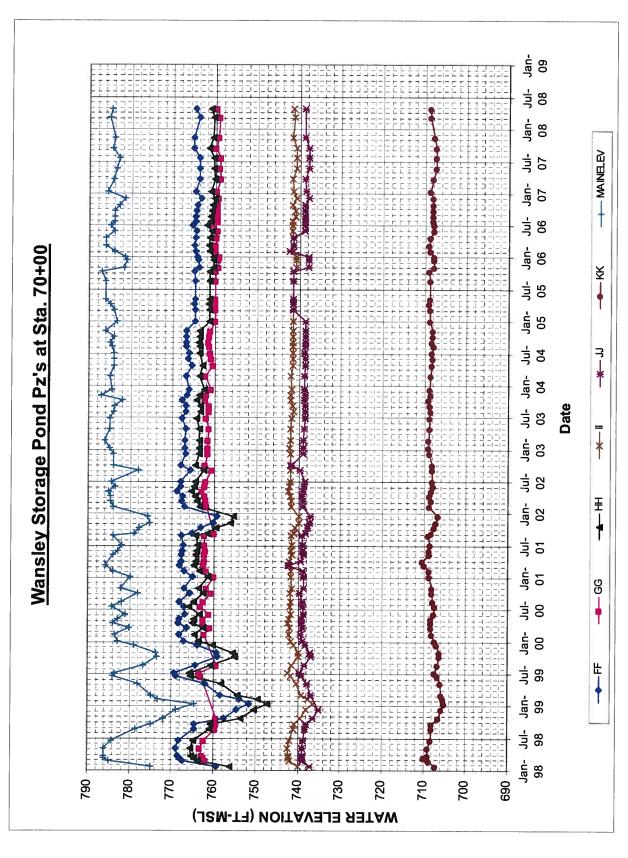
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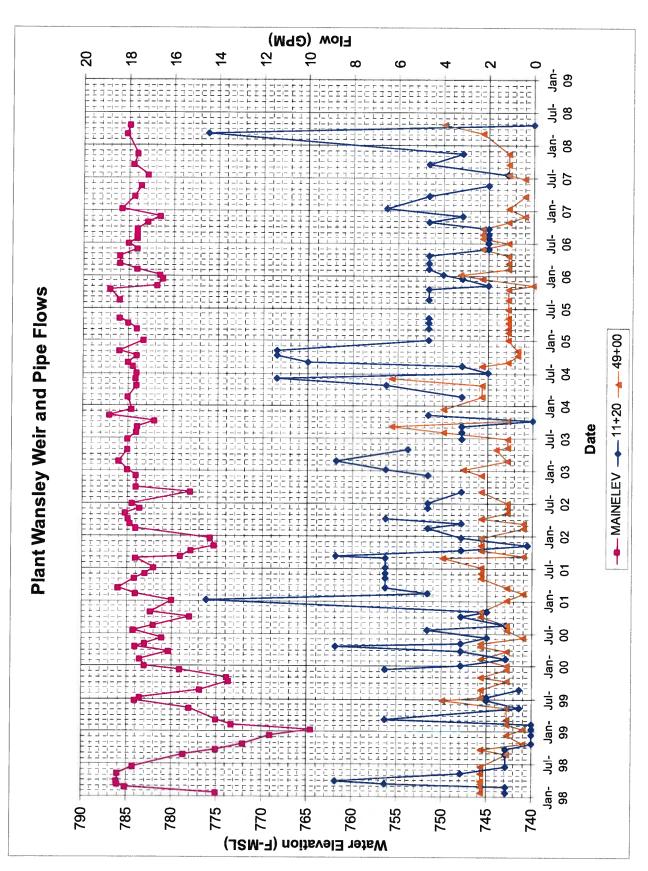
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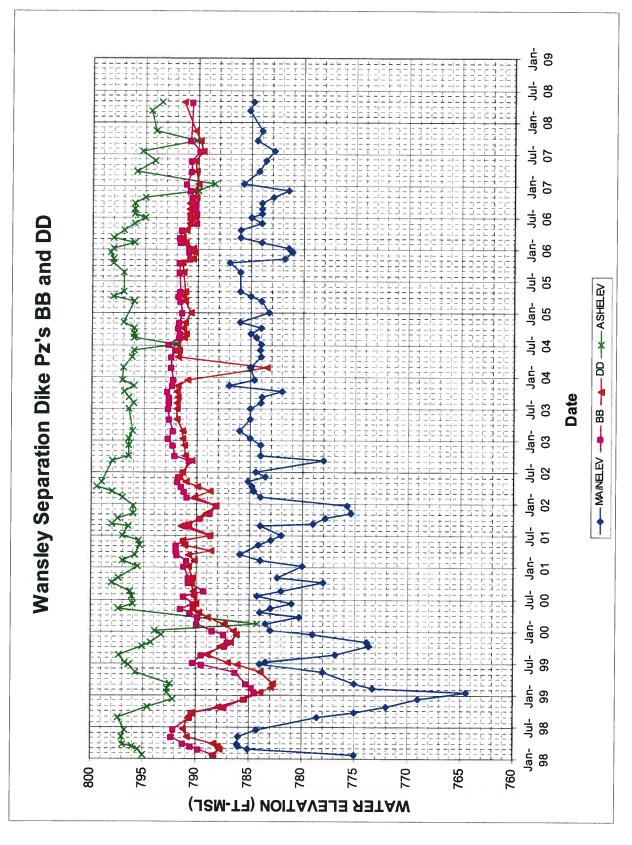
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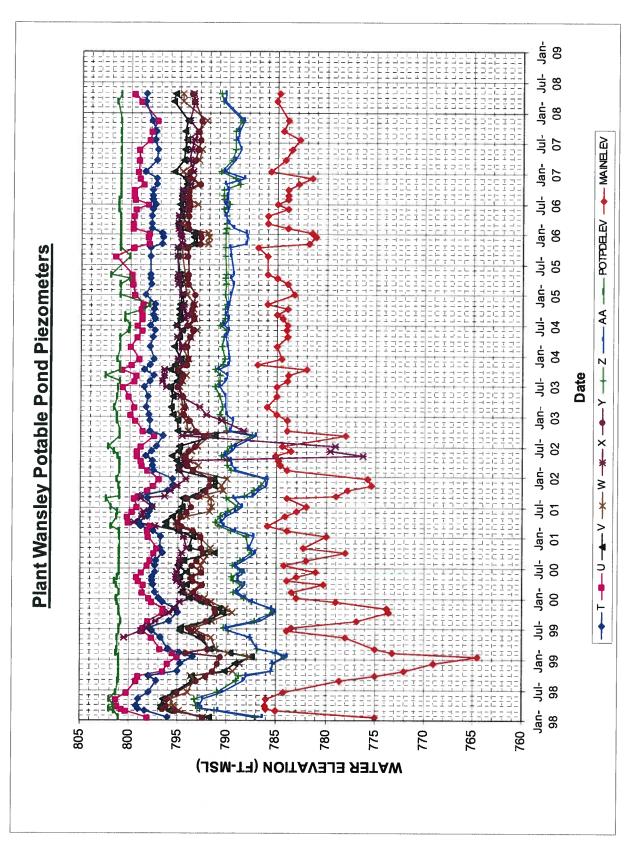
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PLANT WANSLEY

EARTH EMBANKMENTS

AND
APPENDICES

GEORGIA POWER COMPANY CIVIL ENGINEERING DEPARTMENT

MAY, 1975

WAN-API 035

TABLE OF CONTENTS

Introduction

Construction History

Construction Materials

Embankments

East Dike

Storage Pond Dike

Separation Dike

Potable Water Supply Dam

Summary

APPENDICES

VOLUME 1

Appendix I Compilation of Preliminary Reports and Investigations Procedures for Quality Control Appendix II Appendix III Designation of Quality Control Responsibility Appendix IV East Dike Daily Inspection Progress Reports Appendix V Storage Pond Dike, Separation Dam, Potable Water Supply Dam, Soils Reports VOLUME 2 Appendix A East Dike, Compaction Reports Appendix B Storage Pond Dike, Compaction Reports VOLUME 3 Separation Dike, Compaction Appendix C Reports Appendix D Potable Water Supply Dam, Compaction Reports

INTRODUCTION

The following narrative and attached appendices are in response to letter, Southern Services, dated February 27, 1975, requesting a compilation of design and construction data for the earth dams at Plant Wansley. Specifically, the letter requested information on the following:

- a. Dam construction history.
- b. Identification of borrow pits used for various portions of the dam.
- c. Field compaction control and results summary.
- d. Sources of riprap, filter material.
- e. Identification of all Georgia Power Company, Law Engineering and
 Contractor personnel who inspected or supervised the work with the
 dates of their involvement.

Detailed studies on all of the earth dam portions of the project are on file with Southern Services, and are listed by date and title in Appendix I. The plans and specifications for the project, including the as-built drawings, are also available. The narrative report will therefore be relatively brief, but will be supported by the specific reports and detailed studies made throughout the design and construction period either by reference or in appendices. The appendices also contain daily and weekly reports, and field control data, if reference to these details should ever be required.

The information in the report was assembled by T.B. Chaudhary in the form of a preliminary document. It was reviewed and revised in the Civil Engineering Department of Georgia Power Company. In accord with the items "a" through "e" above, the report initially discusses the general construction history, including the personnel involved in the construction, inspection and supervision of the dam and dikes. Subsequent sections describe in more detail the construction of the various dike or dam segments. The data on compaction control and test results for each segment is appended, as well as selected photographs taken during

earthwork construction. Special paragraphs on stone protection, which comprised an important part of the dike construction, are included.

CONSTRUCTION HISTORY

Field supervision of the project for Georgia Power Company throughout the construction period was under the direction of Mr. C.B. Head, Project Superintendent, and Mr. R.S. Ponsell, Civil Supervisor.

In early spring of 1972, Georgia Power Company selected Southland Constructors, Inc., as contractor for all grading work, including the dike and dam construction.

Mr. Simpson Faulkner, President of Southland, was in charge of the Southland work until the end of January 1975, when he was replaced by Mr. Bobby Slaughter.

The survey work was initially done by Riley-Park-Hayden, who were later replaced by a Georgia Power Company survey group.

Law Engineering Testing Company was selected to provide a quality control program to assure that the grading work was performed in accordance with the pellns and specifications, and to report any situation requiring special engineering attention. They assigned Mr. Michael R. Turner as Project Site Engineer to oversee all grading work. A team of soil technicians, including Law Engineering Testing Company and Georgia Power Company employees, was organized to observe and test the compaction of all fill work under the direction of Mr. Turner.

On May 1, 1972, the first compaction test strips were placed to evaluate the performance of the compaction equipment proposed to be used for the grading work. Law and Georgia Power Company's project construction group formulated procedures for Quality Control during fill placement for the dike construction (refer to Appendix #2). The procedure required that all phases of dike construction be witnessed by the civil group or their consultants.

On September 19, 1972, the contractor started stripping operations for the East Dike between Stations 0+00 and 22+00. Mr. Edward O. Prescott, Soils

Engineer, Law Engineering Testing Company, was assigned to the project on September 25, 1972, to assist Mr. Turner, and to be responsible for the East Dike construction inspection. Mr. Prescott remained at the site until December 5, 1972 when construction of the East Dike was suspended because of inclement weather. When the work resumed on February 20, 1973, Mr. Jay Budde, Soils Engineer, LETCo, assumed Mr. Prescott's responsibilities. Mr. Budde remained on the site until March 5, 1973.

On February 23, 1973, Mr. Beadles of LETCo met with Messrs. Ponsell and Edwards of Georgia Power. As a result of this meeting, Mr. Charles Edwards of Georgia Power was assigned to assume the responsibilities of earth construction inspection and testing. Mr. Turrner assisted Mr. Edwards until he became familiar with the inspection and testing operations. Mr. T.B. Chaudhary of LETCo was designated to provide consultation to Mr. Edwards on an as-requested basis. These changes of engineering and inspection responsibilities are summarized in letters of confirmation dated March 1 and March 15, 1973 (Appendix #3). Under Mr. Edwards' supervision, the Separation and Storage Pond Dikes were partially completed, and the major portion of preparation and cleaning of the Potable Water Dam abutments was done.

Mr. Edwards requested assistance during a part of the foundation preparation for the Separation Dike. Mr. Thomas L. Cross (from June 5 to June 29, 1973) and Mr. David Bourne (from July 2 to August 17, 1973), both of LETCo, assisted Mr. Edwards during this phase of inspection. Mr. Edwards resigned from the Georgia Power Company on February 21, 1974. Mr. T.B. Chaudhary assumed the engineering inspection responsibilities for the remaining earth work related construction.

During this assignment, the Separation Dike was completed. Yellowdirt Creek was diverted for Potable Water Supply Dam construction, and the Potable Water Supply Dam was completed. Mr. Chaudhary was re-assigned on April 18, 1975, and the last section of the Storage Pond Dike, between Stations 50 and 70, was completed under the supervision of Georgia Power personnel. CONSTRUCTION MATERIALS

Prior to and concurrent with construction, extensive explorations were conducted to identify and classify the soil and rock materials to be used for embankment construction. Reports for each of these detailed investigations are on file and are listed in Appendix I.

In brief, the site geology and soil profile are typical of much of the Southeastern Piedmont. The plant area and embankments are entirely within the Brevard Zone, which is a pronounced geologic lineament of deformed rocks extending from Alabama northeastward into North Carolina. The rocks at the site are biotite gneisses and schists, typically striking northeast with a southeast dip. Where sound and unweathered, they are highly competent, but because of pronounced foliation, they break into flat particles when excavated and crushed. However, this characteristic is not sufficiently detrimental to prevent the rock from being entirely adequate for riprap, bedding material, sub-ballast, and road base. It was not used for concrete aggregate.

Upland soils throughout the site are residual from in-place weathering of the underlying parent rock. These soils were used in the construction of the dikes and dams, and are generally sandy, micaceous, silts and silty, micaceous, fine sand. The upper layer, however, was more clayey because of advanced weathering of the minerals, and is generally described as red-brown, sandy, silty, clay or sandy, clayey silts.

In the valleys of the streams and smaller drainage features, alluvial soils cover the valley floors. These are primarily silts and fine sands, with pockets of soft, organic muck soils which required removal before placing embankments. They were not used for embankment fills.

Sources for embankment materials were explored prior to construction and are described in detail in appropriate reports (Appendix I). The location and approximate available yards in each area are shown on Drawing H-10061 and on the respective contract drawings. As more than one borrow area might be used at one time for several portions of the dam work, it is not reasonable to attempt to identify each portion of the embankments with a definite elevation of a specific borrow area. However, the daily and weekly reports for each of the structures identify the borrow areas and embankment stations being placed at the time of the report, and the source of the material at any station or elevation could be determined from these reports if necessary (Appendices IV and V). Air photos taken during construction are also helpful in identifying the borrow areas.

The residual soils from the borrow areas produced exceptionally fine embankments. Little difficulty was encountered in securing the required 100% compaction, with densitities averaging 100 to 110 lbs. per cu.ft. (See test reports, Appendices A through D). The strength of the material was demonstrated in several instances where trenches for drains were excavated with a backhoe, producing dense, vertical, soil walls. Laboratory tests made on soil samples taken from the embankment indicated strengths and permeabilities equal to or exceeding the design parameters (Appendix I, Reports 62 and 63).

Stone slope protection in the form of riprap and bedding material represented a substantial part of the cost of the embankment construction, and
every attempt was made to secure the necessary rock from the site with a minimum
of haul distance. Grading for the plant site required considerable excavation
below top of sound rock, and the blasted gneiss was stockpiled for later use

as riprap. Although the foliation of the gneiss produced blasted rock with a tendency toward flat particles, this feature was not sufficiently serious to effect its suitability for stone embankment protection. The blasted rock was stockpiled at the plant site after excavation until it was needed as riprap. Generally, the rock was reasonably well graded, and when placed on the embankment it was entirely satisfactory for embankment protection. The occasional stones which were too large for the specified riprap sizes were broken at the stockpile by a drop ball.

All riprap for the separation dike, and a portion of that required for the storage pond dam, was secured from the excavation stockpile. This supply was eventually depleted, and a quarry was opened in the storage pond area to provide the remainder of the rock needed for the storage pond dikes. All rock for the potable water supply dam was secured from a quarry opened for that purpose between the embankment and the spillway.

Bedding material between the embankment soil and the riprap layer was manufactured by crushing rock secured from site grading. The 3/8 inch crusher run material was mixed with 25% off-site white sand to produce a blend which was satisfactory with respect to gradation requirements. Riprap and bedding were placed on the dikes as follows:

Separation Dike, Pool Side:

- a. Three feet of riprap on 1 foot of bedding up to elevation 745.
- b. Two feet of riprap, without bedding, from elevation 745 to elevation 780.
- c. Two feet of riprap on 1 foot of bedding from elevation 780 to top of embankment.

Separation Dike, Ash pond Side:

Two feet of riprap from 775 berm to top of embankment. Bedding material (12") from E1. 790 to top. No rock protection below E1. 775.

Potable Water Supply Dam:

Potable Water Side - 24" riprap with no bedding to El. 795. From El. 795 to top 24" riprap on 12" of bedding.

Storage Pond Side - 24" riprap with no bedding to E1. 780. From E1. 780 to top 24" riprap on 12" of bedding.

Storage Pond Dam:

All embankment protected by 2 feet of riprap with 1 foot of bedding above El. 780. Areas of natural ground between embankment sections not protected.

On most embankment projects it is difficult to secure compaction at the edge of each compacted lift. When riprap and bedding is placed against such uncompacted material, and saturation occurs after the pool is impounded, slumping occurs which is often mistakenly attributed to riprap failure. On the Wansley embankments this was avoided by overbuilding the fill and then removing the loose exterior soil with a Gradall. Bedding material and riprap were then placed by building the embankment about 12 feet above the previously placed riprap; dumping the stone on top of the embankment; and placing the required bedding and riprap with the Gradall. This procedure resulted in a dense, well graded rock mass without underlying loose embankment fill.

By planned use of available site materials, adequate rock protection for the Wansley embankments has been secured at minimum cost.

EMBANKMENTS

East Dike - The Storage Pond Dike from Stations 0 through 22 is referred to as the East Dike. Construction started on September 19, 1972, with stripping of the entire dike section and additional undercutting of soft material between Station 9+00 and 20+00 prior to proof rolling the foundation subgrade. Mr. Turner witnessed the proof rolling operation prior to fill placement. In conformance with the construction procedures, a LETCo engineer, Mr. Prescott, was assigned the responsibility of construction inspection of the East Dike, starting September

On September 22, 1972, it was discovered that the upstream face of the dike was constructed on a 2(H):1(V) slope rather than 3:1, as specified. The contractor was directed to correct the slopes by benching in the addition width of the dike. (Appendix IV).

Excavation for the longitudinal drain between Station 7+50 and 9+50 was started on September 22. Initially, vertical sand drains (18" in diameter) were installed from Stations 7+65 through 11+65. Each drain was advanced to the natural ground and backfilled with concrete sand densified using concrete vibrators. After installation of the vertial drains, the specified longitudinal drains were constructed using a #57 stone core encased in one foot of concrete sand. All vertical, longitudinal, and transverse drains were constructed in this manner according to the specifications. To determine the effectiveness of the internal drains, a flow test was conducted on October 11, 1972. Water poured into the longitudinal drain at Station 17+10 started flowing through the transverse drain at Station 16+15, indicating that the drain was operational and effective (Appendix IV).

Because of inclement weather, construction of the East Dike was suspended on December 6, 1972. When work resumed on February 20, 1973, Mr. Prescott was replaced by Mr. Budde. With the exception of bedding and riprap placement on the upstream face, the dike was essentially completed by March 5, 1973. The bedding (1 foot thick) and riprap (2 feet thick) was placed in December 1974 and January 1975. This work was done according to the plans and specifications.

Detailed daily inspection reports for the East Dike are included in the "Dike Construction Progress Report," Appendix IV. The compaction test reports are attached as Appendix A.

Storage Pond Dike - The dike between Stations 22 and 110 was referred to during construction as Storage Pond Dike. The main fill embankment extends from Station 33 to Station 85. The section from Station 85 to 110 is primarily cut, and low embankment extends from Stations 95 and 98, and from Stations 105

to 108. Rock was blasted to obtain the specified elevation between Station 85 and 98 and from 109 to 110.

A cofferdam was constructed from Stations 60 to 70 and Yellowdirt Creek was diverted through the minor diversion ditch shown on Plan H-12350. Subsequently, the major diversion ditch was excavated. Presplit rock blasting techniques were required through the ridge for this excavation. The foundation was then cleaned and leveling concrete fill placed to the specified pipe invert elevation. Daniel Construction Company installed the twin steel pipe culverts and encased them in structural concrete. The north end of the twin steel pipes was extended with corrugated metal pipes. After diversion culverts were completed, Yellowdirt Creek was diverted through the pipes.

To facilitate dewatering of the remaining dike foundation at this location, several ditches were excavated and water pumped from a sump located at the south end of the dike. For dewatering the west side of the culvert, another sump was maintained to the left of the major diversion ditch. All alluvial soil (consisting of clay, sand, and gravel) was removed to rock beneath the entire dike foundation. After satisfactory clean-up of the foundation, select silty clay was used to start the embankment. The first layer of the fill was rolled with a penumatic tire roller to obtain better compaction on the uneven rock surface.

Fill placement proceeded satisfactorily and all internal vertical, longitudinal and transverse drains were installed according to plan (except as noted below) using sand purchased from T&L Company and rock manufactured at the site. The elevations of the longitudinal and transverse drain between Stations 54 and 70 were changed by Georgia Power Company's survey group. The actual elevation changes were noted on the plan and forwarded to Southern Services.

Daily compaction reports for the Storage Pond Dike are attached as

Appendix B. CONFIDENTIAL BUSINESS

Separation Dike - Detailed exploration and design studies for the Separation Dike are contained in Communication No. 39, Appendix I. The diversion scheme and construction drawings are shown on Dwgs. H-12364 through H-12366, and Dwgs. H-12396 through H-12398.

The plans required the removal of all alluvial soils and all sand and gravel from the core area, and the removal of the alluvial material only from under the balance of the embankment. Initial work on stripping began on May 17, 1973, and details of the construction of the embankment are contained in the reports shown in Appendix V. In brief, a 48-inch corregated metal pipe was installed for diversion of the creek. The upstream portion of the embankment, Stage 1, was constructed, after stripping, to El. 745. The core area in the flood plain was then undercut to weathered rock, and fill was compacted onto the inspected rock surface. Drainage was accomplished with open ditches and pumping from sumps. The location of the ditches and sumps are shown on report dated September 14, 1973, Appendix V.

Soils used for embankment fill were lean clays and silts obtained from borrow areas as described in the weekly reports (Appendix V) and compaction reports (Appendix C). Unit weights average 100 to 110 lbs per cu.ft., and 100% density was obtained. During September and October, 1973, sandy material was encountered in the borrow areas. Samples of the material was placed in the dike were obtained, and laboratory tests were conducted. The results showed the materials to be satisfactory with respect to design parameters (Communications 62 and 63, Appendix I).

On May 2, 1974, a dam was constructed to stop the water flow through the diversion pipe and both ends of the pipe were plugged with approximately 8 feet of concrete. The foundations for the balance of the dike were prepared, and fill

placement continued. The procedures for tying in the second stage construction with the first stage dike are described in letter dated September 28, 1973,

Appendix V. The horizontal drainage blankets shown on Dwg. H-12365 were constructed, using at first sand from Yellowdirt Creek, later supplemented by sand purchased from T&L Company of Whitesburg, Georgia.

Stone protection was placed on the embankment as described in the preceeding section of this report under "Construction Materials."

Potable Water Supply Dam - Details of the exploration and design for the Potable Water Supply Dam are described in Communications Nos. 43,44,46,47, and 53. Initial stripping began during the first week of November 1973 under the field supervision of Charles Edwards. Excavation of the valley section and the abutments was generally according to Dwg. H-12352, except that over-excavation of the east abutment produced a near vertical face from top of rock at about elevation 740 to overburden at 780. This face was later dressed back as the embankment was placed to produce a sloping contact between the fill and the weathered rock abutment (see attached photos).

Both abutments contained rock outcrops which would have prevented proper compaction of the embankment fill. These were blasted with light charges during foundation preparation to permit proper compaction of the fill against the abutment.

The 72" concrete pipe which will provide permanent control of the Potable Water Supply Pond was used for diversion of the creek (Dwg. H-12356). To control the stream flow during construction of the concrete pipe, the contractor constructed a wooden wall parallel to the stream channel. Holes 8" in diameter were drilled on about 10' centers for anchoring vertical wooden posts.

Tongue and groove joint laggings were then installed between the posts and anchored with sand bags. The creek was diverted to the east side of the wall, providing a fairly dry area for the construction of the concrete pipe. (See attached photographs for diversion details.)

All sand and gravel, alluvium, and broken rock debris were removed from the pipe foundatin and fill concrete placed to the specified elevation. After completion of the foundation work, the concrete pipe was installed and the lower half encased in concrete. Daniel Construction Company then built the north and south head walls and apron. The water was diverted through the pipe and a cofferdam constructed at the north end of the proposed dam to obtain a dry working area on both sides of the pipe.

The west side of the pipe area was cleaned first. Concrete was placed in any narrow slots in the foundation rock to facilitate the initial fill compaction. The first layer of embankment was selected clayey material compacted with pneumatic tired equipment. The fill was placed to the level of the top of the pipe. The east side of the pipe was then cleaned, leveled and similarly filled. When the embankment level reached the vertical cut described previously, the bank was sloped to allow better fill compaction and bond to the abutment.

The sand plug and sand-gravel drains were installed on both sides of the concrete pipe accoring to Dwg H-12356. All upstream and downstream horizontal sand blankets were installed as specified with sand purchased from T&L Company. Stone protection for both upstream and downstream embankment slopes was 12" of bedding material and 24" of riprap. The bedding material was manufactured from the stockpile stone, and riprap stone was secured from a quarry opened for that purpose west of the embankment.

Southland Power Constructors excavated the spillway channel, the lower end of which required blasting of rock to obtain the specified elevation. The spillway structure itself was built by Daniel Construction Company.

SUMMARY

The material contained in this report is intended to briefly furnish information on the personnel who supervised the inspection of the embankments, together with sufficient data to permit identification of the reports which are available for all portions of the project. If it should ever become necessary to review the construction details of any part of the embankment, the proper field inspection report contained in these appendices, together with the project plans and the appropriate portions of the specifications, should be sufficient to identify the records and the procedures used for that part of the project.

Dam embankments constructed of compacted Piedmont soils on properly prepared Piedmont foundations have an excellent performance record. The Wansley foundations are strong and relatively impervious, and the residual Piedmont soils, properly compacted, make excellent embankment fill. The Wansley embankments were conservatively designed, with adequate provisions for foundation and embankment drainge, and with heavy riprap protection against wave action. There is no feature of the construction which should ever be a cause for concern. However, if it should become necessary at some future date to investigate a feature or area of the work, the design studies and the plans and specifications, together with the construction records contained herein, should be adequate for determination of the problem and to indicate any necessary remedial work.



App. II

LAW ENGINEERING TESTING COMPANY

Geotechnical and Materials Engineers
412 PLASTERS AVENUE, N.E. / ATLANTA, GEORGIA 30324 / (404) 873-4761

PROCEDURES FOR QUALITY CONTROL DURING FILL PLACEMENT FOR DIKE CONSTRUCTION

GENERAL

- (1) All phases of dike construction will be witnessed by the civil group or their consultants to verify that the specifications and design drawings are adhered to and to verify that conditions in the field are similar to those upon which the designs were based. No changes in the specifications or designs will be made without prior approval of the design group of Georgia Power Co.
- (2) A soil technician will be assigned to specific areas of dike construction on a full-time basis to carry out the work as prescribed in this procedure.

 Note: The contractor must notify the soils technician in advance of his initiating any work in areas of dike construction.
- (3) The soil technician and site soil engineer will have authority to stop all dike construction in a given area if the design specifications and drawings are not being followed.

FILL CONTROL

- (4) All areas to receive fill will be thoroughly inspected by the site soils engineer to verify that adequate stripping of topsoil and organic debris is performed.
- (5) During filling, the earth moving operations will be continuously monitored visually and with testing to verify that proper soils are being utilized and that adequate compaction is being achieved.
- (6) At least one (1) density test will be made for each approximate 1500 yards of fill placed.
- (7) The nuclear density gage shall be utilized in monitoring the inplace density of the fill material. However, approximately twenty percent of tests made with the nuclear device will be verified by conventional sand cone or shelby tube methods, as described in the Quality Assurance Manual.

- (8) If the dry density obtained by the nuclear gage method varies by more than 4 lbs. from that of the conventional method, an additional conventional test will be made. The average density of the two (2) conventional method tests will then be compared with the average of the previous five (5) tests with the nuclear gage on that particular fill lift. If the average densities obtained by this procedure vary by less than 4 lbs., the tests will be considered valid. If they vary by more than 4 lbs. the site soils engineer will be consulted as to need for further testing of the suitability of the fill.
- (9) Generally, the density as determined by the nuclear density gage will be that which will be compared with the compaction curves for determination of suitability of compaction.
- (10) The density of the fill will be considered failing if a degree of compaction of less than 100 percent of the Standard Proctor Maximum dry density is obtained. Additional densification of the fill will be required if less than 100 percent is obtained.

INTERNAL DRAINS

- (11) The construction of the internal drains shall be continuously observed by a soils engineer from the civil group to verify that the drain locations are as required and that proper thicknesses of the required materials are installed.
- (12) The materials in the drains shall be compacted to the suitability of the soils engineer.
- (13) Fine aggregate and coarse aggregates being utilized in drain construction will be tested each day to verify that their gradation is in compliance with the design requirements.
- (14) On an occasional basis, the soils engineer will inject water into the drains prior to their being covered with fill to verify that they will perform satisfactorily.

RIP-RAP PLACEMENT

- (15) The installation of rip-rap will be continuously observed by an engineer or technician from the soil group.
- (16) He will verify that the required thicknesses of both the cushion blanket of fine stone and outer layer of armor stone are placed.
- (17) The cushion blanket material will be tested daily to verify that its gradation is in compliance with the Kiples Williams.
- (18) The armor stone will be visually inspected and volume measurements of pieces made to verify that its general gradation and maximum stone size are in compliance with the specifications.

FROM App. V.

September 28, 1973

Mr. G. B. Dougherty Southern Services Inc. P. O. Box 2625 Birmingham, Alabama 35202

Subject: Borrow Material And Construction Procedures For The Plant Wansley Separation Dam

Dear Mr. Dougherty:

After encountering low density borrow material (90 to 95 pounds per cubic foot dry density) and very sandy material, we decided that additional testing was needed. A two-foot lift of the sandy material was placed on the upstream edge of the separation dam. Six undisturbed samples were obtained on September 27. These samples were sent to Law Engineering Testing Company for triaxial testing.

As soon as the weather permits, a two-foot lift of the low density material will be placed. This test lift will only cover an area sufficiently large enough to obtain the same compactive effort as is applied to large fill areas. We will obtain six more undisturbed samples for triaxial testing. Until we obtain the test results and your written approval, this low density material will only be placed in one six-inch lift per ten feet of fill. No additional sandy material will be used until we get your written approval.

All triaxial test results and all in-place density test results on the separation dam will be forwarded to you. After the results of these triaxial tests are obtained, we will decide on a definite schedule of triaxial testing. We plan to obtain five additional undisturbed samples of the normal specified fill material for triaxial testing as soon as filling resumes on the separation dam.

As to your question on tying the Stage I Construction to the remaining fill, we plan to continuously bench into the sloping fill. We will have a dozer cut into the existing fill until we obtain material that has the specified density as verified by in-place density tests. Close observation is provided by me and the field technicians to assure that this operation is performed correctly. It is simple to visualize that a proper benching operation requires that, at all times, the compacted material into which the horizontal bench is cut must be showing on the vertical portion of the bench cut.

Wr. G. B. Dougherty - 9-28-73

When tying the fill to the existing abutments, several procedures are used. Steep abutments are cut back as much as possible. The existing natural ground is scarified completely before fill placement starts. The abutment is benched, if necessary. The rollers continuously run from the fill material onto the abutment. Since the sheeps foot or grid roller leaves the upper six inches of the compacted fill in a loose state, successive layers are fused together in a continual process, insuring an excellent seal between the existing abutment and the placed fill.

Yours very truly,

GEORGIA POWER COMPANY

Charles A. Edwards Senior Engineering Associate

CAE:esw

Mr. R. S. Ponsell Mr. Don Foster Quality Assurance File GEORGIA FUWER CUMPANY - SUILS NUCLEAR DENSITY METER WORK SHEET

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GEURGIA FUWER CUMPANT - SUILS NUCLEAR DENSITY METER WORK SHEET

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(I) Dry Wt.	93.6	101.5	47.01	996	1	1000	000/	1000	
(J) Proctor Wt.	26	100		34	102	101	100	10%	
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(E) Wet Wt. (1b/ft3)	127,0	127,5	125,5	126,5	5.781	57.61	0.227	1000
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(I) Dry Wt.	10212	102,7	5/1/	121 5.	101.9	707	7.6.7	, , , , , , , , , , , , , , , , , , , ,
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(D) C ÷ Std Count			÷					0
(E) Wet Wt. (1b/ft ³)	1727.2	124.7	126.2	125.7	124.0	123.2	125.2	126.2.
(f) Moist Count	1040	00/1	1030	1010	1120	070/	1000	066.
(G) F ÷ Std Count			=	, pa	. 11			
(H) Wt. Moist (1bs)	20,3	21,7	20.1	9:61	222	20.0	19.3	1.'61
(I) Dry Wt.	106,9	1030	1.901	1.90/	1018	1030	105 %	1 701
(J) Proctor Wt.	60	103	163	103	101	63	82	150
(K) % Comp. I ÷ J	1007	1007	1001	1001	1001	1001	1007	11105
(L) % Moist	Sr	22.0	GT.					
(N) Location	27+00	27+15	25+00	25+15	2.3700	23+15	2)+00	2/+20
(0) Elev.		754	755	754	12.4	753	752	7.51
			a a			©.		

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required_

Technician

GEURGIA FUWER GUMPANY - SUILS NUCLEAR DENSITY METER WORK SHEET

WANSLEY

0 0.0 32,06 17.70 9 02.7 12 3 2 000 cV 0 2 120/26 107.0 18720 5.4.21 107.9 1007 V/0 19.61 7/6 430 5 MOISTURE STANDARD COUNT 173 110'RE 001 101.5 207 18700 0,0 123.5 22.0 ロンナ 4/1 4 715 37,09/37,09/37,09/ 1001 0 94.4 MATERIAL 124.5 1464 22-150 ら 02. ~ . ○ 13 95.2. 23750 100, 95.0 760 9221 27.3 72.5 >#(CH) 7 24+50 985 95.0 00 + 27.5 5 1240 982, 440 14417 37,091 95.8 28.7 95.0 124.5 308 +00 25450 758 450 OAR 0 97.4 29.6 37,091 95,0 26+50 100, 1440 127.0 422 757 SE1/AUA 110W 0 (E) Wet Wt. (1b/ft³) H) Wt. Moist (1bs) (D) C + Std Count G) F + Std Count C) Dens. Count F) Moist Count ٠,٠ A) Test Number B) Probe Depth (J) Proctor Wt. K) % Comp. I N) Location (L) % Moist I) Dry Wt. PROJECT: Elev. AREA: 0

Compaction Required

1/3 Technician

2-5-76

DATE: 7-2/-/3
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT: 2000 9-27-73 2.86 100 100 101 17.470 127.0 032/ 700 430 1 001 CC 125 47 8-129.5 2.66 30.3 1470 17-50 0/7) : --- : 707 10/2 128.0 1424 5 26.8 N 320 + 51 1,5 100172-8.76 436 S 11 1 1400 2B.7 3400 126, J $O_{\mathcal{S}}$ 127.6 103,6 129.5 259 1280 O, 17.420 412 S 701 1.1 100478 1300 0,00 26.2 0/1 1290 406 00 00+21 Ş E) Wet Wt. (15/ft³) Ser Dam H) Wt. Moist (1bs) (D) C + Std Count (G) F + Std Count F) Moist Count (C) Dens. Count A) Test Number (K) % Comp. I ÷ B) Probe Depth (J) Proctor Wt. (N) Location (L) % Moist (I) Dry Wt. PROJECT: JOB NO.: (0) Elev. AREA:

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required

Technicfan

WORK SHEET

	t				١_,	31 0 10° = 1	- 111			D	(Shelby Tube Meth
, Oje	<u> </u>	8			ш	1	= 10	· · · · · · · · · · · · · · · · · · ·		Jo	ob / lab no
	w. w.	107			w. w.	D. W.					To have
EST O.	SAMPLE & MOLD	WT. MOLD	W. W. SAMPLE	W. W. CU. FT.	MOIST SAMPLE	MOIST SAMPLE	W. MOIST	MOIST	DRY DENSITY	* COM	P. LOCATION
		agive 3a						-			
'				177				1 % 50		14.	- 12 - 13 - 13
				,							
		1		1 4.5	. 5 ₁ *	1.6.7	200 - 7		2.5	1/13	
	.1			1997	<i>1</i> °,	03.2	4 A	13. 15.	1/5 /	: ^-:	
	e ;			•	, ·		,1	الما أما	1	1	
-											
			13.7	· (;	
_	16	/ / · · · ·			: .)	,			
				1070		83.8		100	146.0	-	n r
		3		127.8		00.0		19.3	106.6	-	See Saco
				12.6.3				229	113.0		
Co		n Curve I	No		-	8					<u>18.0</u>
Co	mpaction	n Requir	ed	%			 Mold	l Volum	e Factor		-, \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\
No	ot to be ty	yped		CONFI	DENTIAL MATION	BUSINESS	Work	perfori	ned by	!	-, \d.,

PRÒJECT: / //: s /- v. j JOB NO.: AREA:

DATE: 9-77 STANDARD COUNT: 03/

·				8		;	1000	
(A) Test Number		6)	5	*/-	J	9	3	
(8) Probe Depth								
(C) Dens. Count	(2	7) 1)	7 1 1 1			27		*
(D) C ÷ Std Count				- 25				
(E) Wet Wt. (1b/ft3)	2.8.2		- 5 C	*		14.5 -7	*	
(F) Moist Count	1070	0.0	()			C C		
(G) F ÷ Std Count	8. 43	v =			* * * *			
(H) Wt. Moist (1bs)		6.14	2	8 cm 537 c 5) i	57		
(I) Dry Wt.	1.07	108.7	1.05	3 1 1 2 3	112.3	60		ī
(J) Proctor Wt.	`		// .					n.
(K) % Comp. I ÷ J	1,52					3.3		
(L) % Moist	0.0°	5 5	() 1		7		:	
(N) Location	12174) in ''-	چ د	(1) (2) (3)	7 m	1.7.7		
	135 178		140 - 5	3	7. 1. 1.	以 1 1 1 1 1 2		
(0) Elev.	0		; ,t ,t,		ū	30) 27 4		

Compaction Required

Technician

PROJECT: Wans | eg JOB NO.: Seperation

DATE: 2 2 7 199.
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT

(A) Test Number		C	5	4	9	9	2	Ox
(8) Probe Depth	2							2000
(C) Dens. Count	460	438	450	440	464	458	486	470
(D) C ÷ Std Count	126.4	1295	128.0	1289	126.5	1240	0201	1250
(E) Wet Wt. (1b/ft ³)	The state of the s					2000	1254	9
(F) Moist Count	1432	1422	1630	1472	15.04	1548	1466	1390
(G) F : Std Count								
(H) Wt. Moist (1bs)	23.2	23.0	270	24.0	247	25.4	228	5 66
(I) Dry Wt.	103.2	206.5		:	101.8	9.701	TAT	103.3
(J) Proctor Wt.	103	901	100	103	40/	100	1	103
(K) % Comp. I ÷ J	1,001	100%	100%	16001	100%	16001	10001	10001
(L) % Moist								,
	SO CT SP. C.	60FF L. E	50 FT BE 60 FT H. & Syx & 60 FT 601 24 & 50 25 &	245 109 T	50 FA 6	28 34 6 5014 G 50 24 E	5014 G	50 24 g
(N) Location	54 12 +00	STa 12+15	St 13+05 St 13+20 St 14+30	St 13+20		St 14 +45 St 15+05 St 18-720	5815+05	St 15-38
(0) Elev.	47.40	179	209	708	202	202	699	063
	No.	THE PARTY OF THE P	*					7.7

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required

Technician Q.7

PROJECT: U JOB NO.:

122 DENSITY STANDARD COUNT:

(A) Test Number		7	M	7	4	7	(٥
(B) Probe Depth	, 9		50 50 53:83)		D
(C) Dens. Count	408	422	416	23	201	1160	11.0	1.0
(D) C + Std Count						702	8/2	7.00
(E) Wet Wt. (1b/ft3)	0921	2 8 6	1290	1.14				
	7 3	1000	العراز م	81.5	5.0.5	1300	0 ۲۶/	128.5
(F) Moist Count	2/ 1/	1194	1204	1192	17.04	08/1	1/86	6011
(G) F + Std Count				115				0//
(H) Wt. Moist (1bs)	73.4	23.8.	740	020	1	0 5	1	- 1
				0.7.0	ログン	6 2. 8	23.5	۵.۲.۶
(I) Dry Wt.	106,6	164,7	105.0	105,7	106.5	106.2	8 701	104,9
(J) Proctor Wt.	105	105	105	707	/k c	7.10.5) \/ /
(K) % Comp. I ÷ J	700	00/	Civi	007	(VX)	((() (50/	100
(1) % Moist					20	00.	· (c (C/	00/
281011 0 121				8		() ()		Ļ
(N) Location	1/400	11415	2750	12466	5451	13485	1425	D5751
	30'R.		80R & -		100'R L		30.82	
(0) Elev.	701	200	259	859	239	289	157	3
	•						, , , o	J ~

Compaction Reguired

Technician (//

PROJECT: JOB NO.: AREA:

DATE: 9.27.75 DENSITY STANDARD COUNT: 23/	MOISTOKE STANDAKD COUNT: (27)
<u>.</u>	
135Kt	

			# # # #					
(A) Test Number	0	0/	//	12	/3	14	12	11
(8) Probe Depth	27		10 12 12 13	# @*	. 2			5
(C) Dens. Count	705	4/2	89	700	365	416	404	414
(D) C ÷ Std Count								
(E) Wet Wt. (1b/ft3)	131.0	129.5	0.0001	130.0	1015	0.501	130.5%	139 (
(F) Moist Count	1174	7611	7121	1204	0601	1744	13/16	18/
(G) F ÷ Std Count			**	<i>3</i>			3	9
(H) Wt. Moist (1bs)	23.4	27,8	24.6	24.0	215	C22	9,52	2 > .8
(I) Dry Wt.	107,6	105,7	105.4	10.6.0	0.011	106.3	105,9	1057
(J) Proctor Wt.	1012		,50/	C 3 P 1	, n	スクワ	M	4 0/
(K) % Comp. I ÷ J	70%	0.0	\$ N.		00%	000	700	100
(L) % Moist		260	35 35	2 a 25				
(N) Location	20+11	11+85	12+50	13+65	13+25	13+35	14400	NEDI
	100Rd		6	A commence of the second	30.114	en la company de manage service de la company de la compan	2012	Samuel and spice of the
(0) Elev.	704	205	364	7 53	2.94	239	1.54	1.5%
						+		***************************************

Compaction Required_

Technician

PROJECT: N. ANGELONS

AREA: C. A

DATE: C. C. C. C. DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

		2 Miles						
(A) Test Number		<u>.</u>	r A ^T E		\$C	\.	1	α
(8) Probe Depth	20		, S. 1811 1811)
(C) Dens. Count	G.,	222	0)	- X 10 00 00 00 00 00 00 00 00 00 00 00 00	3 . 3 . 3 .	(g;		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
(D) C ÷ Std Count	=	ı				1		
(E) Wet Wt. (1b/ft ³)	1) 2)	2 2	ut,			* # 10 E	1.	
(F) Moist Count	0.3.0		***	3 (S)	50 S	0.00		F C C
(G) F ÷ Std Count				10 10 11	N S		; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;	J () }
(H) Wt. Moist (1bs)	- 	-r -r	1. E 7	J.,	1 6	\$ 1 P	**	
(I) Dry Wt.								(1)
(J) Proctor Wt.			(
(K) % Comp. I ÷ J)	¥ .		-X		11"	i.c.	
(L) % Moist				15. 19	at:			
(N) Location	[] - 1)		.r 3.	`*** ****	ن مرا	1		1.
	75.87 8	Á,	を存む		; ; ;	**) - - - - -	2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 -
(0) Elev.			3 6	# 35 S		₹0	,	·
		•			**************************************			

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Required	
Compaction	9

Technician

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PROJECT: 7 1 1 1 JOB NO.:

DATE:
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

	8							
(A) Test Number	, =	= ,		1 E	F1 52			
			S#1				n =	
(8) Probe Depth		= 3						
(C) Dens. Count	u Te)		9963		-8	
(D) C + Std Count								2
(E) Wet Wt. (1b/ft ³)		¥100 9. 1000		* *	(+0) (6)			
(F) Moist Count	ं 3 €	5	1. 7.6	2				30 2
(G) F ÷ Std Count): -						
(H) Wt. Moist (1bs)	T. W.		5.0	2.5.0				*
(I) Dry Wt.	1021	9.		995	es Orga	ix	¥	
(J) Proctor Wt.	71 g 14 e							
(K) % Comp. I ÷ J	102	 LC	22/	. j				****
(L) % Moist		==						
(N) Location	12450	1770	13250	, , , , , , , , , , , , , , , , , , ,	, w			
(0) Elev.		1.61						
		e E				 		

Compaction Required

Technician

PROJECT: INDUSTER/ JOB NO.:

DATE: 7225 CS DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

(A) Test Number		2	3	4	5	7	0	4
(B) Probe Depth	100							0
(C) Dens. Count	452	44	47.8	434	418	426	421	1111
(D) C ÷ Std Count	*				8	1	700	144
(E) Wet Wt. (1b/ft3) 124, S	124,5	125.0	127.5	126.5	178.5	13/ 6	1270	1300
(F) Moist Count	1202	1 24	1234	1224	87 //	1611	100	15.5
(G) F ÷ Std Count		#3 '=	E3	В		9		400
(H) Wt. Moist (1bs)	240	24,2	24.8	2,46	20,6	240	23.8	262
(I) Dry Wt.	5001	100,8	L 20/	10/9	6 (0)	107	1027	1210
(J) Proctor Wt.	09/	001), 09/	90/	۲ 991	199	100	2
(K) % Comp. I ÷ J	187	00/	0.9/	/00	69/	49.		0.7
(L) % Moist					22		20/	00/
(N) Location	10/20	09401	25+11	5CA11	12+25	12765	09+121	14430
	نه		80R4		\$ 7,90%	1	25.84	
(0) Elev.	7/2	7/3	2/2	7/3	712	7/3	2/3	7/4
				-				

Compaction Required

Technician

PROJECT: UMUS/C JOB NO.: SEP DR

DATE: 04673 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

			-	*				
}	6	0/			0	N	1	1
	900				2		1	7
	460	454	436	442	4/16	45.3	45%	1112
(D) C ÷ Std Count						3		7
£3)	(E) Wet Wt. (1b/ft3) 123,5	1240	126,5	125.5	(1967)	13/1/5/	0/10	, , , ,
	11.78	1196	12.32	7511	1011	1822		トレイン
(G) F ÷ Std Count						1212	7/80	(16%
(H) Wt. Moist (1bs)	23.6	23.8	248	27.8	19,6	241	23/	724
	99,9	100.2	101.7	101.7.	109.4	0 00	4 6 3	1.7.5
	(No	00/	00	00/	891	1,60	7,007	0 0 7
	90)	qal	Qp.1	63	6/	/n/	2	100
	*0				20		00/	08/
1 10	10/58	1940	12700	12+10	13400	13420	14472	14480
	\$7,00	1	10012E	1	10074	1	0	
	2/3	114	15/10	177	2/4	7/5	2/6	7/5

Sompaction Required /60

Technician ()

PROJECT: NA ANS IS JOB NO.:

COUNT:	
DATE: STANDARD COUNT: MOISTURE STANDARD COUNT:	
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1 - 15 mm 2 - 1	Minhos

			1.1		•			
(A) Test Number		7. 	rr,	5.	r.	1	<i>Y</i>	
(8) Probe Depth			# T	(a)		0		10
(C) Dens. Count	430	4. 10.	26.11.	18:3 7:1		5.00		
(D) C ÷ Std Count		107	line.				χ, τ,	() ()
(E) Wet Wt. (1b/ft3)	321 727			1	12/2/		723 4 73	
(F) Moist Count	1060	1660	026) - 2	01:07	- 11 25 7 7 7 8 7 8 7 8 7 8 8 7 8 8 8 8 8 8 8	107
(G) F ÷ Std Count		- %-)) (7720
(H) Wt. Moist (1bs)	304	200	7.37		110	6.	13 Au -	
(I) Dry Wt.	1064	ケーグ	. C = W.		35% 17	9		
(J) Proctor Wt.	4.4	, , , , , , , , , , , , , , , , , , ,		(X)		0 :		7.0
(K) % Comp. I ÷ J	102	1 6.7	100	7		101	1	
(L) % Moist	19.5 17.0	-		* * * * * * * * * * * * * * * * * * *		9)	A
(N) Location	57756		04050			i, 1	7 7 7 8 8 4	
(0) Elev.	1	Q (1)	1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		H 1/1.	: !		\\ \
								1/

1. 1. 1. Technician

Compaction Required_

DATE:
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

Carlin " " " " Play

PROJECT: 1455 100 JOB NO.:

	I							
(A) lest Number		00			81	- II 1		
(8) Probe Depth								
(C) Dens. Count	1. A.	2	- 1 - 8 - 12 - 12 - 12 - 12 - 12 - 12 - 12 - 12					
(D) C ÷ Std Count								
(E) Wet Wt. (1b/ft ³)	124.7	125.7	·	2	** A ***		2	
(F) Moist Count	1120	7.30						
(G) F ÷ Std Count	222	21.7						
(H) Wt. Moist (1bs)	1625	12.40						12 2.
(I) Dry Wt.	100	() (
(J) Proctor Wt.	7.07	134				28	·	
(K) % Comp. I ÷ J								
(L) % Moist					-			× =
(N) Location	ā	0471			<u>.</u>			
	3077	4						
(0) Elev.	746	5.7		8				

CONFIDENTIAL BUSINESS	INFORMATION

Compaction Required

Technician

DATE: 7/25/22 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

(A) Test Nimber		V / -		-		==	13	
יייי ייייייייייייייייייייייייייייייייי		ŗ	/ /		د / د د	14		7/
(8) Probe Depth		is.	2	0				
(C) Dens. Count		,	0.77		< \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \			
$(0) C \div Std Count$		# _ ()). C) }	次。 す す	100
C - 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0 - :							-
(E) Wet Wt. (1b/ft3) (77,5)	302		07:11	F / 1 4 .	1220	27.21.	17.	i
(F) Moist Count		00	12.20	77	À	0401		1060
(G) F 🗦 Std Count				5.50 0 0 0	9			- 17.
(H) Wt. Moist (1bs)	53.0		*r *r	2:1	. 54	200		
(I) Dry Wt.	3 %:			25.7		111/18		
(1) Pronton Wt	300 000					1.0%	7.7	
יייי פרבים ארי		7 7		537	S.	(E)	•	``!
(K) % Comp. I ÷ J	00,	- (C)	. ,	2 (2) 1 (2) 1 (2)		103	7.17	(M)
(L) % Moist		34		 				11.
(N) Location	シンナンご	つ :: ナ:::::::::::::::::::::::::::::::::	169	- 4 :: :	1	√ . + :. · ·		V
	1	<u>.</u>	1 to			30		0
(0) Elev.	ジナム	55%	1.51	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	7	1	>1	4
						770		ر برب

Compaction Required

Technician

PROJECT: WANSfey JOB NO.: SEP DE

DATE: 3-2 2-73
DENSITY STANDARD COUNT:

	-							
(A) Test Number		2	^	X		`		2
(B) Probe Depth	" 9						,	8
(C) Dens. Count	482	464	486	466	492	488	478	dell
(D) C ÷ Std Count								20)
(E) Wet Wt. (1b/ft ³)	(12/0	118.5	120.5	127.0	5611	13001	12/6/	
(F) Moist Count	050	1080	79 91	70//	1628	1056	/ K. J.	1,00.5
(G) F ÷ Std Count				-		6 (0)	-/0/	1.060
(H) Wt. Moist (1bs)	20.6	2/.2	20.8	22.0	2.0. /	200	216	1/6
(I) Dry Wt.	100,4	90.2	99.7	1.6.0.0.	4 6 9	0 6	3 4 6	000
(J) Proctor Wt.	x 490.0	85	#/00.0	1/20°C		100	10 00 C	
(K) % Comp. I ÷ J	100	99/	186	100				1
(L) % Moist					1.		00	000
(N) Location	52+51	15+80	24.00	2/+15	22 +75	23-1CM	2 Laton	25415
	1-50,7 €		10076		22022	1	2.2576	1
(0) Elev.	753	754	755	256	755	257	73,	1
The state of the s		PROCTO	OR				121	120
	5					•		

CONFIDENTIAL BUSINESS INFORMATION

Technician

Compaction Required

PROJECT: JOB NO.: AREA:

DATE: 7-23-78
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT 9-25-73

			£					
(A) Test Number	5	18	//	. 2/	57	14	1/2	7/
(8) Probe Depth	<i>u</i> 9			1840				
(C) Dens. Count	466	485	49%	468	472	4 46	438	454
(D) C ÷ Std Count				+8 		231 .		
(E) Wet Wt. (1b/ft ³)	122.0	12/.0	123.0	122.5	122.0	1250	126.0	12 4.0
(F) Moist Count	1684	8701	79//	1014	16.20	856	9 76	286
(G) F + Std Count		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		33 34				
(H) Wt. Moist (1bs)	21.3	20,6	21,7	15.6	19.9	19.3	6'81	18.8
(I) Dry Wt.	101.2	100,4	10.1,3	6.20/	1021	105.7	107.1	1001
(J) Proctor Wt.	/ bb	706	-\u00e40/	700	/00	103	106	
(K) % Comp. I ÷ J	106	(db)	/مه	00/	00/	70/	: 09/	101
(L) % Moist	4	li II				<u>.</u>		207
(N) Location	20420	20+65	22450	22+60	2 4400	24+15-	25475	25785
	77,08	1	150.74.	1	100'ZE		1.65.78	
(0) Elev.	\$5L	020	758	759	256	75.5	760	75.9
			*00					

Compaction Required

Technician

アイグライボン PROJECT: JOB NO.:

DATE: SENDARD COUNT: MOISTURE STANDARD COUNT:

(A) Test Number		4	<i>~</i>	A	V	7	7	a
(8) Prởbe Depth	,,9	tes (te	i de la company de la comp	a l'opposition automat a 18 g. village parage.				
(C) Dens. Count	456	469	496	404	278	462	700	111
(D) C ÷ Std Count	30 8 - 8 - 1					3	0	1::
(E) Wet Wt. (1b/ft3)	127.5	126.5	133.0	123.0	130.5	126.0	1767	1007
(F) Moist Count	1564	1530	1.418	1454	11.92	1288	000	(//-:/
(G) F ÷ Std Count		. В г		0 5 8				
(H) Wt. Moist (1bs)	25.8	25.0	23.0	23.6	19.8	20,0	2 174	V - 4
(I) Dry Wt.	101.7	101.5	100.0	99.7	111.7	105.5	102.5	701
(J) Proctor Wt.	100.0	1000	1000	98.0	110.0	1000		0 .7
(K) % Comp. I ÷ J	10019	100 +9	100%	10079	100-001	10001		
(L) % Moist			-					
	22+00	22 + 30	24+50	24+70	27+00	27420	0.74	
(JN) Location	100,77. 6	30,77.6	100' LT. C	327,00	3.17,08	7071.6) 1 1
(0) Elev.	755	754	753	753	755	154	X 70	123
						†	\ \frac{1}{2}	////

Compaction Required

Technician

PROJECT: \N/AMS\

1237 DATE: 7/24//5 DENSITY/STANDARD COUNT: MOISTURE STANDARD COUNT:

	S (8)							
(A) Test Number	0	01		: 2/	13	*	· · · · · · · · ·	
(8) Probe Depth			8			-		0
(C) Dens. Count	43-1		207	() ()	4.0	43B	157	27.7
(D) C + Std Count		. 8	1.6 2.	173	1,64	167	173	1.00
(E) Wet Wt. (lb/ft3)	₹:	. V (1)	/3//	13	130.5		1- 1 - 1	007
(F) Moist Count	120)/71	1340	(Q) (A) (A)	0.2%/	(30)		20.0
(G) F ÷ Std Count	-	Ē	п		1 1			
(H) Wt. Moist (1bs)	22.0		215	11:	1-1-0		000	7, 110
(I) Dry Wt.	0.80	8.60	. 08.5		8607	= 601	1.001	20.1
(J) Proctor Wt.	1.41	1-1	7.7			7	1777	0
(K) % Comp. I ÷ J	107	7	201	101	80/	0.11		
(L) % Moist			5 =					
(N) Location	38.48	5	70400 125878-	. (20450	9 70 3	07417	2
(0) Elev.			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	4)	1		11.0 N. C.	
					, , , , , , , , , , , , , , , , , , , ,			

Compaction Required

Technician

. A. Call definite.

PROJECT: Wansley
JOB NO.: Tatate - accous Rogs

DATE: Leaf 25-1927
DENSITY STANDARD COUNT:

N		Ŋ	7	3	S		q
1	11.9	19	"9	"9	110	10	
424	452	494	500	432	456	474	`
		125.0	200		11		-
128.0	1220	A A	119.00 THE	1258	1200	000	10.01
		426		201	1	1.44.0	14110
031	1140	1853	74	1130	75.01	080	3001
25.2	22,7	18.3	13,4	223	000	# 15.90	100
104.8	104.3	104.7	105.6	104.19	103.8	103 1	7077
163	103	103	103	12,5	103	102	100
100%	100%	106%	100%	1,00%	10001	1000	2 2 2
		اقيا				0/00	10072
1	30146	3, 4 6	名本の	12/010	101 07 6	5 401.61	12/1/12
racing North	Facing Warth		Facing Most	Faeing Marty	Facing North	Facing Hoth	Took Al
san eell	downstreamed down streamed down of		551 North 30 Northof 45' North of 1201 North of Sunt	30' Northor	45' North or	120/North	13510 100 19
633	630	ļ	68# call	GSA Call 693 CRA CALL	C & S	200	7700
678	687	1	6487				0 01

Jompaction Required

Technician Q. 7

92-5-2

PROJECT: (=) // (=) JOB NO.:

100 1 Con 100 Con 100

DATE: 7-2 7 7 5 5 1 DENSITY STANDARD COUNT: 7271

(A) Test Number	6	۵/	//	0/	7.	14	je j	1/1
(B) Probe Depth	× 29)			
(C) Dens. Count	かかた	412	434	448	454	482	2/1/2	426
(D) C ÷ Std Count			© (20)					i.
(E) Wet Wt. (1b/ft ³)	975 Cl .	126.0	126.5	17510	1240	123,5	037	126.5
(F) Moist Count	1628	1094	0(,0/	\$501·	922	296	54.5%	
(G) F ÷ Std Count	•		- 12- - 15) A S			
(H) Wt. Moist (1bs)	6.61	21,5	214	20,6	681	18, 7	300	200
(I) Dry Wt.	104, 1	16.4 C	10 5 01	1044	12	× 7 ° 1	/6., /	(1)
(J) Proctor Wt.	, 5 //	163	10,7	10.3	5.97	TE	101.3	7.7.7.
(K) % Comp. I ÷ J	7.00	/bn	90/	106	100	90/	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	(1),
(L) % Moist	ž.	12 Sec						
(N) Location	21+50	09417	22+75	23+00	081-62	3.400	05492	26465
	26176	4	100.75	The state of the s	\$ 7021 C	Andrew Commission and analysis of the Commission and the Commission an	7674	And the state of t
(0) Elev.	8	-1 (30	717	763	A) C	76.5	764	765
						The second secon		

Compaction Required

Technician

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Technician

PROJECT: (A) SOLVEY SOL

DATE: DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

			1960 50 00					
(A) Test Number		2		1	1	?	Corner .	5
(B) Probe Depth	6. "	of the control of the				: ::		
(C) Dens. Count	205	797	456	462	877	454	400	4.30
(D) C + Std Count	97		8 3 3		7 8			\$.
(E) Wet Wt. (1b/ft ³)	5'8/1	123.0	1340	123.5	1250	124.0	1250	13 % 86
(F) Moist Count	150	132	1168	11.26	260	1062	×101	(CO 5)
(G) F ÷ Std Count	15.		992				2	1. 0
(H) Wt. Moist (1bs)	0.52	22,4	p '62	22,4	26,7	20,0	70.7	
(I) Dry Wt.	95.5	100,6	00.0	10/1	103 3	527	1	0
(J) Proctor Wt.	100	100	100	00/	ζο /	103	1	187
(К) % Сощр. I ÷ J	96	991	qa/	09/	(1.61)	GW?	7.17	42.
(L) % Moist	ReKulli	100	and the state of t	American Company of the Section of t		نامسه معلومها الاستشهدات (* ۱۳۰۰)		5
	23+50/23460	53460	25+00	25720	23740	22+25	00 10	0470
(N) Location	23042	1	27575	A	25026	A A A A STATE OF THE SECTION OF THE ASSESSMENT OF THE SECTION OF T	P 7.091	
(0) Elev.	141	747	742	743	740	739	758	129
							*	·

Compaction Required / ()

Technician /

JOB NO.:

DATE: 7-72-7: DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

	8							
(A) Test Number	0	0/=		12	10	4/		
(8) Probe Depth	29							C
(C) Dens. Count	454	430	430	4 30.	480	062		000
(D) C ÷ Std Count		# F		2	П		. 11	143 × 7×
(E) Wet Wt. (1b/ft ³)	C 100	1272	27.01	; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;	(2101)	i-17	1215	2001
(F) Moist Count	0.70/			49				3
(G) F ÷ Std Count	and the second			1050	0701	000		000
(H) Wt. Moist (1bs)	\$ 0.7	22.2	713	206	24.9		ָּרָר רָיַ	2 17 7
(I) Dry Wt.	103.4	105.0	1.76.	750,	1004	1.2.2	(00)	71-0,
(J) Proctor Wt.	100	241	1.02	867	00/	1.7	75 7	13 UV
(K) % Comp. I ÷ J	10.3	201	103	207	(0)		1 4 1	7
(L) % Moist								
	60 60 84			. 5				2
(N) Location	25+00	5/730	00/197	シーでい	05787	ニンナン	0000	11.
	- 3471	4	1527 2 4		w)	\$ J	- B 2.00	4
(0) Elev.	1,50	130	671	110	ton	N	0.7	72
	200			10 10 10 10 10 10 10 10 10 10 10 10 10 1		The same of the sa		7.5

Compaction Required /

Technician

> PROJECT: JOB NO.:

DATE: 7 STANDARD COUNT: MOISTURE STANDARD COUNT:

			8 8	·				
(A) Test Number	i i	in a constant of the constant	ζ.	+	6	\(\)		Œ
(8) Probe Depth		s		. A		29 27		
(C) Dens. Count	900	118	200	480	540	620	Ø. 1	100 m
(D) C ÷ Std Count	#25i				e e			; ·
(E) Wet Wt. (1b/ft ³)	0.121	1287	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	0121.2	1197	116.7)	120,1	0201
(F) Moist Count	1240	020	058	1080	1060	CAL	0100	2000
(G) F & Std Count	3	n Par						
(H) Wt. Moist (1bs)	2.5.0	19.5	691	4	70.8	0.7	, ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;	4 6
(I) Dry Wt.	106.0	0000	166	000/	94.0	10 % AS	\$ C.	1001
(J) Proctor Wt.	10	CUI	8	8	34	020	101	777
(K) % Comp. $I \div J$	105	707	101	102	00/	100	100	101
(L) % Moist		-					3	1
(N) Location	25400	101-150	(stell	ひとせるこ	05+10	21.170	0.7076	07.77
	100178	485 framewo	727011	A comment	25.778	4	50 RT 2-	
(0) Elev.	798	747	9.7%	5 66	974	1.55	074	
			30.					

Compaction Required

Technician

PROJECT: Who shered JOB NO.:

DATE: 7-21-75
DENSITY STANDARD COUNT: 231
MOISTURE STANDARD COUNT:

								2.
(A) Test Number		7	on	A	7,0			
	1 21	2		*	2	g	***	9
(8) Probe Depth	9	(a) (a)						
(C) Dens. Count	438	426	434	428	420	470	7/17	41.0
(D) C ÷ Std Count					Park.		2	0 9
(E) Wet Wt. (lb/ft ³)	1.26.0	12) S	121.5	0.2 C1	0 771 0.2 01	, CO.	100 5	000
(F) Moist Count	1156	79 11	10 99	7011	0.61	_		1 1 1 0
(G) F ÷ Std Count				3.	3	1	2.5	97
(-41/ +3;0W +M (H)	220	1, 00						·
(10.1)		7	1	23.5	نم 22	D. X	17,8	2 (
(I) Dry Wt.	X" 70	1641	105.8	1611.3	104,8	104,7	0 :01	10 < 0
(J) Proctor Wt.	100	00/	(,) 0/	0.07	7 U £			
(K) % Comp. I ÷ J	00/	160	34	1 P				
(L) % Moist					CINE	(4.1)		
(1)	19-100	19420	54/3	51.772	57750	08750	65 4 36	V 0 / 1 % 0
(N) LOCAETON	30.24		77.08	Care in the second second second second	75%			7
(0) Elev.	250	75/	733	724	7 1/1	121		to the same
					3		15 2	from f

Compaction Required / 8 U

Technician (/1)

PROJECT: JOB NO.: AREA:

DATE: 9-22-73 DENSITY STANDARD COUNT:

			22					
(A) Test Number	5	Q /		į,	N	, **	100	11
(8) Probe Depth	7					8		<u> </u>
(C) Dens. Count	424	430	460	794	87/2	416	4.14	
(D) C ÷ Std Count			**		8 9		,	۹.
(E) Wet Wt. (lb/ft3)	1280	127.0	123,15	23.5	127.5	0% 01	(0,50)	
(F) Moist Count	1162	カコニ	278		89/1	1202	0.25	ジング
(G) F ÷ Std Count	æ.	s	348 25	*			×	
(H) Wt. Moist (1bs)	2.8.2	23.2	(,%,	22.4	23.6	240	861	0,81
(I) Dry Wt.	104.8	103.8	8 301	(5.0)	164		1092	1000
(J) Proctor Wt.	163	(6)	(9)	(2)	. ₹.0/			, 0
(K) % Comp. I ÷ j	/ (1)	1611	760	. 09/	. 700	7.9.1	Ų	<0
(L) % Moist			21 ₂₀			10 (2) (2) (2) (3) (4) (4) (4) (4) (4) (4) (4) (4) (4) (4		
(N) Location	02.70/	schal	50700	20-160	にナンへ	09727	26122.46	346
	100.74		and recipies to the second	Control of the Contro	30,7	A commence and a second and a second assessment	87581	And the second s
(0) Elev.	722	233	138	20.5	7.42	7.67	151	7.873
							7	

Compaction Required

Technician

PROJECT: JOB NO.: AREA:

DATE: 0-2/- 7 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

							S	HELL
(A) Test Number		d	w	4	Ŋ	8	1	90
(B) Probe Depth	•	=	** ***					
(C) Dens. Count	430	420	440	478	450	250	400	4/2
(D) C ÷ Std Count)	
(E) Wet Wt. (1b/ft3) 1272	2721	128.5	126.0	127.5	1245	123.2	1210	1287
(F) Moist Count	1100	1080		1260	08//	12.00		080%
(G) F ÷ Std Count	E 3	2.	15.7 1	6 N N N N N N N N N N N N N N N N N N N				
(H) Wt. Moist (1bs)	21.7	21.3	28.7	254	23,5	20.00	27.5	610
(I) Dry Wt.	705.5	1072	973	1.20	1.01.0	896	11995	10 X A
(j) Proctor Wt.	103	105	94	46	90	4.	1.01	5 8
(K) % Comp. I ÷ J	207	702	007	100	001	100		
(L) % Moist	2 7	e by			SII			
(N) Location	20+00	20+15	01700	21.120	00+20			
	25 17 8-	A	- 317,50		1	本	100 FX	· 4
(0) Elev.	730	729	734	753	7.42	141	7.52	1.51
					197	1	The same of the sa	

Compaction Required 100

Technician //

いとからい PROJECT: JOB NO.: AREA:

DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT: 92123

		,		•				
(A) Test Number	5	07		(2)	7	12.		
(R) Dunha Danth	27 7)		muliyation of	3	¢3/.
ייים הפקידור	1		i i					i
(C) Dens. Count	448	454	456	442	438	877	4.52	464
(D) C + Std Count								
(E) Wet Wt. (1b/ft3)	125,0	12 40	1240	125.51	126.6	1250	123,5	(38)
(F) Moist Count	1212	1194	1216	7811	1232	8701	10.3.0	2000
(G) F ÷ Std Count			=03	-				3 707
(H) Wt. Moist (1bs)	74.5	23,8	24.6	7:87	24.8	2 /. /	6.61	00/
(I) Dry Wt.	100.8	7001	1566	101.9	- 1	2001	163 6	/ 2 4/
(J) Proctor Wt.	99/	901	86	100	100	SX/	103	100/
(K) % Comp. I ÷ Ĵ	100	, 00/	700	100	700	100	×1:17	46.
(L) % Moist					23			2
(N) Location	es+8/	09+81	18+72	20+00	245	22 +00	23753	22765
	80,74	1	125.54	The formation	1102 6.	A commence of the commence of	1004.2	de distante en en en en en en en en
(0) Elev.	656	256	759	325	762	761	76.2	17.
	*	*		7				3

Compaction Required

CONFIDENTIAL BUSINESS INFORMATION

Technician

PROJECT: JOB NO.: AREA:

:								
(A) Test Number	0	0/	11	12	13	4	15	5
(8) Probe Depth	2 5			2 CONT				
(C) Dens. Count		u	5 11 11 11 11 11 11 11 11 11 11 11 11 11				497	I Li
(D) C ÷ Std Count							10	
(E) Wet Wt. (1b/ft ³)	126.0	1272	1282	1295	1787	1265	0.	1 1-7
(F) Moist Count	1100	1060	0111	1030	1060	000	C. F.	173.0
(G) F ÷ Std Count			85.24 35 37					
(H) Wt. Moist (1bs)	21.7	20.8	220	20.1	20.8	60	100	2000
(I) Dry Wt.	1033	106.4	106.2	109.4	1079	1776	100	1-10
(J) Proctor Wt.	103	103	103	701	90/	100	1.0	V Page
(K) % Comp. I ÷ J	101	103	103	103	-201	100	100	24
(L) % Moist		2 9 0	10 2 3 3					
, 1 (N)	27770	しツャンク	36736	201720	1/37 00	50.00		; e
(II) Location	200 RT 9-	10 A	200 R19-1-1		190 02)	こうナウィ	- 6 - 1 - 1
(0) Elev.	725	726	723	722	1 2001			1 66
	7.6							

Compaction Required

PROJECT: JOB NO.: AREA:

DATE: 2-20-7: DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

			e					
(A) Test Number		7	۲	X	7.		1	R.
(B) Probe Depth		∄ _⊙	201. 201. 201		5.		8	1 2
(C) Dens. Count	436	420	01 t	4	242	348	-	Z 12 Z
(D) C ÷ Std Count) II Mai						
(E) Wet Wt. (1b/ft ³)	126.5	128,5	131.0	12.90	1287	125.0	1 10	1.27
(F) Moist Count	1160	1080	1000	1.00	X 2 2.1	0.00	2 St.	 & & & & & & & & & & & & & & & & & & &
(G) F ÷ Std Count	P H		5				÷	, i
(H) Wt. Moist (1bs)	23.2	12 1 20	19.3	2/ "	250	73.0	7.7	1 1 1
(I) Dry Wt.	1033	1072	1.11	1073	1001	263	1 m	
(J) Proctor Wt.	100	107	307	101	20.0	38.	37.	\(\frac{1}{2}\)
(K) % Comp. I ÷ J	104	401	104.	103	102	701		2007
(L) % Moist	2.	8 1	1					
	34		ď	. l.		28		
(N) Location	00000	000000	23-52	シャンハ	2/150	0117	05-761	19465
	3 17 001	4	-31706	A	100172-		105419 -	Manager .
(0) Elev.	12 th 5	1	728	737	733	132	971	101
			•			*		The state of the s

Compaction Required

CONFIDENTIAL BUSINESS INFORMATION

Technician

PROJECT: LATING SPORT JOB NO.: SPORT JOHN

DATE: 742073 DENSITY STANDARD COUNT: 231 MOISTURE STANDARD COUNT: 1271

							1	
(A) Test Number		7	Μ	X	4	7		4
(B) Probe Depth	29	18 18 18 18			. (*) 			
(C) Dens. Count	396	408	994	444	412	406	458	448
(D) C ÷ Std Count		e a						
(E) Wet Wt. (1b/ft3)	131.5	130,5	123,0	125.5	129,5	130,0	103.5	125.0
(F) Moist Count	1138	2601	8911	001/	1684	1/20	70//	1088
(G) F ÷ Std Count	± 4		= 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0			22 72		
(H) Wt. Moist (1bs)	22.7	21,5	22.6	21.7	٣ - ٢	22.2	0.22	21,5
(I) Dry Wt.	108.8	108,5	1.6 1, 0	163.8.	108,2	8.601	1005	1.03 5"
(J) Proctor Wt.	801	80/	0.97	a 0/	801	. ७ ० /	100	00/
(K) % Comp. I ÷ J	100	700	00/	9/	100	00/	00)	700
(L) % Moist	s 2)					81 0 7 10		
(N) Location	2+30	2750	2450	3765	25647	0812	6750	6760
	(C)		150°R&		7518	1	130'44	The second secon
(0) Elev.	266	765	764	78.3	263	762	292	181
				700				**************************************

Compaction Required / 00 / 15

Technician UP/TC

> PROJECT: JOB NO.:

DENSITY STANDARD COUNT:

			\$.= 1					
(A) Test Number	6	10/		17	1.3	t.	N	
(8) Probe Depth			11 12 12					
(C) Dens. Count	440	436	.436	478	404	417	426	408
(D) C + Std Count	**	10 mg/	281				2	
(E) Wet Wt. (1b/ft3)	1260	1267	12.7.0	121.5	130.5	129,5	1777	1200
(F) Moist Count	1100	930	1000	920	1030	1050	10101	10 BC
(G) F ÷ Std Count	÷		3	# ## ##				
(H) Wt. Moist (1bs)	21,7	17.8	193	17.5	20.1	20%	7.61	2 - 2
(I) Dry Wt.	104.3	1.08.9	1.07.7	104.0.	1,0,4	1089	1081	1001
(J) Proctor Wt.	103	107	107	103	101	107	107	1001
(K) % Comp. I ÷ J	101	102	101	101	103	102	101	102
(L) % Moist	-		·					
(N) Location	3400	3+20	00+9	01+9	67 30	3400	51+4	00+9
	8017 E-	A	-31700	4	32736	H	4	BRAR
(O) Elev.	765	764-	757	7.56	755	168	767	45.
								*

Compaction Required

Technician //

19-73

PROJECT: WOUS 6

384 Dun

A

1240 102,3 1090 8/ 00 128.0 な DATE: 7-/7/2 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT: 2000 426 00 2 12515 02.20 4.44 147 129.0 2.60 19.6 416 1001 1.4 0/201 A X 108.5 1074 72 897 21.1 16 J. せ M 128.5 a a N27 476 00 80 N \mathcal{C} 16.6 128. 420 200 % | | | E) Wet Wt. (1b/ft³) H) Wt. Moist (1bs) D) C - Std Count G) F + Std Count F) Moist Count Test Number (B) Probe Depth C) Dens. Count J) Proctor Wt. I) Dry Wt.

Technician

22+85

22726

05+120

21 +40

09+02

20120

19725

52+61

(N) Location

125.42

140.64

160,26

13514

4

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155

756

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756

(0) Elev.

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Compaction Required

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/ O O

100

801

00/

100

106

Q

3

900

150

30

700V

K) % Comp. I ÷

L) % Moist

ROJECT: OB NO.: REA:

DATE: (2) (2) (2) DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

A) Tost Mimbox	6	1,1	//	-			
יין זכטר וומוווחבו		0 /		1/2	72	1/2/	
8) Probe Depth				2 H			
C) Dens. Count	432	424	4.28	454	448	452	
D) C + Std Count			i				
E) Wet Wt. (1b/ft3)	127,0	128.6	1275	1240	17510	1245	
F) Moist Count	248	1074	786	270/	1/0/	7901	
G) F ÷ Std Count			G *				
H) Wt. Moist (1bs)	18,5	196	18.5	2.8.1	129	19.61	113
I) Dry Wt.	108.5	108,4	9.80/	6.201	10/1	1049	
J) Proctor Wt.	108) o q	80/	100	\$07		
K) % Comp. I ÷ J	· 49/	00/	00/	99/	7 (10	00/	
L) % Moist		•					
N) Location	23720	22+60	2 4160	24415	22+75	2.2 4.50	
	125'LE	130'66	140,59	1	165'EE	1	
0) Elev.	ナシア	258	758	755	756	727	
	100					-	

ompaction Required

CONFIDENTIAL BUSINESS INFORMATION

Technician UD

DENSITY STANDARD COUNT. MOISTURE STANDARD COUNT

PROJECT: JOB NO.: AREA:

·			11 18:					
(A) Test Number	**************************************	2	ţ.	g.J.	12			(t)
(8) Probe Depth		8 8	- - -					5200
(C) Dens. Count	ひらず	124	496	700	45K	2000	02. 1	000
(D) C ÷ Std Count	,							
(E) Wet Wt. (1b/ft ³)	1035	176.T	11000	Over.	1240	720.2	100.5	1232)
(F) Moist Count	1170	16.80	1060	0801	0077	1050	0:01	(S) (1) (T)
(G) F ÷ Std Count			100					
(H) Wt. Moist (1bs)	234	20.8	: Es	216	21.7	200	661	0.5
(I) Dry Wt.	1001	1.05.3		2.4.6	1032	766	2501	1.72 12
(J) Proctor Wt.	56	105			153	000		100
(K) % Comp. I ÷ J	12.01	100	Ž.	067	100	007	103	200
(L) % Moist		4) 3)					= -	
	: =	6) 6						
(N) Location	5170	1646	20+16	のにナンと	100+50- 163+30	02.700	25/46	17766
	25'RT & -	A	30478	4	35x7 E - 1		-3-80t	A
(0) Elev.	800	801	803	802	800	802	0.00	£

Compaction Required

CONFIDENTIAL BUSINESS INFORMATION

Technician

PROJECT: JOB NO.:

10					ONC. MOIO	- 0		
(A) Test Number	0	10	//	/2/	SOOT CER			
(B) Probe Depth		10 85 11 15 15 15 15 15 15 15 15 15 15 15 15 1	高 (2) (3) (3) (3)					
(C) Dens. Count	396	424.	255	.410	420		*	
(D) C ÷ Std Count		250	11		x - 7.7	is.	₹.	
(E) Wet Wt. (1b/ft3)	131.5	128.6	124.0	129.7	1285			
(F) Moist Count	//80	1210	1030	1640	1100		·	
(G) F ÷ Std Count	50 1	10 The second se	7 W G	0 0 357				
(H) Wt. Moist (1bs)	23.6	24.0	100	20.3	21.7			
(I) Dry Wt.	109.9	103,8	103.9	09.4	8.90/			
(J) Proctor Wt.	901	103	103	901	103			
(K) % Comp. I ÷ J	104	101	101	103	104			
(L) % Moist		22						
**					4. 4.			
(N) Location	0979%	01. +9%	08+96	95+50	12467			×
	ĠJ	517 E	5RT8 .	20878	31816	No.		
(0) Elev.	803	902	80)	208	801			
60	6		100		ومندعت والمتادية			

Compaction Required

Technician

PROJECT: JOB NO.: AREA:

DATE: 77/75
DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

	2.	lov	0	N	8		
		w T	ativ				
	152	2	150	486	50.1		
	9 (1 m) (1 m						
	124.7	1252	0101	120.7	1185		
	1240	980	0.517	11.30	100		
	Maria Maria	12		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		210	
10	2.5.0	18.9	230	22.4	24.2		
0	1.6.6	106.3	980	983	94.3		
V	98	901	980	98.	26		
	00/	100	100	/60	100		12
	A Section 1						
	06+11	12450	12+70	11+20	06.7611		
Δ	Ā	40,878-	<u>A</u>	-31788	4		
	668	700	669	969	565		
8						6	

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required_

Technician

2-5-76

PROJECT: LJ //w 5/ Pz.
JOB NO.:
AREA:

DATE: 9-2-15
DENSITY STANDARD COUNT: 2-11

	- 22							
(A) Test Number	6	0/		12	12	14	1 60	1
(8) Probe Depth	\$)	in .)			Q.
(C) Dens. Count	400	42	434	Note:	454	462	700	
(D) C ÷ Std Count	-	10 12 12 13	8					
(E) Wet Wt. (1b/ft ³)	124.6	126.0	1265	12510	1240	57.77	1250	101.51
(F) Moist Count	1022	1094	01,01	2501	382	77.6	2001	D 9
(G) F ÷ Std Count								0
(H) Wt. Moist (1bs)	6 61	21,5	700	20,8	6.87	1. 31		8.00
(I) Dry Wt.	104, 1	7	105.4	104/20	150	1	(, , , ,	, , , , , , , , , , , , , , , , , , ,
(J) Proctor Wt.	163	(3)	3	50/ /			7/0/	7 7 7
(K) % Comp. I ÷ J	, 0,Ç	///	00/	\$ O/	7.03	7.0%	/00/	
(L) % Moist	£.			- i		2		
(N) Location	21450	09+17	22+75	23405	034/20	0 m 7	76450	26-162
12	2612 E	1	7,701		770216-		15%	1
(0) Elev.		7 60	711	1.6.5	for !	211.5	764	765
		¥0 /						

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required

Technician

PROJECT: JOB NO.: AREA:

;≱ ≨6. Sir Daw

DATE: 7-77 POR DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

			(9					
(A) Test Number	6	10	77	W.	13	4	1	
(8) Probe Depth		i e			l de			9
(C) Dens. Count	454	430	430	36 6	28+	077		321
(D) C ÷ Std Count		- e 9						i i
(E) Wet Wt. (1b/ft3)	2.42	1272	2221	; ;	2/01	2.5	1215	
(F) Moist Count	090/	5) == 2) 4						()
(G) F ÷ Std Count	s Šī	397 30 40 11	13.10	1050	0304	1001	~	
(H) Wt. Moist (1bs)	20.8	27.2	115	3) (2	. 5.3 5.	Ç	ξ. (C.	
(I) Dry Wt.	1034	105.0	106.1		FO 7	7. 5. 7.	6.001	
(J) Proctor Wt.	201	15.5	103	ţr			145	
(K) % Comp. I ÷ J	103	102	£0/	/03				
(L) % Moist		1	180			1(0)		
		2	2 Si	7				· ce
(N) Location	ングナシン	1		\\ \frac{1}{1}			2	
	カーニ	Ĺ '	15-27 9	/ 	G.	1	:- :-	
(0) Elev.		7.52	749	· .	4	i,V	1.) J.	27
		*:						200

Compaction Required

Technician

GEURGIA FUWER CUMPANY — SUILS NUCLEAR DENSITY METER WORK SHEET

PROJÉCT: JOB NO.: AREA:

DATE: (2) /2 /2 / DENSITY STANDARD COUNT:

<u>ن</u>								
(A) Test Number		- 40 - 40 - 40 - 40 - 40 - 40 - 40 - 40	5	40	6	0	1	(0)
(8) Probe Depth			© #	es th				
(C) Dens. Count	S S	7	9	480	540	520	4/8	() () ()
(D) C ÷ Std Count	·	28 28	:1					
(E) Wet Wt. (15/ft ³)	0.121.0	128.7		27/21	1.4.1	1157	77.87	127.0
(F) Moist Count	1240	0 10	(E)	1060	1060	oret	3101	30.77
(G) F ÷ Std Count			15 m	# # # # # # # # # # # # # # # # # # #				1 1 2 1
(H) Wt. Moist (1bs)	0.5.2	6.4	691	212	7.0.4	675	C	3
(I) Dry Wt.	1050	1/5.8	991	/ 00.0	34.0	76.7	1.601	だとつ
(J) Proctor Wt.	10 +	100	9.6	78	20	0.00	107	107
(K) % Comp. I ÷ J	105	5	1.21	.22/	00/	7.07	701	101
(L) % Moist			5.	3 (S)	*8			
(N) Location	25400	1971.50	02150		05:17	01712	22/50	ここさつこ
	100 178-	4	5 . 7 . 7 . 7	Le	7 77 77	1	50 KT 2-	1
(0) Elev.	748	7177	9/-	いかい	2.7	5:1	745	

Compaction Required

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Technician

CONFIDENTIAL BUSINESS INFORMATION

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GLUNGIA FUWER CUMPANT - SUILS NUCLEAR DENSITY METER WORK SHEET

PROJECT: WYDNSler JOB NO.: KER DOMEN

DATE: 9-22-73
DENSITY STANDARD COUNT: 25

		S 121	-					
(A) Test Number	<u>-</u>	q1		7/	M	14	14	11
(8) Probe Depth	÷ 9	#1			a			0,
(C) Dens. Count	424	420	460	99/	425	316	24.7	1. 1. 4.5
(D) C ÷ Std Count		¥.		h			, ,	
(E) Wet Wt. (1b/ft ³)	. 125.0	127.0	123.5	2.27	27.5	0.6 61	0.461	1.5.1
(F) Moist Count	79//	11 (4.	71.7	22//	22.77	1.56.7		
(G) F ÷ Std Count							5	
(H) Wt. Moist (1bs)	2 % 2	22.2	787	22.4	2.2.4	240	17.8	18,0
(I) Dry Wt.	540	11 3 8	106 3	À		101	109.2	
(J) Proctor Wt.	10>	/c>	C9/	がフィー	762			
(K) % Comp. I ÷ J	/40	. 46B	7.85	Q0/	= 780		- 1.	7.30
(L) % Moist	E			11				
(N) Location	22.2	scha	50/20		57.77	09/122	3442 SULY S	24/16
	100/2		W. Carrier	A	70,75	La commence de la com	12274	1
(0) Elev.	732	733	156	1770	23/2	たらへ	5	
		27			7			

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Compaction Required

Technician (///

PROJECT: (1) A) LO S /C) JOB NO.:

DENSITY STANDARD COUNT:

(A) Test Number		2.	8	1	5	3	brama	
(B) Probe Depth	'' ")	Į.			1 H2		122	
(C) Dens. Count	202	777	456	4.62	871	454	1.25 A	シベナ
(D) C + Std Count								
(E) Wet Wt. (1b/ft ³)	5.8/1	123.0	134.0	123.5	125.0	124,0	1250	277
(F) Moist Count	130	1137	8911	1126	5.601	1062	1028	700
(G) F : Std Count			3	2 1 2 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2				
(H) Wt. Moist (1bs)	0.52	22,4	27, Y	22,4	7112	800	20.02	C 1 x
(I) Dry Wt.	95.5	1001	1000		105.3	103.2	104.7	1 7 ×
(J) Proctor Wt.	901	100	001	0 %/	3	50%	501	107
(K) % Comp. I ÷ J	26	160	0a7	38/	1,00	765	111/	(,0)
(L) % Moist	Rekelle	100	and the second s	American Company of the State o				3
(N) Cocation	23+50, 53+60	23460	25+00	25.420	23400	22+25	00+61	19+20
	23014	A Company of the Comp	27528	1	25056	Marie Control	77.791	(
(0) Elev.	141	747	742	743	740	739	758	759
					11			+

Compaction Required_

Technician

OROJECT: WAWsley

DATE: 7 4 1) DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT: 9-21-73

	SI.						siñ	
(A) Test Number	~	7	ŋ	¥	5	9	7	a
(B) Probe Depth	9							
(C) Dens. Count	438	426	434	448	430	470	717	460
(D) C + Std Count			4 3 7				2	
(E) Wet Wt. (1b/ft ³)	1260	127.5	121.5	125.0	07.01	1.00	122,5	1030
(F) Moist Count	1156	3 9 11	10 98		021	97.7	1	7) 6.
(G) F ÷ Std Count	8 1 19 05		il.				0	2
(H) Wt. Moist (1bs)	23,7	73.4	21.7	23.2	27.12	& CI	9 61 361	4 (
(I) Dry Wt.	102.8	104.1	10.5.8	161.5.	161.5.104.8	104,7	0.00	1050
(J) Proctor Wt.	100	007	C 5 0/	007	16.5 %			
(K) % Comp. I ÷ J	(%)	0.91	99/		1 3/7	76.5	7.87.5	
(L) % Moist		De.	,					
N .	90) (cl	19-120	21450	17/2	234	4	03 7.5 4.5	2012
	30.24	Control of the contro	77.08	A	1286		F 7.07	
0) Elev.	つくし	151	733	454	250	7	157	

)) 2 8 Compaction Required

CONFIDENTIAL BUSINESS INFORMATION

Technician

GEOKGIA FUWER CUMPANY — SUILS NUCLEAR DENSITY METER WORK SHEET

PROJECT: JOB NO.:

DATE: 9-2/-73DENSITY STANDARD COUNT:

			80						
(A) Test Number		7	3	6	2	8	~	€0	-
(8) Probe Depth		M.		4 4 6					-
(C) Dens. Count	430	4.20	440	478	450	294	400	4/8	
(D) C ÷ Std Count	· ·		2			11 20 3000			4
(E) Wet Wt. (1b/ft ³)	127.2	122.5	126.0	727.5	1245	123.2	0751	128.7	
(F) Moist Count	1100	1080	Đ.	1266	୍ଟର/	1300	260/	0807	- 5
(G) F ÷ Std Count									·····
(H) Wt. Moist (1bs)	24.7	21.3	28.7	25.1	5.2.2	264	27.5	213	
(I) Dry Wt.	105.5	1072	973	96.1	/ 01.0	8.96	1095	7 70	
(J) Proctor Wt.	10.3	105	94	24	ن ن	74	1.0-	13 V	
(K) % Comp. I ÷ J	103	201	700	201	00	00/	13.53		_
(L) % Moist	:				2 (4)		SAT		
(N) Location	00+00	20+15	00418	2112	73+00	2172.6	05/76	, j.	
	25 27 8-	A	- 317,52	4	7		H	4	
(0) Elev.	730	729	124	200	742	741	N. A.	151	
					10				

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Technician

GEORGIA FUNER CUMPANT - SULLS NUCLEAR DENSITY METER WORK SHEET

PROJECT: WANS/ey
JOB NO.: < P D 7177

58 PD 1917

DATE: 9-21-73
DENSITY STANDARD COUNT:

Ç	*		L				
	0/	//	12	12	14	\\ \'	//
(()					contribution of the second		2
844	454	456	745	438	448	467	41.0
·			9				9
(E) Wet Wt. (1b/ft3) 125,0	1240	1240	125.57	126.6	125.0	123.5	123.0
1212	1194	1216	7811	1232	8701	1020	1111
	*		15			2 20/	70/
7,42	23,8	24.6	23.6	24.8	21.1	19.9	661
100,8	1002	49,4	101.9.		1000 9	163.6	102
100	901	86	700		/x 3	103	103
00/	00/	100	700	700	100	/ 1413	190
es+81	09+81	18+75	20+06	2.775	22.400	23450	22765
80,79	1	175.74	and the second s	1102 6.	1	1001.8	
757	*	(24)		762	761	262	18.
	448 448 125,0 125,0 120,8 100,8 100 100 100 100 100 100	448 454 125,0 124,0 1212 1194 1200,8 1002 100,8 100 100 100 100 100 100 100 150 100	448 454 456 125,0 124,0 124,0 1212 1194 1216 24,2 23,8 24,6 100,8 1002 99,4 100 100 100 100 100 100 100 100 100	448 454 456 442 125,0 124,0 124,0 125,5 1212 1194 1216 1182 24,2 23,8 24,6 23.6 100,8 100 2 99,4 101,9 100 100 100 100 100 100 100 100 100 100	448 454 456 442 438 125,0 124,0 124,0 125,5 126,6 1212 1194 1216 1182 1232 24,2 23,8 24,6 23.6 24,8 100,8 100 2 99,4 101,9 101,2 100,8 100 2 99,4 101,9 101,2 100 100 100 100 100 100 100 100 7524 8958 756 7574 101,9 101,2	448 454 456 442 438 448 125,0 124,0 124,0 125,5 126,6 125,0 1212 1194 1216 1182 1232 1068 24,2 23,8 24,6 23.6 24,8 21,1 100.8 100 78 100 100 100 103,9 100 100 100 100 100 100 100 100 100 100	448 454 456 442 438 448 125,0 124,0 124,0 125,5 126,6 125,0 1212 1194 1216 1182 1232 1068 24,2 23,8 24,6 23.6 24,8 21,1 100.8 100 78 100 100 100 103,9 100 100 100 100 100 100 100 100 100 78 100 100 100 100 100 100 100 100 100 100

Compaction Required

Technician

PROJECT: JOB NO.:

DATE: 9/2/73
DENSITY STANDARD COUNT:

	ε		92 20					
(A) Test Number	0	0/	11	12	13	14	1.7	71
(B) Probe Depth	E2)
(C) Dens. Count			20				707	
(D) C + Std Count			: 1				400	
(E) Wet Wt. (1b/ft3)	126.0	1272	128.7	12051	1-861	1765	1.07	1 - 6 - 6 - 6 - 6
(F) Moist Count	1100	1060	0///	1330	0701	000	7077	77.77
(G) F ÷ Std Count	•		3	113	000		00/	200
(H) Wt. Moist (1bs)	21.7	20.8	220	1.02	20.0	189	: 1	2
(I) Dry Wt.	1043	106.4	106.2	1094	1079	11-11	- 10	- 1 6
(J) Proctor Wt.	103	103	103	701	101		1.0.1	1//
(K) % Comp. I ÷ j	101	103	63	63	1001	000	1.902.	0 10
(L) % Moist							7.77	101
	7 1 1 7 9		. 1 (2) 1.					
(N) Location	パロナノウ	76+60	25-475	いったい	05725	22+70	ウェナウス	17 17 17 17 17 17 17 17 17 17 17 17 17 1
	200 N. 1 2-	A	200 R18-	1	190 RIF.	4	180'NT P-	4
(0) Elev.	7.05	726	521	722	021.	10%	2000	1112.
	· v							1

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required_

Technician

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PROJECT: JOB NO.:

DATE: 9-20-73
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

			e E					120
(A) Test Number		7	ŗ	8	V	7	7	
(8) Probe Depth		163	12 5-11					() -
(C) Dens. Count	436	€ 7.	·00/	- f	442	348		400
(D) C ÷ Std Count			-					0
(E) Wet Wt. (1b/ft ³)	. 1265	- (- - (- - (-) - (-)	131.0	0861	178	1250		1.201
(F) Moist Count	1.60	05.7	1000	0.0.7	7 · · · · · · · · · · · · · · · · · · ·	12 60		1000
(G) F ÷ Std Count			1					00/
(H) Wt. Moist (1bs)	23.2	gr.	19.3	217	255	n 5. a	160 160	. 7. 2
(I) Dry Wt.	6501	107.01	1 1 1 1 1 1	1073.	559	106	1-136	
(J) Proctor Wt.	3 <i>0 /</i>	103	00 C.	104	38	200	36	/50/
(K) % Comp. I ÷ J	103	104	104	103	707	1001	200	307
(L) % Moist		8						
(N) Location	0 at 10		08t x4	23+35	21480	\D\.	10.150	3
	100178-	A	3 1706	A	- 3L7001	1/10 1-10	175718	107
(0) Elev.	4	0-1	L a	737	733	7.87	37/	1
			8					

Technician

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required

PROJECT: UJ7/20 SI P. JOB NO.: S.P. 17/20 SI P.

DATE: 74-20-73
DENSITY STANDARD COUNT: 23
MOISTURE STANDARD COUNT: 12

			F.,					
(A) Test Number		7	7	X	1	7	1	0
(8) Probe Depth	9		H B		1	5		V
(C) Dens. Count	396	408	466	444	412	466	408	440
(D) C ÷ Std Count		(4) 10 383 70	10 20 10 10 10 10 10 10 10 10 10 10 10 10 10					0//
(E) Wet Wt. (1b/ft ³)	131.5	130,0	123,0	125.5	12.9.5	1295 1300	0	12/1
(F) Moist Count	1138	7601	8911	1/00	14.84	1120	1/4/	1300
(G) F + Std Count	33 13	2.		2	0 0	7/20	1100	1088
(H) Wt. Moist (1bs)	22.7	21,5	22,6	21.7	W. K	27.7.	77.0	216
(I) Dry Wt.	108.8	108,5	0101	163.8	2.80/	0 671		1 1
(J) Proctor Wt.	801	80./	700	700	801	0.00	1000	10 x 51
(K) % Comp. I ÷ J	100	,00/	700	(%)	04/	(0)		
(L) % Moist	8	8					0.0)	00/
(N) Location	2+30	2750	05+2	3+65	25644	0848	0579	09+9
	(c)		150º Rd	1	78. Kg	1	15	A comment
(0) Elev.	746	765	764	7.9%	263	70,5	77.2	177
					T			

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Compaction Required_

Technician VETO

PROJECT: JOB NO.:

127 DATE: P - ZO - 73DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

		=3 3 ¥0						
(A) Test Number	6	70	3 %	17	1.3	*) 6	
(8) Probe Depth						4		2 24
(C) Dens. Count	440	436	. 430	478	404	417	700	000
(D) C ÷ Std Count		* *				7	O.≫.	000
(E) Wet Wt. (15/ft ³)	7.7260	126.7	1270	121.5	130.5	1295	ha la	1200
(F) Moist Count	1.100	930	1000	92.0	1030	1050		0.000
(G) F ÷ Std Count				10) 4. 1.		000	0/0/	703
(H) Wt. Moist (1bs)	217	17.8	1923	175	1.40	700	6	2
(I) Dry Wt.	104.3	1.08.9	107.7	104.0.	104	1080	1001	0:10
(J) Proctor Wt.	103	701	107	103	101	107		108.7
(K) % Comp. I ÷ J	191	102	101	101	103	101	101	101
(L) % Moist		***		a a)
(N) Location	3400	3420	0019	0, +9	08 +4	2400	-+ t	00+7
	8017 8-	4	-3 1706	4	32736	17	2 4	8 578
(0) Elev.	765	197	757	252	755	89/	191	727

Compaction Required

Technician

5 (P A M. M.

DATE: 9 20-73 DENSITY STANDARD COUNT: 237 MOISTURE STANDARD COUNT: 227

3			1.00						
(A) Test Number	6	70	//	12/	57:	14	,	B	
(8) Probe Depth		e.							
(C) Dens. Count	432	424	428	454	877	6112			
(D) C ÷ Std Count	= =1				9 / /	3			
(E) Wet Wt. (1b/ft ³)	9'CCI .	128.0	127.5	1240	17510	134			 -
(F) Moist Count	948	1014	786	7072	10 11	7907			
(G) F ÷ Std Count			2 2	10 10 34				. 2	
(H) Wt. Moist (1bs)	18.5	19.6	18.5	7 10 1	29.9	181		***************************************	
(I) Dry Wt.	108,5	108,4	1086	102.9	10.01	1049			
(J) Proctor Wt.	108	801	80/	707	201	, , , , ,		8	
(K) % Comp. I ÷ j	70/	00/	1.60	9//	4000	10/			
(L) % Moist				i D	2	2			G
(N) Location	23720	23+60	2 4166	24415	22+75	22 + 60			
	125'26	130'EL	140,24	1	165124	1			
(0) Elev.	757	85.6	326	725	756	727			
						30			

Sompaction Required / ()

Technician (12)

GEORGIA FUWER CUMPANY — SUILS NUCLEAR DENSITY METER WORK SHEET

PROJECT: W. Sel- 1211

DATE: 7-73
DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT 9-19-73

(A) Test Number	/	7	n	X	4	9	7	0
(B) Probe Depth	=		84°					
(C) Dens. Count	420	426	422	1/48	117	4.44	471	1100
(D) C ÷ Std Count							9	207
(E) Wet Wt. (lb/ft3)	5.821	128.5	128.5	135,0	129.0	2301.	1280	1340
(F) Moist Count	088	1822	4201	76//	1001	1.6.21	3 3	
(G) F ÷ Std Count					5	74/	4014	3/0/
(H) Wt. Moist (1bs)	16,6	19.9	21.1	23.6	19.61	727	191.	717
(I) Dry Wt.	11,9	108.6	4,001.	101,4	7 501	107.0	10 % 4	10.0
(J) Proctor Wt.	80/	80	8 4/	7.60	× 0 /		2 7	1111
(K) % Comp. I ÷ J	00/	130	(0.6	01/	2/31	200	00/) 0 / .
(L) % Moist		Ų			2		55	700
(N) Location	52+61	19735	2012	50+02	21 +40	05+12	22775	23+65
i.	13514.	The second of th	77,091	<u> </u>	F7.0p1	-	P7.521	1
(0) Elev.	756	556	756	755	751	55.7	756	727
120						,		,

CONFIDENTIAL BUSINESS INFORMATION

Technician

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Compaction Required_

GEUKGIA PUWEK CUMPANY — SOILS NUCLEAR DENSITY METER WORK SHEET

PROJECT: 1A4 ANUS.E. JOB NO.:

DATE: 7/3/75
DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

			*					
(A) Test Number	s 1	7	ĵΛ.	6	5	y	290	
(8) Probe Depth		X .	14 (4)			22		
(C) Dens. Count	462	325	946	1480	201	1-05		
(D) C ÷ Std Count			IL.		Ti			
(E) Wet Wt. (1b/ft ³)	2.5.7.1	124.7	7:5:7	012	120.7	118.5		
(F) Moist Count	1200	1240	780	1750	11.30	101		
(G) F ÷ Std Count	28		2					
(H) Wt. Moist (1bs)	200	25.0	18.9	50,00	22.4	24.7		
(I) Dry Wt.	99.2	777	106.30	78.0.	983	942		
(J) Proctor Wt.	900	ે જ	9.51	7.86	98.	26		
(K) % Comp. I ÷ J	700	100	100	100	99/	100	5	
(L) % Moist		a. Ex						
(N) Location	1725	11-140	12450	12 +70	11+20	06+11		
8	30 KT E -	h T	W.M.E.	4	-31788	4		
(0) Elev.	869	669	2007	669	969	565		
	1	70						

CONFIDENTIAL BUSINESS INFORMATION

Technician

Compaction Required

GEORGIA POWER COMPANY - SOILS NUCLEAR DENSITY METER WORK SHEET

PROJECT: JOB NO.: AREA:

DATE:
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT: 3-1272

ن								20
(A) Test Number	0	0/	11	2	7	701		1/4
(8) Probe Depth	59	8	31 11.		25 25 37 37 37			
(C) Dens. Count	250	466	472	894	785	474	2 80	470
(D) C ÷ Std Count								
(E) Wet Wt. (1b/ft ³)	124,0	123.0	122,0	122,0	12/6	. 122.0	121.0	(21,5)
(F) Moist Count	カとニ	1242	1202	7/21	4511	1042		10/1
(G) F ÷ Std Count		S 37		11 12 10 11 12				
(H) Wt. Moist (1bs)	p122	25.0	24.0	24,6	27,0	20.8	2 - 3	276
(I) Dry Wt.	101,6	98.0	0.86	4.66	0.85	7-10	997	, 5 % (5
(J) Proctor Wt.	ر ا	96	9 8	26	85	30	3.6	36
(K) % Comp. I ÷ J	100	00/	.00/	001	740	100	FNO:	
(L) % Moist		8					2.0	
(N)	es 40/	0740/	20175	10485	25411	11 = 30	SCHIL	1 7 7 7 7
	45/Rf	55 PL	A Committee		60'R 2	And the state of t	- 77.0Z	
(0) Elev.	C 8 9	887	(S. 2)	(8)	5 2 5	919	7.3.5	
						+	}	+

Compaction Required_

Technician (

GEURGIA POWER COMPANY — SOILS NUCLEAR DENSITY METER WORK SHEET

PROJECT: JOB NO.: AREA:

DATE: 7 - 77-77 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT 2)

			D4					
(A) Test Number	~	2	η	A	4	9	7	7
(8) Probe Depth	, 9		iii i					
(C) Dens. Count	462	456	4.86	サレ カ・	867	0.17	47,	472
(D) C ÷ Std Count	-	= 1						
(E) Wet Wt. (1b/ft ³)	123,0	124.0	120,5	122.0	122.0 121.5	1246	0 (()	100
(F) Moist Count	8/01	1032	97 <i>(</i> -	556	060	1122	1	0.10
(G) F ÷ Std Count		** **		e N		71		(to 0 feature)
(H) Wt. Moist (1bs)	27.72	22.4	17,8	2,8,	21.5	100 426	24.1	29.3
(I) Dry Wt.	100.8	101.6	102.7	103,5	100	8 1 91	6 101 8 101	C (0)
(J) Proctor Wt.	700	1 00	20/	40/	100	001	1007	0 / 0
(K) % Comp. I ÷ J	(C)()	100	00/	001	001	00/	(0.7	
(L) % Moist		=				N 225		
20 1 (N)	00 FC2	51+52	22+ab	224/0	20176	20185	19456	1. 1. 3
	120'KE	100'R 2	•	45'RA	100'29	1		*
(0) Elev.	744	745	742	743	742	ンダン	0/2 (160
		in the state of th						, ,

Compaction Required

CONFIDENTIAL BUSINESS INFORMATION

Technician

PROJECT: JOB NO.: AREA: SEP. DAIM

DATE: 0 - 11 - 73 2 3 1 DENSITY STANDARD COUNT: 2 3 1

,		2							,
(A) Test Number	0	1/0	11	77	//3	14	1	1	,
(B) Probe Depth	17 9						2	2	
(C) Dens. Count	450	432	438	40.6	420	1175	200	1	
(D) C ÷ Std Count						177	570	408	
(E) Wet Wt. (1b/ft3)	5,74,5	127.0	176.0	0.24/	100	30.	100		
(F) Moist Count	1112	10.76	866	1040		1827	183	1	
(G) F ÷ Std Count		9 8	9	- S.	0	2 7 2	100	1100	
(H) Wt. Moist (1bs)	220	213	19.3	20.3	2	5	77 (,		***************************************
(I) Dry Wt.	102.5	L1501	C 901	1047		- 5	7 × 0	1.6.7	-
(J) Proctor Wt.	00/	702	201	165		1080	2 =	7000	+
(K) % Comp. I ÷ J	F00/	/60	007	001	04/	2 2 2	10 N	(10)	
(L) % Moist)	=		2	000	200	
(N) Location	wt5/	15415	201.05	2.1100	22785	5.760	2590	21625	
, and the second se	125'RE		150'RE	A CONTRACTOR OF THE PARTY OF TH	790'RE-		4.24	1	
(0) Elev.	734	226	JE1	737	737	200		77.6	
			1 as			~	7	0,6	

Compaction Required /00%

Technician VD/TC

PROJECT: JOB NO.: SeP おか

Parlin

DATE: 9-//-73
DENSITY STANDARD COUNT: 23/

(A) Test Number		7	>	A	4	2	7	4
(8) Probe Depth	,,,9		36 37 37 38	•				7
(C) Dens. Count	390	412	462	418	430	47 6	4.34	458
(D) C = Std Count	· 8	887	20)		0 //
(E) Wet Wt. (1b/ft ³)	132.5	129.5	131.0	128,5	127,0	1286	>701	132
(F) Moist Count	974	386	904	776	1050	07/1	3501	1674
(G) F ÷ Std Count	*1					9		
(H) Wt. Moist (1bs)	18,7	19,1	121	18,0	20,6	23.2	21,7	21.12
(I) Dry Wt.	113,8	110,4	1139	110,5.	106.4	104,8	104.8	107 4
(J) Proctor Wt.	0/1	1(0	1/0	011	105	,105	201	N.S. A.
(K) % Comp. I ÷ J	7.00	1001	100	707	00/	150	/07	86
(L) % Moist					Rep. 1/2	1/07/)	
(N) Location	12+25	12+35	12+21	13+60	14425	18436	14/20	1420
	120,24-	1	90.22	1	200'R&-	1	150,29	Annual instrumental and and
(0) Elev.	769	693	692	139	959	589	678	46.9
		RE 1	_		*			,

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Compaction Required_

GEUKGIA FUWEK CUMPANY — SUILS NUCLEAR DENSITY METER WORK SHEET

DATE: // I() / CDUNT: DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

PROJECT: PHINT WITH SILLY AREA: SEPONDED

(A) Test Number	6	0/		12	13	14	15	11/2
(8) Probe Depth	Ginches	4	1	4	4	1	4) 4
(C) Dens. Count	400	A may been a	486	466	4/8	724	450	4.34
(D) C ÷ Std Count	11	8)
(E) Wet Wt. (1b/ft ³)	1287	1270	120.5	1221	1861	1777	1537	1701
(F) Moist Count	1050	920	930	0630	1020	000	000	ラシュ
(G) F ÷ Std Count	B 1							
(H) Wt. Moist (1bs)	9.02	17.5	1.8	691	66/	180	192	112
(I) Dry Wt.	9/0/	109.5	1001	1000 1000	1000/	0000	1000	1000
(J) Proctor Wt.	108	000	103	103	500	000	100	なり
(K) % Comp. I ÷ J	001	001		100/		001		TO STATE OF
(L) % Moist						,		
: =								ı.Bi
(N) Location	ナ <u>の</u> つくで	のナーの	00+0	5+15		465		10.10.
	47700	4	A	A	1001878	1341001	12/1/01	10/4×21
(0) Elev.	164	(6.3	755	754	760	158	750	87

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required_

Technician

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GEORGIA FUNER CUMPANY — SUILS NUCLEAR DENSITY METER WORK SHEET

PROJECT: JOB NO.:

	-							1060									
	0		A 20 X		1277	066		161	1 89	,,,,,,	700		3.5	19425	4	Ø: 1	00
	7		200	0	1260	1000		192	1,00,7	106	700	201		19-400	215 17 8-	642	(m/
	9		274		1222	980		601	109.3	103	()0//			20465	4	722	
	l)		454		124.7	920		175	1067	106	00%			20+40	230'47 8-	723	
	4		454		1202	840		15.7	108.5	80/	100			22+40	A	735	
	Λ,	1 2	966	g	119.0	096	54	185	2001	100	/0.0	п	un e	22,425	220LT & -	736	
	N		424	6 ·	128.0	0/01	·	19.6	480/	/7 p	100			2.91.37	4	739	
			434	r E	126.7	900		17.1	109.6	& <i>\\</i>	100		9		250179-	739	
1-1	(A) Test Number	(B) Probe Depth	(C) Dens. Count	(D) C ÷ Std Count	(E) Wet Wt. (1b/ft3)	(F) Moist Count	(G) F ÷ Std Count	(H) Wt. Moist (1bs)	(I) Dry Wt.	(J) Proctor Wt.	(K) % Comp. I ÷ J	(L) % Moist		(N) Location		(0) Elev.	

Compaction Required

Technician

CONFIDENTIAL BUSINESS INFORMATION

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MINDIN

PROJECT: JOB NO.: AREA:

DATE: 7 - / 6.) ?
DENSITY STANDARD COUNT:

			20 M						
(A) Test Number		2	~	. 4	4	>	0	SZ.	2 %
(8) Probe Depth	" 9	A	100	=			27		=
(C) Dens. Count	900	418	997	.457	364	470	707	17	
(D) C ÷ Std Count								: :	
(E) Wet Wt. (1b/ft ³)	121.5	177.5	1230	12415	1215	1220	126.5	103 B	
(F) Moist Count	926	848	786	5.86			1 3 3 3 4	Port.	
(G) F ÷ Std Count	П	3.							
(H) Wt. Moist (1bs)	0.81	15.8) '61	1	1.61	121	> 10	5	
(I) Dry Wt.	という	161,7	103,9	102,4	105,4	102 P	0 66		
(J) Proctor Wt.		1931	C3/	72/	201	10.3	\\ \C_1		
(K) % Comp. I ÷ J	1	091	166	700	7.06	100	1.6 %	14/7	
(L) % Moist	ħ.								*:
(N) Location	05+11	13-4-CI			02/2/	13+50	13/7		
	10 d jo	7 4.55	10/24		100,001	30.26		Troping	
(0) E1ev.	650	683	156	769	767	. I	690	157	
				The state of the s	>)	-	

CONFIDENTIAL BUSINESS INFORMATION

Technician C 🖰

Compaction Required

DATE: 7/0/73
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

ON BY

PROJECT: JOB NO.:

AREA:

5 7 10 1 9 9年 (E) Wet Wt. (16/ft3) 125 (H) Wt. Moist (1bs) (D) C + Std Count G) F + Std Count (K) % Comp. I ÷ J (C) Dens. Count (F) Moist Count (A) Test Number (B) Probe Depth J) Proctor Wt. (N) Location (L) % Moist I) Dry Wt. (0) Elev.

CONFIDENTIAL BUSINESS INFORMATION

Technician

Compaction Required

GLUNGIA FUNER CUMPANT - SUILS NUCLEAR DENSITY METER WORK SHEET

PROJECT: JOB NO.: 1 AREA: Se.p

Sep Dan

DATE: 2/0/33
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

(A) Test Number	/	2	M	+	5	0	1	O.
(8) Probe Depth				<i>(</i> *)				
(C) Dens. Count	416	4/0	434	418	440	442.	400	1
(D) C ÷ Std Count			1					
(E) Wet Wt. $(1b/ft^3)''/29.0$	1290	129.5	126.5	128.7	7.70/	125%	264	6:16
(F) Mojst Count	010/	1690	950	980		000	700	196
(G) F ÷ Std Count								
(H) Wt. Moist (1bs)	19.6	215	18.2	(8.3)	0.20	いじ	24 (290
(I) Dry Wt.	109.4	108.0	2801	x 60/	0.50	107	730/	シイン
(J) Proctor Wt.	107	1.07	107	107	0v,	100		
(K) % Comp. I ÷ J	00/	00/	100	100	00/	001	7	1111
(L) % Moist				6) 6) X				
10				=				
(N) Location	120 th	13420	シング	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	52/15/	26/2/	OHES	91420
	とジルニ	4.	N/X/(U-	1	171/125-	1	17/1/2/	净
(0) Elev.	694	693	607	6. 1.	:(72.	725

Compaction Required //

Technician M

3R0JECT: 308 NO.: 30.2 22.2

DENSITY/STANDARD COUNT:

			200°					
(A) Test Number	N'1776	Ton	Eleven	Justy 1	Twelve Thirteen Farter	Farteen	4572 NESTER	SixTollar
(8) Probe Depth			¥					
(C) Dens. Count	416	404	438	.432	436	246		
(D) C ÷ Std Count		T =						
(E) Wet Wt. (1b/ft ³) /29.0	129.0	130.5	126.0	12.70	126.5	1000		I BI
(F) Moist Count	1/50	1210	0611	1200	13/0	0661		
(G) F.+ Std Count		3		2 1 1 1 1 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1				
(H) Wt. Moist (1bs)	23.0	242	238	26.4	776	200		
(I) Dry Wt.	106.0	106.3	102.2	1007	000/	066		The state of the s
(J) Proctor Wt.	103	103	100	1	30	36		
(K) % Comp. I ÷ J	100	. 001	001	100/	1001	001		
(L) % Moist	3	_						
	6	1	6.			£ (
(N) Location	10+00 75 R7 E-	S1.701	11+00 80'87 8	11+15	12400	12+21		
(0) Elev.	069	688		7	789	585		
						+	-	

Sompaction Required

Technician ///

PROJECT: JOB NO.: AREA:

DATE: 9-/0-73
DENSITY STANDARD COUNT:

(A) Test Number		7	W	4	1	V	1	8
(B) Probe Depth			, 13 , 13 , 13 , 13 , 13 , 13 , 13 , 13					
(C) Dens. Count	454	432	512	5/2	436	480	458	275
(D) C + Std Count	·	a .	e e e e e e e e e e e e e e e e e e e			15		
(E) Wet Wt. (1b/ft3) 124.2	124.2	1270 (117.5	(17.5)	126.5	121.2	123.7	125.7
(F) Moist Count	1300	1360	09%	096	016	0.001	808	016
(G) F ÷ Std Count	u u	H W						
(H), Wt. Moist (1bs)	26.4	27.8	18.5	185	17.3	661	15.0	173
(I) Dry Wt.	976	2.66	990	- 0.66	7.601	1013	1087	1084
(J) Proctor Wt.	98.0	98.0	98	98.9	107	98.0	107.0	1020
(K) % Comp. I ÷ Ĵ	/00	100	100	100	100	00/	100	100/
(L) % Moist								
		.ii						
(N) Location	10+50	10+70	00101	10+15	1.450	09+11	05+11 011+20	11+50
	50 KT & -	4	250.K7 & -	A	240 RT 8-	4	5 8406 6	130CT
(0) Elev.	(SS)	089	705	in O.	101	700	66%	705
	STONE STONE			#/				
Compaction Required	100	Dr.	CONFIDEN	CONFIDENTIAL BUSINESS INFORMATION	Technicfan	tan 711.	X	21

14.6 Sereration PROJECT: JOB NO.:

XXI DENSITY STANDARD COUNT:

(A) Test Number		7	(^)	2-1	5	7	1	Q
(8) Probe Depth	- "9	-1	The second second		- Time		\ <u>\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</u>	
(C) Dens. Count	400	410	448	430	4/8	4/9	476	012
(D) C ÷ Std Count			, best with rese truy.		Å		. 12	1/4
(E) Wet Wt. (1b/ft ³) /3/,0	1.131.0	1297	1250	0701	120.7			1
		7,7	477.7	12/12	140.1	1.40.7	1411	1275
(F) Moist Count	1020	1010	990	000/	926	8201	940	1620
(G) F ÷ Std Count	-			and the second			2	000
50								
(H) Wt. Moist (1bs)	6%	1.5.6	/6/	19.3	12.5	661	787	1 00
			**************************************				くう	3

4.40

1601

108.8

2/

1079

6501

0

60

00/

601

601

0.901

30

800

607

J) Proctor Wt.

I) Dry Wt.

00/

00/

0

00

К) % Сощр. I ÷

L) % Moist

8

6

8

3+21

3400

4+65

14+50.

6/49

5400

134.20

3400

(N) Location

15178

4

RT820-

160ET &

150 678

150

(0) Elev.

782

449

748

748

749

CONFIDENTIAL BUSINESS INFORMATION

121 Technician

000 Compaction Required

GEORGIA FUNER CUMPAINT - SUILS NUCLEAR DENSITY METER WORK SHEET

PROJECT: JOB NO.:

DATE: 7-8-75
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT: 9-8-73

			٠						
(A) Test Number	6	0/	111	(%)	13	14	161	14	
(8) Probe Depth	"9	63	0	2		. 11	?	3	
(C) Dens. Count	402	217	408	414	202	48%	497.	473	
(D) C ÷ Std Count		ž.	3			.x		3	
(E) Wet Wt. (1b/ft ³)	13/10	129.5	130.0	1290	1/8,5	.126,5	120.0	122.0	
(F) Moist Count	988	976	296	934	8801	1072	10.01.		
(G) F ÷ Std Count							5/0/	2011	
(H) Wt. Moist (1bs)	1.61	17,8	18,5	17.8	76.3	21.1	20.6	0.00	
(I) Dry Wt.	111,9	11117	27.11		5.82	99.1	28.4	< :: * * * * * * * * * * * * * * * * * *	
(J) Proctor Wt.	0//	0//	011	011	7.5	79	98	36	
(K) % Comp. I ÷ J	(107	700	7/16	00/	100	400	001	1.00	
(L) % Moist			,						
	7+90	08+6	51+9	0179	2016	2410	25+15	2542.5	
(N) Location	952 6	77,901	75/RE	85'R E	Ja.			(8)	
(0) Elev.	732	733	737	735	- 1		793	744	
				100					

Compaction Required

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Technician

OB NO.: Sap Dax

DATE: 9/8/73 DENSITY STANDARD COUNT:

=======================================				15	240				
(A) Test Number	0	€.	٠ <u>٢</u>	4	¥			7	
(B) Probe Depth			್ವಿ	·					
(C) Dens. Count	410	43%	010	134	462	458	405	4.78	
(D) C ÷ Std Count		y y y	-	2004 1 2 2					
(E) Wet Wt. (1b/ft ³)	1285	0251	1297	1257	123.2	7227	118.3	121.5	
(F) Moist Count	980	096	1000	960	02//	1190	0901	1. 40	
(G) F ÷ Std Count			198	8					
(H) Wt. Moist (1bs)	0 0 1	18.5	18	10.00	2.00	23.0	200	スワウ	
(I) Dry Wt.	1/1/2	108.5	110.4	1080	6.1.	かかか	983	1.12	
(J) Proctor Wt.	100	108	1:50		0.07	1001	000	\$ \chi_{\text{\tin}\exiting{\text{\tert{\text{\text{\text{\text{\text{\text{\text{\texi}\text{\text{\ti}\titt{\text{\text{\texi}\text{\texitit{\text{\ti}\titt{\text{\texitit{\text{\texi{\text{\texi{\texi\tin}\titt{\ti}\titt{\texi}\texititt{\texi{\texi{\texi{\texi{\texi{\texi{\texi{\texi{	
(K) % Comp. I ÷ J	100	150	100	101	0.07	00/	1001	1	
(L) % Moist		æ		27					
		#		16	100		.1	14	
(N) Location	8+6	8+60	7+50	7465	23+50	23460	20.450	7. 5.	
	-32.001	- A	75RT8 -	***	E Dain	7		1	
(0) Elev.	730	729	7.25	7:4	730	737	047	729	
								,	

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CONFIDENTIAL BUSINESS INFORMATION

Technician

PROJECT: JOB NO.: AREA:

DATE: 9/8/73
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

	6 5 5							
(A) Test Number	0	0/	11	12	/3	19	14	11
(8) Probe Depth	ja T			7.5				d
(C) Dens. Count	416	404	492	520	420	436	414	477
(D) C ÷ Std Count								7-7-
(E) Wet Wt. (1b/ft ³)	129.0	130.5	119.7	116.7	128.5	1262	1291	780
(F) Moist Count	010/	1030	860	066	980	890	1070	0.04/
(G) F + Std Count							212	0.20
(H) Wt. Moist (1bs)	961	20.1	191	19.1	(6.8)	14.9	661	273
(I) Dry Wt.	108.4	4.011	103.6	9.26	109 6	1	000	10-77
(J) Proctor Wt.	901	1.09	103	900	601		100	107
(K) % Comp. I ÷ J	108	(00/	(301	100	100	100	30/	(00)
(L) % Moist			t= 3					
		ų. Vie	N II	1			ŧ	
(N) Location	200 29 20+	20+15	23+00	23+37	24 450	24+65	23400	
(0) Elev.	731	729	724	737	700-	A	3270	
			0		140	121	154	135

Compaction Required_

Technician

PROJECT: WINDS 6'1/ JOB NO.:

DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

				N.	3			
(A) Test Number	·	2	\sim	1	٢,	9	. (-	8
(B) Probe Depth								
(C) Dens. Count	388	394	396	396. 406	4.8	416	4,00	
(D) C + Std Count		430	£2					
(E) Wet Wt. (1b/ft ³)	132.8	1320	131,5	130,0	1.29.0	17,71 17,10	/3/	
(F) Moist Count	965	1140	1038	176	980	866	776 3001	
(G) F ÷ Std Count	É							2.00
(H) Wt. Moist (1bs)	13,1	676	20,3	15.9	6'81	8/61	193	1- 3
(I) Dry Wt.	115.4	1093	1112			1. 531	1097 1127	1 0.7
(J) Proctor Wt.	7	0//	211	0//	0 //		0//	
(K) % Comp. I ÷ J	1116	00/	7 00	700	00/.	00/	. 90/	(/ / /
(L) % Moist			82			3		
(N) Location	25+6	09/2	5147	7517	0079	5129	17+10	7220
	7516	100%	100,66	26'RE 115'R &	8073	70% 4	1.5. 1.7	12.54
(0) Elev.	760.	759	756	753				7
i				4				Ü

Compaction Required

ジーバン Technician

PROJECT: JOB NO.: AREA: Seperation [

Seperation Dan

DENSITY/STANDARD COUNT:

(A) Test Number	_	N	2	4	7	V	-	£(
(8) Probe Depth	9	A	— A	<u>A</u>	Ť		A 7	
(C) Dens. Count	428	430	404	418	426	488	420	420
(D) C + Std Count	a jijarayaa	and the first test	8	The state of	, service of 1.5	rath.	·	
(E) Wet Wt. (1b/ft ³)	27.2	127.2	130.5	128.7	127.7	1202	128.5	127.2
(F) Moist Count	970	1030	1000	990	808	860	066	1040
(G) F ÷ Std Count) part and		g je sa dipend	e. Angrepin ke ^{nga}	transpared as the		contract association
(H) Wt. Moist (1bs)	18.7	20.1	19.3	/9. /	15.0	14.1	1.6.	20.2
(I) Dry Wt.	108.8	107,1	111.2	.9:601	1127	1.401	1074	106.9
(J) Proctor Wt.	108	10.5	108	80/	801	103	108	105
(K) % Comp. I ÷ j	100	100	100	100	100	100	200	100
(L) % Moist					E III			
		3						
(N) Location	3+00	3+16	2400	5+15	3+20	3+00	00+9	6420-
	50 17 2-	4	75.478-	4	100 RT &	10087 E	125 RIE	125,878
(0) Elev.	756	75.5	750	749	758	151	747	726
		7 4	*		17.1	್ರ ಧ		

Compaction Required

Technician

.E. Salato Allia

PROJECT: JOB NO.: AREA:

DATE: 9/73
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

			,					
(A) Test Number	0	10	77	12	13	14	15	1/6
(B) Probe Depth			1		3		**************************************	
(C) Dens. Count	432	450	408	426	472	4	420	040
(D) C ÷ Std Count		a Ti	e _{li}			3		266
(E) Wet Wt. (1b/ft ³)	127.0	124.7	130.0	1227	1222	1287	128.5	10 4 0
(F) Moist Count	1020	920	0///	980	1000	0701	000	0///
(G) F ÷ Std Count			- s:					
(H) Wt. Moist (1bs)	19.9	17.5	220	18.9	19.3	208	12.27	22.0
(I) Dry Wt.	107.1	106.2	108.0	108.8	7	6701	110.3	1028
(J) Proctor Wt.	103	103	80/	807		108	108	220
(K) % Comp. I ÷ J	001	100	100	100	100	100	100	000
(L) % Moist			al and				V ***	
				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				a ne
(N) Location	4400	4+20	6400	5149	4450 100KTE-	Ates	6750	V 2465
(0) Elev.	7.53	752	746	745	759	(C.	12.	*27
			5%					

Compaction.Required_

Technician

17 17 27

11 11 11

PROJECT: LAMAS & EL

DENSITY STANDARD COUNT: 9-7-73

			E					
(A) Test Number		8/	/5	20				
(8) Probe Depth	, 9	8	0) () ()					
(C) Dens. Count	462	474	478	777	2			
(D) C ÷ Std Count			æ					176-2
(E) Wet Wt. (1b/ft3) 123, 0	123,0	122,0	121.5	123.0				Si
(F) Moist Count	1130	1104	7201	11 46	*	VSE		
(G) F ÷ Std Count	æ	g 99 1	- 1 x		v			
(H) Wt. Moist (1bs)	22.4	21.7	50,5	23.0			:	
(I) Dry Wt.	100.6	C 0 0/	7.101.	16.0.6				
(J) Proctor Wt.	\0 p	90/	100	/ 00,				
(K) % Comp. I ÷ j	/06	703	90/	29/	-31			
(L) % Moist		T.					<u>1</u>	·
(N) Location	23+75	53485	52-100	250			ine .	r cos
×	85.86	50.RE	B	70.7 4	ş			
(0) Elev.	739	740	741	242				

Compaction Required

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CONFIDENTIAL BUSINESS INFORMATION

Technician U

. K. ASPAGE

PROJECT: 10 MUS/ C. 17 JOB NO.: SEN CAMM

WINT OUS

DATE: 9.6.73
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

					2			
(A) Test Number	Ñ	/8	//	7	13	14	15%	11
(8) Probe Depth	9		(f. 197					
(C) Dens. Count	206	818	488	765	466	474	450	476
(D) C ÷ Std Count		· ·			. 14			
(E) Wet Wt. (1b/ft ³)	0'811	121.5	120.0	120,0	1230	122.0	5401	1265
(F) Moist Count	10/6	9911	8001	9201	1074	1000	0 7 01	8601
(G) F ÷ Std Count								
(H) Wt. Moist (1bs)	اع (23.4	2.6	2011	21.1	70,6	20, %	217
(I) Dry Wt.	97,4	98.1	p'00/	6.6.6	6	101.4	(20/	0 >
(J) Proctor Wt.	99	200	86	86	007	(10 /		0 5 0
(K) % Comp. I ÷ J	160	00/	9/	700	00/	108	747	1 1701
(L) % Moist					ti •			2
(N) Location	18400	28420	05+67	09 FLE	20+02	2415	21475	21495
	4016	\$ 7,05	811 2 4	90184	9	30,5	180'8	143,72 \$
(0) Elev.	742	176	740	750	7 28	739		752
						-		

Compaction Required

Technician

CONFIDENTIAL BUSINESS INFORMATION

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PROJECT: (C) NASS 1/2 1/2 JOB NO.:

DATE: 9-6-72
DENSITY STANDARD COUNT: 2.3/
MOISTURE STANDARD COUNT: 2.3/

(A) Test Number		7	\wedge	Þ	5	9	_	0=
(8) Probe Depth	9	æ	i .	1	# . S			
(C) Dens. Count	444	428	9/0	426	422	804	9/10	416
(D) C ÷ Std Count								
(E) Wet Wt. (1b/ft ³)	. 125.5	127.5	129.5	127.5	128,0	1300	1295'	129.0
(F) Moist Count	9.80	284	920	902	9,56	820/	066	086
(G) F ÷ Std Count		to and			•			
(H) Wt. Moist (1bs)	7.81	18,9	17.5	17,1	18.5	700	19,1	6'8'
(I) Dry Wt.	107,3	9801	1170	1104	109, 5	6'601	110.d	1 0 1
(J) Proctor Wt.	106,3	106.3	0//	<i>011</i>	01/	01%	17.0	0.7.
(K) % Comp. I ÷ J	100	100	00/	150	QQ./=	00/	700/	
(L) % Moist		8			-			
(N)	4-115	4+25	245	97+5	7+00	2410	7490	8400
	80'R€	50'EL	132,78	2007 6	7,07	30/6 2	146,84	
(0) Elev.	450	753	976	747	737	736	2 2	j.
						=		

Compaction Required /00

Technician 0)

CONFIDENTIAL BUSINESS INFORMATION

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7

PROJECT: Part - Wansley JOB NO.: AREA: Sar Down

76.17

DATE: 7/6/73
DENSITY STANDARD COUNT: 23
MOISTURE STANDARD COUNT:

		**************************************)et					
(A) Test Number		7	8	4	7	9	7	∞.	
(8) Probe Depth	6 inches		::	· (1)		*			
(C) Dens. Count	420	430	434	420	428	4/8	400	436	-γ
(D) C ÷ Std Count				\$\frac{1}{2}			-	4	+
(E) Wet Wt. (1b/ft ³)	128.5	127.2	126.7	128.5	127.5	1287	131.0	1262	
(F) Moist Count	990	260	1120	1000	1000	0/0/	00//	1110	·
(G) F + Std Count	i e		9 3 1	10 1 23					·
(H) Wt. Moist (1bs)	1.6%	18.5	22.2	/9.3	19.3	9% 1	21.7	22,0	· · · · · · · · · · · · · · · · · · ·
(I) Dry Wt.	1.09.4	108.7	104.5	109.2	/ 08.2	1.901	109.11	1042	
(J) Proctor Wt.	90	108	103	108	801	109	80	103	
(K) % Comp. I ÷ j	100	100	100	100	100	100	100	100	
(L) % Moist	5			14.	£				
(N) Location	5+40	5+30	5+50	0440	4+00.		4+70	06+4	
	-327052	4	S	CAI	ò	RIES	(4)	20618	
(0) Elev.	7.45	794	747	746	752	757	750	799	
							A	**************************************	

Compaction Required

Technician

ULUNGIA FUWEN CUMPANT - SUILS NUCLEAR DENSITY METER WORK SHEET

2

PROJECT: JOB NO.:

DATE: 7/6/73
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

(A) Test Number	8	10)]	171	(3	14		
(8) Probe Depth			28 30 22					
(C) Dens. Count	424	454	4/0	4 / 8	457	420		
(D) C ÷ Std Count			C.					
(E) Wet Wt. (1b/ft ³)	128.0	124.2	129.7	128.7	124.2	127.2		
(F) Moist Count	950	096	1060	066	9.50	1020		
(G) F ÷ Std Count	5						11173	
(H) Wt. Moist (1bs)	2'81	1.8.5	20.8	18.1	18.7	19.9		
(I) Dry Wt.	9.60/	105.7	1089	1096		107 2		
(J) Proctor Wt.	108	106	108	200/		701		
(K) % Comp. I ÷ J	00/	001	00/	100	001	100		
(L) % Moist		100 E						31 d
	ш		= 5				·	
(N) Location	1400	7+15	05 +2	59+1	00+00	Q		

CONFIDENTIAL BUSINESS INFORMATION

Technician

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31799

00+8

59+1

7750 5017 &

155 LTE

15047

726

727

729

730

736

735

(0) Elev.

Compaction Required

PROJECT: (1)/2:/ JOB NO.: AREA:

DATE: 2-673 DENSITY STANDARD COUNT: 231 MOISTURE STANDARD COUNT: 1277

				n.				
(A) Test Number	15	9/	17	E	. 15	20	2/	22
(B) Probe Depth	(, (,		E					
(C) Dens. Count	484	995	160	8/8	205	458	XC7	V
(D) C + Std Count			A 100 mg					7
(E) Wet Wt. (1b/ft ³)	(12/.0	123.0	1215	117.0	112.5	06//	121.5	7.67
(F) Moist Count	10/4	J072	1060	886	1000/	707.2	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	97.6
(G) F ÷ Std Count			= 11					
(H) Wt. Moist (1bs)	17.6	1'02	30.8	0	19.3	3.5	20.6	681
(I) Dry Wt.	161,4	6.201	1001	6/26	266	19.4	1007	3.5.0
(J) Proctor Wt.	90/	/06	w/ 	86	386	36	09/	500
(K) % Comp. I ÷ J	700	20/	7.00	7.56	168	130	700	247
(L) % Moist		-			(co.)			
	10 4 V	1514. 2	264co	56-120	2610			
(N) Location	120'RE 125'RE	125'RR	80,08	76.12	五八09	77.05	W	20.78
(0) Elev.	801	73.5.	747	74	744	746	7.4	7,
							- 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	

CONFIDENTIAL BUSINESS INFORMATION

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Compaction Required

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Technician

41 -mkm

JOB NO.: SPET SPET AREA:

DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

							. •	
(A) Test Number	5	0/	11	12	\$	74	u u	5 3 2222
(8) Probe Depth	62		2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 -		#2 #2		22 =	
(C) Dens. Count	426	438	4/2	406	424	438	TT 0.02	
(D) C ÷ Std Count			45	e a		AI.		
(E) Wet Wt. (15/ft ³)	127.5	126.6.	125.5	130,0	128,0	126.0		
(F) Moist Count	1602	858	945	992	726	288		- 3
(G) F ÷ Std Count		# %						
(H) Wt. Moist (1bs)	19.3	1701	0/8/	191	17,8	200		
(I) Dry Wt.	7:83	168,9	, 5 .)	5011	7.0//	1094		
(J) Proctor Wt.	801	80/	110	1/10	0//	0//		4
(K) % Comp. I ÷ J	- S)	100	. / ¢ 0.	10/	007	0.67	5	es
(L) % Moist					. 53	•		
(N) Location	るよ	09+9	OOKL	51-1-6	0017	7	1	
	100/R \$	115'R.R.	-D	20.7 6	7293	2014		
(0) Elev.	256	737	731	130	729	728		
					10			+

Compaction Required_

Technician /

CONFIDENTIAL BUSINESS INFORMATION

1. 4466

NUCLEAR DENSITY METER WORK SHEET

PROJECT: WW. S/P. JOB NO.: AREA:

DATE: 2 5 5 7 3
DENSITY STANDARD COUNT: 23
MOISTURE STANDARD COUNT: 127

(A) Test Number) 	2	(h)	X	7	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		4
(B) Probe Depth	· •		36)		0 -
(C) Dens. Count	877	430	446	4/8	448	432	472	477
(D) C ÷ Std Count	•	a)	
(E) Wet Wt. (1b/ft ³)	127.5	1270	125.0	1250	125.0	0761	1270	128 0
(F) Moist Count	924	916	872	876	10:04	284	306	6/2/2
(G) F ÷ Std Count							0	
(H) Wt. Moist (1bs)	17.5	17.5	16.3	18.5	193	16.6	861	18.7
(I) Dry Wt.	1100	109,5	10801	5 9 11	(20/	7011	1097	3 5 1
(J) Proctor Wt.	01)	//0	7.06.7	01/	1063	011	110	
(K) % Comp. I ÷ J	100	166	700	901	707	ya r	100	
(L) % Moist								
(N) Location	425	0610	21 th	0175	40	4+60	5175	28+5
	6	15'R&	40,86	50'R of	\$ 1,08	7/14	77.18	2016
(0) Elev.	747	246	(8)	745	74%	745	746	745
								,

Compaction Required 100

Technician

GLUNGIA FUNET CONFRANT - SULLS NUCLEAR DENSITY METER WORK SHEET

PROJECT: JOB NO.:

DATE: 7/5/777 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT: The line / σ

	87							
(A) Test Number	00	0	0/	11	12	4.1	15	1
(8) Probe Depth			70 70-	2			1	0
(C) Dens. Count	464	450	150	508	202	AZO	640	478
(D) C ÷ Std Count							3 - 1	140
(E) Wet Wt. (1b/ft ³)	123.0	124.7	124.7	118.0	1.361	123.5	1757	1075
(F) Moist Count	860	0/6	8 40	920	02.0	200	000	200
(G) F + Std Count			*	- X				077
(H) Wt. Moist (1bs)	18	17.3	7	17.5	202	007	17.	181
(I) Dry Wt.	6901	107.4	0.601	100.5.	107 5	1040	/ 00/	. 00
(J) Proctor Wt.	30/	1901	1012		707	9.7.7	0.00	108,4
(K) % Comp. T = .1	100	1001	701		007	7	90)	-10E
		7 7	000	100	100	700	190	100
(L) % Moist	0.0			a :			11	
		2					a.	
(N) Location	9200	0.76	7400	7420	2450	5+60	001-5	0745
		1	20.47.2	A	- 7.17 011	4	100 17 8-	4
(0) Elev.	717	7/6	730	729	145	-144		- 1. A. S.
	-							, ,

Compaction Required

Technician

CONFIDENTIAL BUSINESS INFORMATION

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PROJECT: JOB NO.:

DATE: 3/5/2/3 DENSITY STANDARD COUNT: 25/ MOISTURE STANDARD COUNT: 27/

(A) Test Number	=	2	M	4	γλ	10	1	Co
(8) Probe Depth	9::0	4	4	<u> </u>	7	Ą	**************************************	1
(C) Dens. Count	450	438	.464	1 440	430	460	406	24 U
(D) C + Std Count	J	***************************************	Vicebourge	P-A-may-maga		-1		Michael
(E) Wet Wt. (1b/ft ³)	123.7	12.6.2	123.0		1272	723.5	arte described	1252
(F) Moist Count	000	1020	078	0201	780	1040	05//	1125
(G) F ÷ Std Count		and transporting	Year to 10 magg			grandworkd man	-	
(H) Wt. Moist (1bs)	17.1	19.9	(9)	19.9	1	(A)	232	- C - C - C - C - C - C - C - C - C - C
(I) Dry Wt.	106.6	106.3	106.9	1551	1087	103.2	5 % C	6
(J) Proctor Wt.	=			41 A	z.			
(K) % Comp. I ÷ J							11	
(L) % Moist		31 31		211				
)E	11 500	Ε.		**		
(N) Location	00101	00+6-	20+26	10 mm 12 mm	227.40	22460 V	23+50	09/50
	125		430612		7 17077		20 C12	1
NO) Elev.	()	# J	787	\$ C.	128	729	752	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~

Compaction Required 100

Technician ///

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NUCLEAR DENSITY METER WORK SHEET

DATE: / COUNT: DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT: PROJECT: JOB NO.:

		4	0 6	***				
(A) Test Number		7	M	7	7		-	
(8) Probe Depth	" ?		20 (c)	š			5	3.
(C) Dens. Count	462	27			1 1 1 1 1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1. (1)	
(D) C + Std Count	I	3						
(E) Wet Wt. (1b/ft ³)	- \\	123,0						i gust
(F) Moist Count	`` ``	7 1	ا					9 d 3 d 2 d 2 d 2 d 2 d 2 d 2 d 2 d 2 d 2
(G) F ÷ Std Count	25 H	*		elec Se)	
(H) Wt. Moist (1bs)		10.3	21.1	150	8703	30,0	` ''()	
(I) Dry Wt.		101.7		16 7:		919 9/		
(J) Proctor Wt.		1863	103	100		3.07	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
(K) % Comp. I ÷ J);(02/	F (5-		- 5 '		1	
(L) % Moist		.94	Rend	1-12-1	48 4			
(N) Ocation				1,1,1	i e e e			37
	1614	111 4	> ' '	- >i - / - 1	7	777	·	
(0) Elev.			727		2016 11 22			5.
			L	(1)		;	}	

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GLONGIA FUNER COMPANY - SUILS NUCLEAR DENSITY METER WORK SHEET

3 ή, Z — 1 DATE:
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT: ~ ~ / ت ` 7 7 - / / 7 0 5 7 <> 3 00, V 0 •) • ! 307 ن ز: \ -1 478 0.7 -4₇ 100 0 00 9.50 - : - : シント 1 . 201 E) Wet Wt. (1b/ft³) (H) Wt. Moist (1bs) (D) C + Std Count (G) F - Std Count ~ C) Dens. Count F) Moist Count (K) % Comp. I ÷ A) Test Number B) Probe Depth (J) Proctor Wt. (L) % Moist (I) Dry Wt. PROJECT: JOB NO.: AREA:

Technician

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(N) Location

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1.7

Elev.

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Compaction Required

NUCLEAR DENSITY METER WORK SHEET

DATE: 9-5-73
DENSITY STANDARD COUNT: 23/

(A) Test Nimber		C. de	13.			9		24 ₂₂
indiana a sa da	ĥ	7						GT-9
(B) Probe Depth	; ~_Э	=:						
(C) Dens. Count		e .)	7	- 1 - 1 - 1				
(D) C + Std Count		9					B	
(E) Wet Wt. $(1b/ft^3)$. 1	12.5	10 C	2/	15 = 35 11): U1 1 1220
(F) Moist Count	- <u>5</u>	172	12.54	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		* 2:		:: :2: :4:
(G) F ÷ Std Count	,	2		8	2.5			
(H) Wt. Moist (1bs)	\$ \$\frac{1}{2} \tag{2}	181						
(I) Dry Wt.								<u>5</u> 1 0
(J) Proctor Wt.	2 2		ე G /	23				
(K) % Comp. I ÷ J	A.		ត្ <i>ត/</i>		20.		7-7-2 1-5	
(L) % Moist	æ	-2						
(N) Location		815 10 10				1.) a	
	1000 C 6		B		*			
(0) Elev.	N N			1907 18				
	**			T	7	-		

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CONFIDENTIAL BUSINESS INFORMATION

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WUCLEAR DENSITY METER WORK SHEET

PROJECT: JOB NO.:

DATE: 94/73
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

		100	Ga.					
(A) Test Number	17	8/	6/	20	*			
(8) Probe Depth			1,550			.		
(C) Dens. Count	4.00	420	4/8	422				
(D) C + Std Count		E		. 12				
(E) Wet Wt. (1b/ft3)	131.0	128.5	128.7	128.2			II	
(F) Moist Count	1270	1180	1250	1300				
(G) F ÷ Std Count	1			100 E	·.			
(H) Wt. Moist (1bs)	25.7	23.6	25.2	76.4		83		
(I) Dry Wt.	105.3	104.9	103.5	101.8				,
(J) Proctor Wt.	103	103	103	103		ē,		
(K) % Comp. I ÷ J	100	100	700	66	-			
(L) % Moist			•	S 8	-			
		0						
(N) Location	20+00	68+61	18475	1840			Α.	:1
	180'R+E-	4	165 Rte-	4	51			
(0) Elev.	7/2	7111	7/2	7111				e e
			2.00	20				

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Technician

CONFIDENTIAL BUSINESS INFORMATION

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GEURGIA FUWER CUMPAINT - SUILS ... NUCLEAR DENSITY METER WORK SHEET

PROJECT: Plant Manslay JOB NO.: Seneration Dam

Seperation Dim

DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

(A) Test Number	. 6	10	11	67	- 22	8 -	1		. ا
(8) Probe Depth		a ⊗			ar A	<u> </u>	C/	0	
(C) Dens. Count	452	516	454	434	466	- 4 A	426	- < > \	
(D) C + Std Count		Æ E			0		971	760	·
(E) Wet Wt. (lb/ft3)	124.7	711	124.2	1.26.7	1227	1272	1077	- 7 - C	
(F) Moist Count	950	920	1300	970	1200	10/0/	1330	000	
(G) F ÷ Std Count		<u>s</u>					2		
(H) Wt. Moist (1bs)	18.5	17.0	26.4	18.7	24.0	361	1.1-0	691	
(I) Dry Wt.	106.2.	100.2	97.8	108.0	987	7201	1000		
(J) Proctor Wt.	1040	100	98	1070	98	200	0 0	0.00/	
(K) % Comp. I ÷ j	001	100	/00	100	00%	007	20%	00/	
(L) % Moist					**				
							¥		
(N) Location		10 RTE	24+50	24+60	224.50	22460	21+00	59402	
- T	6+00	00+9	250R1 & -	4	240 M &	4	250 Rig-		
(0) Elev.	725	726	1/1	770	717	111	717	7//	
	=	D.	જે ુ				21		

Compaction Required

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CONFIDENTIAL BUSINESS INFORMATION

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PROJECT: JOB NO.: AREA:

DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

				ta a				
(A) Test Number	g*					, II o		
(8) Probe Depth			*	207			-	_
(C) Dens. Count	454	466	428	438	4 70	094	470	462
(D) C + Std Count							-	
(E) Wet Wt. (1b/ft3)	124.2	122.7	127.5	126.2	122.2	. 123.5	122 2	123.2
(F) Moist Count	0901	0%6	980	940	700	078	940	0000
(G) F + Std Count	7							
(H) Wt. Moist (1bs)	20.8	185	189	18.0	12.3	7 21	0.87	\$- G
(I) Dry Wt.	1034	104.2	108.6	108.2	6 60%	107.8	2 60%	- FO
(J) Proctor Wt.	104	+01	. 8 <i>01</i>	108	807	80%	4.	104
(K) % Comp. I ÷ J	00/	107	S construction	(7.5)	007	C.7	00	001
(L) % Moist			_	2 2				
(N) Location	0012	. 00t9	25.672	547,52	B	31701	0.5+8	0 5 7 8
	75,848	85818		0019	7400	2400	318001	3011
(0) Elev.	735	734	735	734	730	72.9	725	Buch
		· 1	đi:					

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Technician

NUCLEAR DENSITY METER WORK SHEET

PROJECT: // / JOB NO.: AREA:					DATI DENS MOIS	DATE: // / DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT	D COUNT:	
(A) Test Number							###	
(8) Probe Depth			7223					
(C) Dens. Count	C ,	200	377	1.4.1.	:5		n	
(D) C + Std Count						**		
(E) Wet Wt. (1b/ft ³)	\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \	0/62	€,8%/	12.5				
(F) Moist Count		7	6.3 8 9 8 4 8 4					
(G) F ÷ Std Count		14 90	9	25 25 26		39	3	=
(H) Wt. Moist (1bs)	$Q^{A} \subset \mathbb{R}^{n}$		8 a	¥		÷		
(I) Dry Wt.					# P			
(J) Proctor Wt.	∰ #8: □	fisc Ta		p.C.		200		
(K) % Comp. I ÷ J	C	-7		00%	3		•	20 H
(L) % Moist		4	**		. ,	(*)		- A
(N) Location		A	02+81	18+30	**	=		
D.		->) ii			
(0) E1ev.	ŽI (Ţ.	9 <u>1</u>				c
	\$.	*	=	**		å		

CONFIDENTIAL BUSINESS INFORMATION

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WEUNGIA FUWER LUMPAINT - SUILS NUCLEAR DENSITY METER WORK SHEET

4.1. 合 作 DATE:
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT: 21.75 ٠, 100 3 3 205 · . Ç. r:} *:: i. -1 1 7 ...!! 55 ~ 17 6 0.7 5 .: C:1 E) Wet Wt. (1b/ft3) (H) Wt. Moist (1bs) (D) C + Std Count G) F + Std Count J. C) Dens. Count (F) Moist Count A) Test Number (B) Probe Depth (K) % Comp. I ÷ (J) Proctor Wt. (L) % Moist (I) Dry Wt. PROJECT: AREA:

Technician

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(0) Elev.

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(N.) Location

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WELLEAR DENSITY METER WORK SHEET

4 DATE: Note that DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT: -1.71 S 12 3 · · · / , ئ ز. ;; がこと ٩ 1170 1. 1. * 5 \mathbb{C}_{n} 444 12:0 77 . . ~ ं V 27 001 ~ DE. S. 1 1. ⟨○ . 7 **"**. 100 100 100 (100) 000 51 1 m 16 % E) Wet Wt. (1b/ft3) H) Wt. Moist (1bs) D) C + Std Count G) F + Std Count K) % Comp. I ÷ j (C) Dens. Count (B) Probe Depth F) Moist Count A) Test Number (J) Proctor Wt. (L) % Moist I) Dry Wt. PROJECT: JOB NO.: AREA:

Technician

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1.31

A N

(0) Elev.

Compaction Required

7.3.29

(N) Location

NUCLEAR DENSITY METER WORK SHEET

'ROJECT: 10B NO.: Se.p. Dawn

DATE: 8/3/73 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

	2							
A) Test Number	, / SF	7	B	*	ح	>	2	\$
(B) Probe Depth	و	11		· - /		-		9*
(C) Dens. Count	466	464	.540	.534	450	438	480	5.30
(D) C + Std Count			25	32			1	
(E) Wet Wt. (15/ft ³)	122.7	123.0	114.7	115.2	124.7	126.2	12112	115.7
(F) Moist Count	12/0	1260	1250	1170	1120	1200	1290	0901
(G) F ÷ Std Count				* 0				
(H) Wt. Moist (1bs)	24.2	25.4	25.2	23.4	222	0.65	26.2	20.8
(I) Dry Wt.	385	98.6	89.5	3/6	102.5	1027	75.0	94.9
(J) Proctor Wt.	98	98,	92	92	001	p0/	26	92.0
(K) % Comp. I ÷ J	700	700	98	100	100	001	001	00/
(L) % Moist	8	© 34	28,2		•	. (A)		
			Rerolled	1	20101		() () ()	09171
(N) Location	21400	01+10	17+75	17+25	ノダイン	18+65	17481) } &
	50 RT & -	4	75, 678-	4	75/118-	4	200 RT & -	4
(0) Elev.	210	709	705	704	702	701	700	101
	×	* B B				c.		

Compaction Required /O

Technician ///

PROJECT: JOB NO.: AREA:

DATE: 8/31/73
DENSITY'STANDARD COUNT:

		ě	8		8			
(A) Test Number		2	()	12	/2	14.		
(8) Probe Depth	8		er sa					0
(C) Dens. Count	432	472	400	428	8/4	420	470	707
(D) C + Std Count							0 7 1	740
(E) Wet Wt. (1b/ft ³)	127,02	122,2	137,0	127,5	1287	127.2	1275	1260
(F) Moist Count	1040	1100	1140	112.0	1350	8	10.80	0.0//
(G) F ÷ Std Count				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			000	7777
(H) Wt. Moist (1bs)	20.3	21.7	22,7	22,2	27.5	23.6	25.9	23.60
(I) Dry Wt.	106.7	100.5	108.3	7.6.5:3	101,2	1026	/210/	000
(J) Proctor Wt.	105.0	100	108.0	10500	100	0201	0/0/	7/7/
(K) % Comp. I ÷ J	700	100	10.0	100	100	100	100	141
(L) % Moist	19.0	(%)						
	, , , , , , , , , , , , , , , , , , ,							
(N) Location	70+20	20160	00+61	0/761	20+25	20+35	23+50	23 +60
	200 R7 Q	200'RTE	265 61 4 -	A	72077 2 -	4	230178-	
(0) Elev.	7.06	705	722	121	927	775	730	127
		•	91	150	7.0	0.0	27	0

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CONFIDENTIAL BUSINESS INFORMATION

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> PROJECT: JOB NO.: AREA: Sep Para

DATE: 8/36/25
DENSITY STANDARD COUNT:

				•				
(A) Test Number	17	/8	61		-		E	
(8) Probe Depth			18 10 5 55 420			¥		
(C) Dens. Count	432	440	428		5 · · · · · · · · · · · · · · · · · · ·			
(D) C + Std Count	·	. 15	-					
(E) Wet Wt. (1b/ft ³)	127.0	126.0	127.5	121 241	8			
(F) Moist Count	1250	1/80	1120			U		
(G) F ÷ Std Count			U			1		
(H) Wt. Moist (1bs)	.25.2	23.6	22.5	a.				
(I) Dry Wt.	101.8	102.4	7.05,0					
(J) Proctor Wt.	100	160	507					
(K) % Comp. I ÷ J	1001	001	100					
(L) % Moist		at a		÷	**		=	
(N) Location	19755	18400	01+81	- - -2		ā	8	
	-31702	4	4					
(0) Elev.	7/8	720	7.19	. 20 T	t	1 12 2		
		100 mg (100 mg)	14			+	7	

Compaction Required

Technician M. X

PROJECT: Mart Mansley JOB NO.:

DENSITY STANDARD COUNT:

(A) Test Number		7	Ŋ	4	7	V	2	00
(8) Probe Depth	; 9					7		
(C) Dens. Count	418	916	4/4	. 4/0	450	444	430	478
(D) C ÷ Std Count	7			1	1			1
(E) Wet Wt. (15/ft3)	1283	1290	129.2	129.7	124.7	125,5	1274	1775
(F) Moist Count	1172	1210	1182	0.211	1700	1140	0 : 3/	09//
(G) F ÷ Std Count			1			7		
(H) Wt. Moist (1bs)	23,4	S#3	0.5.0	23.4	21.7	22.7	242	23.2
(I) Dry Wt.	105.4	8 to1 .	1026	7.06.3	103.0	102.8	1032	174 7
(J) Proctor Wt.	103	103	103	103	183	103	103	10 M
(K) % Comp. I ÷ J	100	100	100	100	001	707	00/	77
(L) % Moist								
				-				
(N) Location	24,400	2.4+15	23+30	23+45	21+05	2/+20	19475	59761
	175 42-	4	7007	<u></u>	220218-	A	240	A
(0) Elev.	155	254	700	0.19	724	723	72.0	6/1
							, 4,	

Compaction Required

Technician

PROJECT: JOB NO.: AREA:

DATE: 8/30/73 DENSITY/STANDARD COUNT: MOISTURE STANDARD COUNT:

(A) Test Number	٨	10	//	12	/3	4	15	
(B) Probe Depth	9	4	4	4	4	4	4	O A
(C) Dens. Count	402	416	444	454	422	424	9/4	404
(D) C + Std Count	7	1	1	1	7	1	1	1
(E) Wet Wt. (1b/ft3)	130.7	1290	125.5	124.2	128,6	128.0	1287	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
(F) Moist Count	1340	13.50	1200	11.70	1058	1252	1541	1040
(G) F ÷ Std Count	1			1	1			1
(H) Wt. Moist (1bs)	27.3	27.5	240	23.4	20.8	25.2	276	2.96
(I) Dry Wt.	103.4	70/5	101.5	100.8	107.2	102.0	100 8	1018
(J) Proctor Wt.	103	100	100	001	105	103	62	0 00/
(K) % Comp. I ÷ J	700	100	100	001	1001	100	100	701
(L) % Moist			43.	£.				
	2	28						ł.
(N) Location	24+50	24+60	23+50	23460	23440	22+50	27160	0 4710
	50 LTE -	4	75179-	4	4	80778		80179
(0) Elev.	735	734	730	729	72.8	726	725	725
				•30			The second secon	,

Compaction Required_

Technician N. O.

PROJECT:

AREA:

DATE: DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT Technician ... 0 ٧ ٩ · ų į 57.4 1602 2/1/2 1 V 821 [1]] 1.0 37 0 ... [] .1 1173 , () 5 . . 100 ÷.1 3011 ... 90 Ö نت Ţ, (E) Wet Wt. (1b/ft³) (H) Wt. Moist (1bs) Compaction Required (D) C + Std Count G) F + Std Count K) % Comp. I ÷ j (C) Dens. Count (A) Test Number (B) Probe Depth (F) Moist Count (J) Proctor Wt. (N) Location L) % Moist I) Dry Wt. (0) Elev.

> PROJECT: JOB NO.: AREA:

DATE: DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

,	-	•	.a					
(A) Test Number		(Park)	1.7	/-)			
(8) Probe Depth	. 9							
(C) Dens. Count	37	7/6	327	4	424	411		
(D) C ÷ Std Count						000		
(E) Wet Wt. (1b/ft ³)		129.0.	1250	6,761	128.0	0001	fie-	1.76.13
(F) Moist Count	284	1301	11 mg		17.40		6	
(G) F ÷ Std Count		10 10 10 10						
(H) Wt. Moist (1bs)	5 5 2	21.6		246	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	27.7	7	
(I) Dry Wt.	103.6	152,4	1000	4.20	1107			
(J) Proctor Wt.	9 2 1 3 2 1 3	Q/	. 0.	,	100/			
(K) % Comp. I ÷ J	703	73.7	36	2	(A)		- 3	
(L) % Moist		25	Report	I = I				ž.
(N) Location	1.2.7	SIN	1.74		20400			7
	75%	\$ 7,58	F 709	7	100. L. d	1101	100	7 7 7
(0) Elev.	721	722	00/	12.	722	227	.7.2.1	
			3.0					E6

CONFIDENTIAL BUSINESS INFORMATION

Technician

Compaction Required

PROJECT: JOB NO.: AREA:

DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

					25			
(A) Test Number 👙	. / 7) <u>(</u>						
(8) Probe Depth	(6.77		e n s	ä	S			
(C) Dens. Count	7.7	1.67	4 0 7 1		75,7	F.,		Commence of the company of
(D) C ÷ Std Count		. =	8					
(E) Wet Wt. (1b/ft³) 12 €.0	1250	0.8.6	0.77	032	1240	0 /	1 43 2 1 1 2 1 1	
(F) Moist Count	100 100 1	1 N C C 2 (ا ا ا			
(G) F ÷ Std Count		વ			-	ē	-	
(H) Wt. Moist (1bs)				2/2	101		e de Sa	
(I) Dry Wt.	ું છે	A SEC		9.13	1011			
(J) Proctor Wt.	,	5.0		Γ				
(K) % Comp. I ÷ J	1.5			S.		3 5		
(L) % Moist	41	II		×				
(N) location	05 FX (57+5/	77.57		20175	9 3		<i>y</i>
	12479	12526	77.511	120 4	41.14	47 5%		* 1, 1
(0) Elev.	C-C-	724		7/1	72.1	ΓĄ	200	

Compaction Required_

CONFIDENTIAL BUSINESS INFORMATION

Technician

LAW ENGINEERING TESTING COMPANY NUCLEAR DENSITY METER WORK SHEET

Plant Wansley i. C 6-4100 ROJECT: JB NO.: REA:

Dani

DATE: 8-25 : 3 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

4) Test Number	6	0/	//	12	100 m	1 4	7.	The same	******
3) Probe Depth								.g	
C) Dens. Count	442	0.7	1.64	1-74	. J. O. D.	426	7 7 0	120	
)) C ÷ Std Count	-	-			1 4 12	130	400	454	
E) Wet Wt. (15/ft ³)	125.7	128.5	123.0	1244	1279	0000	1072	I C	
=) Moist Count	0///	1120	1623	13.4	1260	10.00	1740	0000	1
3) F + Std Count	24 65	#5				0 = 4 = 4	77.77	14.00	
4) Wt. Moist (1bs)	22.0	22.2	23.7	27.3	254	0 16	000	0 7 0	
I) Dry Wt.	7.03.7	106.3	99.3	97.7	97.1 102.5	1014	7114.5	107 12	
1) Proctor Wt.	1030	(05	90/		· · · · · · · · · · · · · · · · · · ·	. 707	2.0.		
<) % Comp. I ÷₃J	100	001	66	76	100	100	000	750	
-) % Moist	5) 5)						00/		
·	54 19+25	5/219+40	Sta-20200 Cti- 318	179- 349	Sta 22+25 St 22+40 Sta-24+30 Sta 21+33	St 22+#	054-24+30	\$40.20+43	
N) Location	175.43 4	175.24 3	1181 24 8 175 24 9		200' 22 9 200' 18 0	200118	Section 1		
o) Elev.	717	972	916	715	720	719	727		
		2 3 2 3 3 4 3 7	CONFIDE	CONFIDENTIAL BLISINESS	c() /	0			

CONFIDENTIAL BUSINESS INFORMATION

Impaction Required 100^{3}

Technician

WUCLEAR DENSITY METER WORK SHEET

PROJECT: JOB NO.: AREA:

DATE: S/7 9/ 5 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

				·				
(A) Test Number	1.7	· 6/	6)	0	1 ~2	22	2 12	7 4
(B) Probe Depth					80			
(C) Dens. Count	458	460	422	434	440	257	440	1.3.4.
(D) C + Std Count							7 4 7	
(E) Wet Wt. (1b/ft ³)	123.7	123.5	128.2	7.901	0951	1060	100 to 1	1199
(F) Moist Count	1400	1380	1280	1240	1/60	1200	1210	1000
(G) F ÷ Std Count							0/61	001
(H) Wt. Moist (1bs)	287	28,3	262	75.3	23.2	0.00	090	0.45
(I) Dry Wt.	95.6	2.56	1020		102.8	1022	6 467	1000
(J) Proctor Wt.	(00)	130	100	001	100	100	7001	000
(K) % Comp. I ÷ J	to West	lad	90/	00/	/00/	700	777	907
(L) % Moist	70 X) (
(N) Location	0.5+61	09761	21400	21+20	22450	27.760	24+25	スムナンス
	-317001	4	-317001	4	3,17,08	4	4-37.51	4
(O) Elev.	to Chami	7/3	7/6	7115	200	754	13.7	E 0 1 %

Compaction Required_

CONFIDENTIAL BUSINESS INFORMATION

Technician

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LAW ENGINEERING LESIING COMPANY NUCLEAR DENSITY METER WORK SHEET

Plant Wansley G-4100 S 00. ROJECT: OB NO.:

DATE: 8/29/73
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

A) Test Number	1	2	. W	4	- 4		1	
B) Probe Depth	. 9	4-02	4	4		0 4		W
C) Dens. Count	436	420	4 60	0.7	- 1			AH
		0 7 7	7.50	177	436	428	432	436
D) C + Std Count					***************************************			
E) Wet Wt. (1b/ft3)	126.2	1262	124.7	.0.8.2.1	2761	1975	1276	111/11
F) Moist Count	1090	1020	1080	/000	10001	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	2000	10:01
G)'F + Std Count	* = -	j	: ;;		3031	10.0	000	000
H) Wt. Moist (1bs)	215	19.9	21.3	19.3	208	202	2 2	\$ 61.
I) Dry Wt.	1097	106.3	103.4	1087	NO.5 4	1070	1057	7 2 / 1/1
J) Proctor Wt.	105,	105	100	X 0 X	2 2 2 2 2 2 2		>	(0.0/
K) % Comp. I ÷ J	100	001	001	7.0017	72	0.57		(7.7)
L) % Moist								
			e	S.		2		•
N) Location	. 09+61	02 +61	21+40	95112	22425	いたけたり	36166	29162
	7807+6-	4	255474-	4	77517 6-	4	270.179	4
0) Elev.	2/8	717	720	719	77.6	707		9 6%
					Q.	1		

Ompaction Required .

Technician

PROJECT: WZZAS/ JOB NO.: AREA: Sectional

221 DATE: 2.2 8.73 DENSITY STANDARD COUNT:

		-			*			
(A) Test Number	5	10	11	77	7	M	101	1
(8) Probe Depth	9		766	8	*			2
(C) Dens. Count	874	55.50	456	JE 5	404	4,5	15/4	d.c.a
(D) C ÷ Std Count				ed *		1		130
(E) Wet Wt. (1b/ft3)	127.5	1276	124.0	131.5	130,5	128.5	1245	13451
(F) Moist Count	1198	1184	988	1216	7571	116.5	26.65	
(G) F ÷ Std Count	la:		3					00/
(H) Wt. Moist (1bs)	240	23.6	181	24.6	スペン	22.0	19.3	18,7
(I) Dry Wt.	163,5	103.4	6701	107.9.	x 2 %	1001	4 5 0/	8 3 47
(J) Proctor Wt.	/63.	297	105	8 2/	80/	108	100	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7
(K) % Comp. I ÷ J	100	/ 40	60%	707	001	160	100	1/43
(L) % Moist						-		
(N) Location	23+60	23+10	24750	24460	25+04	25415	18456	18-16:
	37.08	80.78	77.051	\$ 7.091	\$ 7.00/	77.01/	\$ 7.96 B 7.98	\$ 7.96
(0) Elev.	727	728	730	126	1	725	218	21.9
						*	7	

Compaction Required

Technician (

CONFIDENTIAL BUSINESS INFORMATION

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PROJECT: JOB NO.: AREA:

DATE: 8-29-73
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

(A) Test Number		7	~	4	3	9	0	2
(B) Probe Depth	و ۲	5)						3
(C) Dens. Count	438	874	406	405	412	392	434	460
(D) C ÷ Std Count		. III	dt)				-	
(E) Wet Wt. (1b/ft ³)	0.921	127.5	130,0	13/0	129.5	132,0	126,5	123.5
(F) Moist Count	1230	1220	1038	7201	9/9/	934	p/11	1
(G) F ÷ Std Count		· · · · · · · · · · · · · · · · · · ·						
(H) Wt. Moist (1bs)	24.8	24,6	5000	19.9	6'51	18.0	22.6	18.9
(I) Dry Wt.	7'101	102,9	1697	1111	109.6	114.5	1045	154.6
(J) Proctor Wt.	100	407	601	109		114	105	105
(K) % Comp. I ÷ J	700 t	1001	3/	100st	0,07	150	33	120
(L) % Moist	ás*		Mi					
(N) Location	18451	155 75	50406	2012	2/400	2/4/5	22725	22435
	260.64 275.24	275.24	17524	7524 18522 20074	37.002	215/4	2502 \$ 2602	26028
(0) Elev.	715/	7/6	3/6	616	717	816	724	723

Compaction Required_

Technician

PROJECT: JOB NO.: AREA:

DATE:
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

		3			29	9.		
(A) Test Number		(.)	17	12	7	777		
(8) Probe Depth	.)							
(C) Dens. Count	200	ハーカ			The second second	7.67		
(D) C ÷ Std Count		·						
(E) Wet Wt. (1b/ft ³)		050			12 46.5	13.6.0	0.167	
(F) Moist Count	1939	12.16	10 m	, e	6/56		£ /	
(G) F ÷ Std Count			= *		2 2 2			
(H) Wt. Moist (1bs)		2.4.6		2	5.0	33.4 5 1		
(I) Dry Wt.	\$ 1 0 (\$ 64.7	6467 63 6	2-11	12.3 4.4
(J) Proctor Wt.	₹2. 	540	90.34 2 3	14 23	s**	= 1 24 34 36 46		
(K) % Comp. I ÷ J	\$ = 7	ພາກ ່ ເນື້ ພ	, 1	= - -	9:	. S.		â
(L) % Moist	F		Ś				163	
(N) Location	C = 2.2/		7.6	8	9774	£ .		
	194	3.7.5	10, 4		1200	77.1.7		
(0) Elev.		(2)		7 /	0.2		6) 1	
		(a)					**************************************	—————————————————————————————————————

Compaction Required

Technician

PROJECT: JOB NO.: AREA:

DATE: 3 7 6 7 5 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

ń.				100 Miles				**
(A) Test Number				* = <u></u>	2			
(8) Probe Depth	1.1				2		a 52	
(C) Dens. Count	448	JS77	327	4.3	dir.	402	4415	100
(D) C + Std Count		îi Îi		(#) ()	Đ	-	48	
(E) Wet Wt. (1b/ft ³)	11254	127.0		1290	1.23,5	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		(2)
(F) Moist Count	827	1382		8701	73.07		2 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	10× 6
(G) F ÷ Std Count		· · ·			3.1		ш	
(H) Wt. Moist (1bs)	CC	W 55	2. 2. 4.	2.1.1	6:37			0
(I) Dry Wt.	6 (5		1.561	107.3	() Lb		0.40	
(J) Proctor Wt.	100	160	(3)	₹ [©]	An	VC7 .	/ 26	*
(K) % Comp. I ÷ J			/ 68%	6(7)	86		. (10)	
(L) % Moist		15 15 15		8			3 S	
	51+00	Mark 1.	1.0 m	tu	05402	2 4465		
(N) Location	3007 4	778	2001 4 2016	210K	P7.071	<i>\$7.71</i>	25074	
(0) Elev.	1/2_	710	1.12.	2/2	7/2	113	7/2	13
							***************************************	*

Compaction Required

Technician [/]

LAW ENGINEERING TESTING COMPANY NUCLEAR DENSITY METER WORK SHEET

DATE: 8/28/25 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

30JECT: Plant Wansley
JB NO.: 6-4100
REA: So.D Darn

2.p Cam

4) Test Number	~~	2	50	+	ig		1-	
3) Probe Depth	8	×	, i			9		0
3) Dens. Count.	9/6	426	400	4.20	450	A	100	
)) C - Std Count		Ē		2 11		3	150	4/8
E) Wet Wt. (1b/ft ³)	1297	1277	13/2	0121	1247	100 C	10.01	7.07
-) Moist Count	1060	980	1/96	1010		7.7.7	10001	7.00
3) F ÷ Std Count							600	0 0
4) Wt. Moist (1bs)	207	18.9	21.7	7.61	21.5	010	310	000
I) Dry Wt.	1070	108.00	110 5	2///	1030	× / 2/	777	000
1) Proctor Wt.	108	108.0	108:	/0.8	103	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	101	
<) % Comp. I ÷ J	100	100	100	1001	100	100	200	(()) /
-) % Moist	32		2 2 2 4 3				H	
V) Location	051-61	05+61	09+61	09+61	19+80.	17480	20420	20+20
	265,178	2507E	17548	19548	31709	75278	3 17 53	2:128
)) Elev.	712	7/1	712	7/1	0/1	60	000	£
			19	37				

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Technician

CONFIDENTIAL BUSINESS INFORMATION

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JOB NO.:

DENSITY STANDARD COUNT:

(A) Test Number	<i>,</i>	67		. /		\$ 100 miles		
(8) Probe Depth		=	12					
(C) Dens. Count	327	566		19 3 3 N	27. 19			
(D) C ÷ Std Count		91.1				2 2- E		
(E) Wet Wt. (lb/ft ³)		10 kg = 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				12.11.5		6,0
(F) Moist Count	13.3	470	10 = 1 10 = 1				1 - 2/	
(G) F ÷ Std Count			y	38 38 35	٠.			
(H) Wt. Moist (1bs)		(j) 6 8 8	-		(- - ;			
(I) Dry Wt.		597	104 (17:52 3	5.00		1	200/
(J) Proctor Wt.		3. 3. 3. 4.		27 	9717			Comment of the commen
(K) % Comp. I ÷ j	e 4	đ.	i .	6-7-		H	3 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
(L) % Moist								
(N) Location		· · · · · · · · · · · · · · · · · · ·	44 4 4.2	6 5 5	1 1 1 1 1 1 1	7011	1.3	E
		3012	7 1 2	7775	2007 Z	P = 57		40/
(0) Elev.	3.6.6	770	73.6	1.51	2.7			
			2		s) Al	+		

Compaction Required 100

CONFIDENTIAL BUSINESS INFORMATION

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PROJECT: JOB NO.: AREA:

DATE: DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

(A) Test Number	/	2	ri	77	-1	`	Ċ	
(8) Probe Depth	(' ''	8 8 8		2		in in	22	
(C) Dens. Count	4.64.	446			470	V	1	
(D) C ÷ Std Count			E .			,		
(E) Wet Wt. (1b/ft ³)	123,0	125.0	0.551	10 10 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	5.23			C : /
(F) Moist Count	762	348	216	78.5	100		100.3	1.04.01
(G) F ÷ Std Count	2	g ^m) [5] 24	*	9.			
(H) Wt. Moist (1bs)		18.2			F Gal		1.5.5.	172
(I) Dry Wt.	13.4.5	8.001	104	106 2	105.4	0.500	10%0	F. (W)
(J) Proctor Wt.		5 901	501		1:6.3	n i	2 701	* 1
(K) % Comp. I ÷ J	001	100	w-01	- A - D	65	43	- 5	
(L) % Moist		⊕:						
; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;	T. T.	0912			0216	5-116	0314/	
(ii) =0ca c i 0ii	4 0 R	20 R. L	30'P 4	26'R d	· 6.	*	22014	777
(0) Elev.	725	276	220	72.57	7.35	21.6	7.5	7:6
					A			

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required_

Technician

LAW ENGINEERING TESTING COMPANY NUCLEAR DENSITY METER WORK SHEET

REA: CAP DAW No. 18-4100
REA: CAP DAW

DATE: A / 2 / - 3 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

A) Test Number	7	1.5	6/	2.0				
8) Probe Depth				24 8 44 8 8 8 8	•			
C) Dens. Count	480	448	4KQ	460		***	ų.	50
D) C - Std Count			<i>1</i> 1				£	e e
E) Wet Wt. (15/ft3)	123.7	128.2	125.7	1267		54		
F) Moist Count	1320	1280	1120	1480				
G) F + Std Count		2 2 2 2 3 1						
H) Wt. Moist (1bs)	21.0	20,2	17.2	24.2				
I) Dry Wt.	102.7	1080	108.5	1025		26		
J) Proctor Wt.	103	108	. 8. 8.1	101	200			
K) % Comp. I ÷ J	100	100	100	10%	A 900 TO			
L) % Moist				4 .		3 8 8 8 8		
N) Location	13+75	13+90	00121	17413	W 1910X			
	26547E	Auman	317022	4				
0) Elev.	732	731	731	780	10 0 40 3 2 2 2 2 4 4 4 4 4		À	
	Š.		ુ ક	4 /8 /00			07	

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CONFIDENTIAL BUSINESS INFORMATION

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LAW ENGINEERING TESTING COMPANY NUCLEAR DENSITY METER WORK SHEET

Plant Wansley G-4100 ROJECT: 108 RO.:

DATE: 2773 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

			*		•	2		
A) Test Mumber		19		12	13	P /	12	
B) Probe Depth		A		i es		L)	9/
C) Dens. Count	442	4/0	422	7.14	.47.0			
D) C + Std Count	-	-			140	456	436	440
E) Wet Wt. (15/ft ³)	125.7	129.7	130.0	128.7	1217	1001	1 20 1	
F) Hoist Count	980	1020	12.70	1240	1000	18.6.5	127.7	127.2
G) F + Std Count			• 0		001	20.00	1570	02/
H) Wt. Moist (1bs)	6.67	20.8	20.0	16.61	22.7	000	\(\frac{1}{2} \)	
I) Dry Wt.	1068	680/	110.0	7601	10%	1:00/		0//
J) Proctor Mt.	106	109	. 60/	106	187	C1/0/	1001	0 "
K) % Comp. I ÷ J	100	\$Q.7	90/	101		900	000	101
L) % Moist	2 2 2 2				000	0	000	00/
N) Location	1+15	06+2	7+2.5	7.440	9+20	0 4 1 0	1 A 1 & C	
	220 27 8	Δ.	245'7'6	61 S	27042	1	700/te	796
0) Elev.	72.5	73.2	733	732	730	729	79.7	1
*	204	1.11	000	C	7	-	06	

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CONFIDENTIAL BUSINESS INFORMATION

Technician

LAW ENGINEERING LESTING COMPANY NUCLEAR DENSITY METER WORK SHEET

Plant Wansley G-4100 REA:

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120 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

				r	•			
A) Test Number		4	<i>?</i> 3.	* * * * * * * * * * * * * * * * * * *	,			50
B) Probe Depth		2 St	3			S		
C) Dens. Count	900	432	4.00	4 30	. 20F.	200	475 475 477	1
D) C + Std Count		æ.						
E) Wet Wt. (15/ft ³)	195.7	00	121.0	125.7	120.0	000	707	N
F) Hoist Count	0%	500	0/0/	080	<u> </u>	(20)	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	
G) F + Std Count	=		8	· · · · · · · · · · · · · · · · · · ·	<u>.</u>	200		
H) Mt. Moist (1bs)	18,4	19.3	19.6	18.3	1110	74.		5
I) Dry Wt.	127.2	1.1-04	\$ 1.7.	106.8	1.601.		3 1 6	0.07
J) Proctor Wt.	106	907	10.9		. 85	. / 5/		000
K) % Comp. I ÷ J	100	100	100	100	60	1.70	90	90%
L) % Moist	· 2 · · · · · · · · · · · · · · · · · ·	. 45 A	9.51.			Ž.		
N) Location	17+10	17+26	00.751	57+3	12450	23751	. 00701	10401
	190 17 8.	本	235 778	4	260 19 4	4	1,50278	
0) Elev.	728	7007	729	о И Л	732	731		- : :
**	06		3.4 2.	na ta		1		

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CONFIDENTIAL BUSINESS INFORMATION

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PROJECT: U FINS / / JOB NO.:

Unifer Sandista

DATE: \$ 2 </ > > 5

	, (200						
(A) Test Number	94 1	7.0		3	<u>ئ</u>	14	1.	11/
(B) Probe Depth	"7			a			2 · · · · · · · · · · · · · · · · · · ·	>
(C) Dens. Count	426.	434	462	876.	797	454	41.7	4.70
(D) C ÷ Std Count	ar t	- ·					0	
(E) Wet Wt. (1b/ft3)	1280	126,5	1230	127.5	123.0	124,0	125.6	13.0
(F) Moist Count	762	744	984	71.6	9.12	126	200	7/(
(G) F ÷ Std Count			ŧ				3	
(H) Wt. Moist (1bs)	14.2	13,3	18,9	19.1	18,7	7	\$ 7/	1.15
(I) Dry Wt.	113,8	113.2	104, 1	108,4.	154.3	2 201	7:59	16.7.01
(J) Proctor Wt.	011	110	/0/	707	70/	50/	20/	50/
(K) % Comp. I ÷ J	10.01	1:04	9.4	72/	99	121	7.00	780
(L) % Moist		# 19	ReRolled		Transport to the second			
(N) Location	15425	01.751	13+40	13.120	11+65	11+50	9+15	09.76
	2007 6	21016	7 7.05Z	505 4 B302 £	7,861	220/4	25% 4 265%	757.6
(0) Elev.	724	725	723	724	723	724	722	123
		9 (3					**************************************	

Compaction Required

CONFIDENTIAL BUSINESS INFORMATION

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Plant Wansley G-4100 30) ECT: JB NO.:

Se. n nam

DATE: 8/24/73 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

				52			•		
1) Tock Mumbos	17	\ \ 	0		*	0.00			ſ
Too a mainter	7 ,	7	4 /	100	i i				
3).Probe Depth	8			100 N					28
) Dens. Count	422	400	440	428		2			
) C ÷ Std Count		4 	841 O				a s		
[] Wet Wt. (15/ft ³)	1282	131.0	125.8	1.27.5					T
) Hoist Count	990	940	0 20.7	096		21			
i) F - Std Count	27	80°,	и: : :			E 21			7
!) Wt. Moist (1bs)	19.1	180	19.9	182				e Marie	
) Dry Wt.	1.29.1	713,0	105.9	0.607					
) Proctor Wt.) 기선		1.0.6	1098		8 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1 1	1
() % Comp. I ÷ J						*		2 E	
.) % Moist	1 286 21 286		*5				13	75 202 Ti	
	11700	4		59 					100
:) Location	7400	1770	0++6	9+55					-18
	402 67 4	4	200 278	AC		V			
), Elev,	727	726	728	727				07 98-1 E-1 08	1
		* 11			-				

apaction Required

CONFIDENTIAL BUSINESS INFORMATION

REA: C-4100

DATE: DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

A) Test Number	6	0/		12	И	-		
3) Probe Depth			* =			†	15	97
C) Dens. Count	460	4	914	4.20		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	5	
3) C ÷ Std Count					101	126	4 33	408
E) Wet Wt. (15/ft ³)	5007	13,5,5	129.0	6.6.61.	1761	6101	\$	6
F) Moist Count	976	0000	930	0/0/	1.050	1000	1000	130.0
3) F ÷ Std Count		in a				0807	0501	0/0/
4) Wt. Moist (1bs)	187		17.8	9.61	20.6	20.6	20.2	(8)
I) Dry Wt.	1043	76.2	7/1/2	107.6 102.1	107.1	1051	6201	0///
1) Proctor Wt.		1001	1103	10%	7.6/-	106:	1,001	1.0.4
<) % Comp. I ÷ J	.00			123	(10)	2.8.7	02/	
_) % Moist	3 H 1 H 2						2 2	,
() Location	16+00	02791	00+91	15120	15+00	15415	13+50	13.5 4.5.
	211056	*	200 278	↑	190 279	1	3 17561	4
)) Elev.	17.3	1	723	72.4	725	724	77.4	1. 4
						,		- Same

Unpaction Required

CONFIDENTIAL BUSINESS INFORMATION

Plant Wansley G-4100 ROJECT: FOB NO.:

Sp. 1.1.20

DATE: 8-7: DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

3				•	•			
A) Test Number		1	30,	+		10	•	
B) Probe Depth		063 2 V 09						G
C) Dens. Count	4.18	426	120	1 × 0 A		Y 1-7.		34
D) C + Std Count	ja v	3 .			1	7.4.0	00.	S.
E) Wet Wt. ($15/ft^3$)	12.8.7	127.7	1.2.7.	V 0 4 4	202			
F) Moist Count	940	107.0	7.7.7			0.07/		12. 1. 6
G) F + Std Count	(F)			0.07		0037	022	9.50 9.50
H) Wt. Moist (1bs)	18.0	6.61	1.66	201	101	\$\docume{\chi}\$	70 0	
I) Dry Wt.	110.7	7.8	545	0.80	7 607	470		0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
J) Proctor Wt.	170 -	ιG_I					5	
K) % Comp. I ÷ J	1604	δ				C	90/	15.31
L) % Moist							n n	000
M) Location	3+000	51+6	10+60	10,75°	12:25	700千01	14+00 ·	3-1-4
0) Elev.	726	725	72.6	624	26/2	A		the Colors
8	*		80	1	3.5	1		

ompaction Required

CONFIDENTIAL BUSINESS INFORMATION

PROJECT: JOB NO.: AREA:

DATE: 25 / 5 | DENSITY STANDARD COUNT: 25 | MOISTURE STANDARD COUNT: 125 |

		32.						
(A) Test Number	9879	7	M	X	4	>	5	C
(8) Probe Depth	,,9	- 1 II.	2.0		5			7
(C) Dens. Count	454	438	442	432	428	438	4/0	70%
(D) C + Std Count			8					
(E) Wet Wt. (1b/ft ³)	0721	126.0	125.5	127.6	1275	126.6	130.0	130.57
(F) Moist Count	924	936	226	996	996	992	931	116
(G) F ÷ Std Count			26 26 2	# E	: (8 ₀)			
(H) Wt. Moist (1bs)	17.5'	18.0	17.8	18.7	1.61	1.33.7	8,01	18,0
(I) Dry Wt.	106,5	108.0	107.7	T 18 9/	10,8,5	かっつ	1127	\\ \times_{\chi}
(J) Proctor Wt.	107	707	(0/	101	ı	70.7	0//	(10)
(K) % Comp. I ÷ J) U'U	1.00/	100	1004	106	(O)	-000	
(L) % Moist			, ³ 5 .	1001 201 201 34				
(N) Location	02470	51+51	· 09+01		11+60	08.40/	10+15	00+01
	250742606	2664	7 7592 \$ 7587 €	19.	2007 E	210,74	22574 214 4	2124
(0) Elev.	723	724	722	723	7.43	723	7:20	
				23				

100% Compaction Required_

CONFIDENTIAL BUSINESS INFORMATION

PROJECT: (4) / A. AREA:

DATE: 5.25 7.3 DENSITY STANDARD COUNT:

(A) Test Number	n s		a T	3 3		(e) (
(B) Probe Depth				-				
(C) Dens. Count	 	S. C.	¥. 77	9.7.	8012	f _X	41.7	
(D) C ÷ Std Count					10			
(E) Wet Wt. (1b/ft ³)	1295	127.5	128 5	1290	0.67		127.0	12.7
(F) Moist Count	1132	1084			1080	1000		
(G) F + Std Count				11		=		
(H) Wt. Moist (1bs)		21.5			21.3	1×.		N 0
(I) Dry Wt.	100%, 6	5 7	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1.501		Ċ	
(J) Proctor Wt.	3				101			
(K) % Comp. I ÷ J		1.50		683	100		* * * * * * * * * * * * * * * * * * *	
(L) % Moist		ge ^o			9 2			i i
(N) Location	- V	· · · · · · · · · · · · · · · · · · ·	16.7	. C.	12 + 63	07	15470	3.7.7
		1 7 5 7	1957 4	747 d	77.052	7607 4		
(0) Elev.	9		7	ر ر	1	6:6	~	0
		- 14				,	———	

Compaction Required 30

CONFIDENTIAL BUSINESS INFORMATION

Plant Manslev 6-4100 ROJECT:

Bur There

DATE: 8-23-73
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

	نيهر	<u></u>						~~~~·	14		0.00	2 6	•0.	E 1	
	=							1 74,		6 ji	,				ar .
						= 1		=						-	
						Tā.		39			-	1.			
π.			all	•		24	0								-
);	3		17				±3		-			200	M = 1
				la Ti	*	7.25			2				à		1
•	885				\$ 2	g"					G 8 V		e C		
1	9 0	-							=	8 8 3%	* 2				
	. ⊙		1		- A		13. F 26.	163 20 101	125	**	24		6 6		
	9	n,		4/8		1.871	000	a ses	19.3	109.4	0,3	1.00	9. 1 26 3	5176	1221
	i i				7/83	N	<	:	40	0		290	V 100		20%
	.6)	48 53 (8)		930	9	1272	₩₩	10.	21.3	6561.	100	100	3	9400	727
	2					5	9		232	60			8		14 ₀ 12
	<i>6</i> 0			450	हा - 	122.5	09/1		23	99,3	SC.	100		01.01	727
	22 =	22	6	2		0	0	8	ij	ហ	9	r e	24 172		5
	17		7	ξ 2		130.0	1090	<u>a</u>	7.1.5	198.5	100	106		10750	722
28	-62			\dagger	t.	ft3)	2	در	bs)	1	7.5			7 2	
	umber	Depth	+ co		d Coun	. (15/	Count	d Coun	ist (1	ď	lit.	++	7	นะ	E E
***) Test Number	3) Probe Depth) Dens Count) C + Std Count	:) Wet Wt. (15/ft ³)	') Hoist Count	i) F + Std Count	1) Wt. Moist (1bs)) Dry Wt.	1) Proctor Mt.	() % Comp. I ÷ J	.) % Moist	() Location) Elev.
	(1)	<u> </u>										3 (()	ÚL	

mpaction Required

CONFIDENTIAL BUSINESS INFORMATION

Plant Wansley 6-4100 See Pare ROJECT: JOB NO.:

61 DATE: 8-23-73
DENSITY STANDARD COUNT
MOISTURE STANDARD COUNT

					ं			
A) Test Number	9	Ten		<u>.</u>	. a			
B) Probe Depth	ڻ	و				#	1.5	7.7
C) Dens. Count	4.28	27.0	474					
			16.1	0) #	446	0/4:	434	462
U) C + Std Count		; ;	: 		3	•		
E) Wet Wt. (15/ft ³)	127.5	128.5	0 8 67	0.6 61.	0201	7.07	10101	1 2 2
E) Hoist Count	1120	108:0	098.	1016	ONE	0,6	7 :0 %	1432
G) F ÷ Std Count	Đ						1200	240
H) Wt. Moist (1bs)	22.5	21.3	1 () (7 64	7 0	1		<
I) Dry Wt.	0 2 0	1073	01.1.1.	/ A.D. 1	2.01	S .	9	0 ×
		2-7 ()		10/.4	7.70	7117	1.327	10/5/2
J) Proctor Wt.	105	105.0	1001	196	90/	19.		100/
K) % Comp. I ÷ J	100	100			130	1,00	2,00	
L) % Moist						· · · · · · · · · · · · · · · · · · ·		
N) Location	15+50	15+65	16+00	16+20	14+25	1:1	12+50	12+30
÷	240 1.70	1	26,5178	4	317062	A	29047 E	4
0) Elev.	720	721	722	101	707	701	0 0	
*			Q.S.	1			- Ge	

Supaction Required

CONFIDENTIAL BUSINESS INFORMATION

Plant Wansley G-4100 ROJECT: 108 NO.:

Sep Dam

DATE: 8-23-73 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

	1				•			
A) Test Number		2	, m	4	מי		·	
			. 2			0	,	ø,
8), Probe Depth	6 Inches	Α	4	4	A	4	Commence of the second	,5
C) Dens. Count	400	422	900	4) 0	744.		9	
D) C + Std Count					7	440	47.4	402
E) Wet Mt. (15/ft ³)	12.12	1						
		/ 67/	2.021	12.7.7	127.8	1257	1280	130.7
r) Fiolst Count	1030	1010	1100	1100	1110	1120	Ç G	
G) F + Std Count		Ξ		54 95 8 8 9 1				Q 7
H) Mt. Moist (1bs)	101	361	7.10	7.7	6 6 6			
				Jen 1 o	7.4.0	222	230	22.7
I) Dry Wt.	1.1.1	1.6.01	1045	1070	105.8	1035	× 5.7.4	030
J) Proctor Mt.	109	109	105	76.				O'e A
K) & Como I & (A	4					103	70.5	70.0
(A) & COIIID: 1 : 0	001	100	100	1001	100	99	100	99
L) % Moist								
A) - 000 - 0	2446	7+90	Ourto	2376	11+25	0771		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
	20017	1	220 17 8		250 17 6	4	72+35	0/+70
0) Elev.	7.23	7.22	1.00		7.78	1		
				, to the	(43	122	723	722

ompaction Required

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CONFIDENTIAL BUSINESS INFORMATION

Plant Wansley G-4100 Seni ROJECT: OB NO.:

DATE: 8/22/73 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

				8.5			111 (k	
4) Test Number	1.2	/8	61	20	12	22		
3) Probe Depth								83
C) Dens. Count	424	474	428	440	404	XCA	i i	
) C ÷ Std Count		# #E	1 0			07		
E) Wet Wt. (15/ft ³)	128.0	122.6	1275	126.0	130.2	127.8		
-) Hoist Count	1170	066	1090	960	1030	1027		
3) F + Std Count	o De	8	: 1 13:		: ::	0		
4) Wt. Moist (1bs)	73.4	191	21.5	185	20.1	19,9		
[) Dry Mt.	109.6	102.9	106.0	107.5 1101	1.10.1	107.9	(3)	,
J) Proctor Mt.	103	103	. 9.07	901		106	* *	
⟨ % Comp, I ÷ J	100	100	100	100	109	108		
-) % Moist								
() Location	11 +00	11+15	05+6	29 76	8+50	2 2 2 2		
	260'17	1	275 45	275.72	225 278	4		
)) Elev.	719	720	722	72.1	723	722		

Supaction Required

CONFIDENTIAL BUSINESS INFORMATION

Plant Wansley G-4100 ROJECT: OB NO.: REA:

Sep.

DATE: 8-22-73
DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

						72		
A) Test Number	0	01	1.	12	12	14		1
B) Probe Depth	-						7	9
C) Dens. 'Count	462	438	434	408	. 672	007	000	1
D) C ÷ Std Count		e E al l				1,70	200	777
E) Wet Wt. (15/ft ³)	123.2	126,3	126.7	1300	122.2	1237	132.7	1202
F) Moist Count	920	930	940	960	970.	9.20	1300	927
3) F ÷ Std Count		2		13 13	5 S	i.		
4) Wt. Moist (1bs)	17.5	17.8	18.0	185	18.7	17.5	21.7	0.7.
I) Dry Wt.	10.5.7	/ 08.5	108.7	111.5	103.5	1062	V///	4011
J) Proctor Mt.	103	105	. 60/	109		103	129	1.79
⟨ ⟨ ⟨ ⟨ ⟨ ⟩ ⟩ ⟨ ⟩ ⟨ ⟩ ⟨ ⟩ ⟨ ⟩ ⟨ ⟩ ⟨ ⟩ ⟨	100	100	100	100	001	160	180.	×
-) % Moist		# %						
4) Location	16+00.	16+25	90+91	51+91	13+85	14 400	1240.	12+75
12 IA	265 27 €	5	300 LT E	1	270,082	1	310, 27	1
)) Elev.	776	717	716	7/5	7/8	21.9	7/9	7/8

Mipaction Required

CONFIDENTIAL BUSINESS INFORMATION

> ROJECT: Plant-Hens-le 38 No.: Se D. D.

Sep. Dar

DATE: 8/22/73
DENSITY STANDARD COUNT: 25
MOISTURE STANDARD COUNT: 72

	_	T	1 1 1	7		7:-	- 1					8	-1			£1
	C		170	4/0	0.101	910	0//	172	0 00/	1.00.1	707			2+ T	20 3 20 3 20 3 20 4 20 4 20 4 20 4 20 4 20 4 20 4 20 4	111
	1		700	100	0 001	1020	0701	661	1079	102	101		11150	265278		7/0
	×.′S		120	170	1060	076.	00	185	1077	105	100		13+70			7/7
	5	es t			125.8			18.7	1071	165	100		13+50	317.097	7.0	@
	4		470		.122.0	1030 .*.		201	107.9 107.1	100	100		11+15	1	1.1	///
5.98	W	den 's a F	442		1.25.7	1040	7	20.3	1.05.4	103	. 007		11+00	3 17,097	7/0	(/0
	7		498		119.0	900		17:1	10:1.9	100	100		59+6	1	717	/ / /
	-		4.70	# *	122.0	1020		19.9	102.1	100	00/		0546	250,718.	7/8	
	1) Test Number	3).Probe Depth	<pre>c) Dens. Count</pre>) C ÷ Std Count	:) Wet Wt. (15/ft3)) Moist Count	3) F + Std Count	1) Wt. Moist (1bs)) Dry Wt.	1) Proctor Wt.	() % Comp. I ÷ J	.) % Moist		() Location)) Elev.	

mpaction Required .

CONFIDENTIAL BUSINESS INFORMATION

Technician /// /

PROJECT: JOB NO.:

DATE: 2 2 1 5 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

	-8	ŗ	1	10 10 10 10 10 10 10 10 10 10 10 10 10 1				
A) rest number		Ü		Į,	11			(
(8) Probe Depth	¢ :		e ()					
(C) Dens. Count	4.77	27.8	A.	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	113		7 7	
(D) C + Std Count				. 1	0 / 1	3); [-	ンプか	, , , , , , , , , , , , , , , , , , ,
(F) Wot Wt (12,	\$ X C ::		- 1				ERK	
יבי אפר ארי (וט/דני			() -		129,0	130.0	1. C.S.	7 7 7
(F) Moist Count	00 *X	1626		3 3	10 %		1	
(G) F + Std Count		.e.	81,21	 				
(H) Wt. Moist (1bs)		20,1	18,0		21.5	£ (*)		
I) Dry Wt.	\S Q	****	500	11.0.4	8.		3 - 1	17.9
J) Proctor Wt.				os	0	Š	165.1	55
K) % Comp. I ÷ J					, ,			
	=				100	/ (11.00	2.5/
25001 8 /2						± 1	7.0 7.	
N) Location	4.4						I.	
	150. 74	160'L d	701		1		1 1.0%	1/1/100
O) Elev.	713	717	7/3	16	Ø		7	
							1	7
ompaction Required	13.0 /6		CONFIDEN	CONFIDENTIAL BUSINESS				
	1 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	INTORMAL	2		`		

ompaction Required

CONFIDENTIAL BUSINESS INFORMATION

Technician ()

PROJECT: JOB NO.:

DATE: 3 / / / DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

SEIM	SENTIMENTOTO			:				
(A) Test Number	6	1/0		C/	~	7.7	1 n	
(8) Probe Depth		v		i e	1			. 2
(C) Dens. Count	72 6	777	474	152	277	07.0		
(D) C ÷ Std Count				41 41 41 M		<i>e</i>		
(E) Wet Wt. (1b/ft3)	124,0	122.5	0,521	124.5	1.23.5	25,607		
(F) Moist Count						= =	-1	=======================================
(G) F ÷ Std Count	085	かたし		37.6	940	(A)		
(H) Wt. Moist (1bs)	6.81	ં કે /		5/2/	0.8/		÷	
(I) Dry Wt.	1,501	104,5	1.201	51701	106,3-11055	NO.		:-
(J) Proctor Wt.	105	1050	£9/	7.701	10/3	200		5 73 73
(K) % Comp. I ÷ j	100	7.60	90/	9-/-	201	17.0	# # =	
(L) % Moist	U U U U U U U U U U U U U U U U U U U			a 3				
(N) Location	1140	5/ 1-11	15 (-6)		27 85	00/77	4.0 6.1	
(iv) Edcaeron	77,09 = 777,05 E	7,09%	7702 5	32424 3302 €	36012 2 3156 2	3 156 2		
(0) Elev.	6 a C	306	707	7/0	111	21/2		
			3	(A)		-	 	

Compaction Required

CONFIDENTIAL BUSINESS INFORMATION

PROJECT: JOB' NO.: AREA:

COX0 4711

DATE: AUG 21, 73
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

SEP	SEPARATION	LAB					=,=	
(A) Test Number	17	6/	6/	#20	211	22	100	7.4
(8) Probe Depth	119	▼				\		111
(C) Dens. Count	462	458	490	986	456	420	200	0 20
(D) C ÷ Std Count	123.5	1240	0,611	120.5	1240	10.70	1120	21.41
(E) Wet Wt. (1b/ft ³)		# 10 E	*					,
(F) Moist Count	1032	950	1072	1168	9/0/	0,01	1078	070
(G) F * Std Count	20.1	182		23.4	6'6/			
(H) Wt. Moist (1bs)	103.4	105.8	21.1	18		751	0	55
(I) Dry Wt.		106,0	626	93.4	104.1	137.4	/ 8(,	000
(J) Proctor Wt.	103.0	а В П	100	000/	163.0	¥	70,	
(K) % Comp. I ÷ J	10090	100%	626	93.4	100%	15.7	3,4,5	
(L) % Moist	28 19 1		UE KOLLED	REMONTHIS	030			
11 (A)	13 440	15+00	14490	16720	16+25	14+00	1400	05+21
(N) Location	225 17 6	225 L7 E	230,41.€	240,0H	230,716	350 19 8	320 17 2	32002
(0) Elev.	216	715	2/16	715	7/6	715	7/14	and of him
		A. A.	L16H7	LIGHT LATERIAL	TO N .	USED		
Compaction Required	1001	~	スクケ	X /	Tooka	M		U
					ועכוווויי	e l	()	6-1

CONFIDENTIAL BUSINESS INFORMATION

RENOTURE 0.00 220'44225'476 225'VE 430 98.3 0.96 48.5 13+50 13+20 120,0 110 23 103.2 1240 20.8 1030 1001 1056 454 DATE: 8/ 1/2 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT $\tilde{\gamma}$ 8/21/73 'oot 123,5 1039 462 1034 10/0 11+20 716 4 0 230/16 1240 458 020 6.61 1.40 200 11+10 5 03 00.00 225'46 18,7 79.0 500 9+70 N 948 1001 00 225'LTE 123.0 10.07 46A 080 6 04.1 103.0 9 +50 32050 1 1 3 6 N 14124 ţ* 7.7 00 0 ť £. in (81 901 EPAR ATION - 4 317.057 (1) 01.0 000 12/4-0 6 Ç. ジャ・・ 5 200 000 i Š 116 (E) Wet Wt. (1b/ft³) H) Wt. Moist (1bs) (D) C + Std Count G) F + Std Count ٠١٠ ن (F) Moist Count B) Probe Depth (C) Dens. Count A) Test Number J) Proctor Wt. (N) Location (K) % Comp. I) Dry Wt. (L) % Moist PROJECT: JOB NO.: Elev. 9

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required

Technician MIKE

PROJECT:
JOB NO.:
AREA:

DATE: 8 2 / - 3 DENSITY STANDARD COUNT: 23 MOISTURE STANDARD COUNT: 72

								*
(A) Test Number		21	le.	d	14	\.\.	•	0
(8) Probe Depth	12							. 5,
								9
(C) Dens. Count	4 4.4	246	3 £ .	シュナ	402	51	4- 	رد در
(D) C + Std Count	105.5	200	1. 22	L	1:0:1	101	0001	000
(E) Wet Wt. (1b/ft ³)	*.		352			u V	Ĭ.	12
(F) Moist Count	8.70	CO. /	0101	0000	050,	V V	(7117)	\$ 00 kg
(G) F + Std Count								
(H) Wt. Moist (1bs)	€, 30	6.0		1.00	60 0 7		0.0	Ei C
(I) Dry Wt.	2001	175.9	104.1	1016.	1000	アレン	0 6.1.77	1300
(J) Proctor Wt.	105	186	801	100	108	tç	14	
(K) % Comp. I ÷ J	2	1001	1001	1001	00/		19.67	3
(L) % Morst		11 146 13 16 16 16						
(N) Location	225.77 8	15/46	05+01	10000	2400	2115		
				21 21 21 22			4	
(0) Elev.	109	0.1	8 0.4	709	77.1	7/1		
	¥.			*			~	-

Compaction Required

Technician MIKE K

PROJECT: (4.77,15/2)
JOB NO.:

DATE: ()) S DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

		5) (1	Ω	Also.	580	•		3
(A) Test Number	9-	9/	14	7/	00	7	>1	11
(B) Probe Depth	iu –)	3/.
(C) Dens. Count	8/6	436	452	200	431	2675	E. W. (2)	113
(D) C ÷ Std Count				22 23 3				284
(E) Wet Wt. (1b/ft ³)	128.5	126.5	124,5	125.0.	1266	13/1/2011	101	
(F) Moist Count	994	886	= 70		000	1014		25.50
(G) F + Std Count						.,		
(H) Wt. Moist (1bs)	- ' '	1.61	22.2		19,3	22,0	1.7.6	11.
(I) Dry Wt.	1094	107.4	102,3	1501	105,1-107,2	70.01	000	
(J) Proctor Wt.	109.0	166. 3	700 5	07501	2.797		/W. 7.	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1
(K) % Comp. I ÷ J	160	/ CD 1		1.0	1001		1 / X /	
(L) % Moist	1						200	7.013
(N) Location	0h Hb	00+0/	12+50	124.60	scrti	05 1 km	16.154	19-91
	3-50,7 2	77,095	33624 34624	340.74	165 / 371/2	37.74	7006 y 310th	31.24
(0) Elev.	5/6	114	21/5	7/1	7/6	717	7/10	7/7
		100						

Compaction Required / 00

Technician (/ /)

> PROJECT: /A/2//2=/@ JOB NO.: AREA:

DATE: STANDARD COUNT: 23/ MOISTURE STANDARD COUNT: 737

(A) Test Number		7	17	4			(-	£
(8) Probe Depth	,, g			1		0		÷0
(C) Dens. Count	414	406	434	422	428	420	100	30 () Y
(D) C + Std Count	24 4 T	147) 141)	8 3				3.4.	7.0 7.4 7.
(E) Wet Wt. (1b/ft ³)	129.5	130,5	> 201	178.5	9.761	127.8	3.00	
(F) Moist Count	826	416	366	266	922	0.00	9011	20.00
(G) F ÷ Std Count		a 43	2				176	126
(H) Wt. Moist (1bs)	6.81	18.5	18.9	18.9	17.5	7.8	6 0/	2)
(I) Dry Wt.	110,6	1, 2, 0	107.6	7691	1096.100	1,97	107. 3	1000
(J) Proctor Wt.	0601	0.601	1. M.C. 3	1050	706.7	050/	2 707	109.0
(K) % Comp. I ÷ J	1600	1 101	10104	+40/	1.6.6.4	70.4	000	
(L) % Moist								
(N) Location	10-5	10764	07 - C1	12+20	15.100	3.20	11+10	~ \ I-11
- (a)	2254 6 2304 4	230.6 &	260'L & 270'L &	\$7,002	J 2007 €	77582	1001/4	1001
(0) Elev.	7/3	714	416	7/3	516	216	714	715
				538		*		

Compaction Required / 0

CONFIDENTIAL BUSINESS INFORMATION

Technician ()

PROJECT: JOB NO.:

DATE: 8/21/75 DENSITY STANDARD COUNT:

					81			
(A) Test Number	1	. 26	T	л 1 2	e. 6	*I		
(8) Probe Depth					8.		5.3	
(C) Dens. Count	446	457	- CG					
(D) C ÷ Std Count				3				i i
(E) Wet Wt. (16/ft ³) 125,2	125,2	124.2	12.1.2				1	
(F) Moist Count	1050	970	0511				; = =	
(G) F ÷ Std Count	20.6	17.3	23.0					
(H) Wt. Moist (1bs) /	104.6	6.96.	48.0	2 =				
(I) Dry Wt.	103	501	99	akr sak er s Ek			-	
(J) Proctor Wt.	8			11	82			
(K) % Comp. I ÷ J	100	770	100	10 11				
(L) % Moist) (2			- 10 - 10 - 10 - 10		2 5.	
8	05+61	09701	02 + 0/					
(N) Location 3 9	340 27 6	Marie Commence	4. A.					
(0) Elev.	713	712	710					

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required

PROJECT: JOB NO.: AREA:

DATE: 8/20/25
DENSITY STANDARD COUNT:

		2 = 2						
(A) Test Number		. 2	PG.	-1-	6	*	break	00
(8) Probe Depth								
(C) Dens. Count	404	430	130	432	2/6	्र र	70.8	6
(D) C ÷ Std Count		14					S .	O .
(E) Wet Wt. (1b/ft ³)	125.5	1272	127.2	1270	1290	1 00 00 00 00 00 00 00 00 00 00 00 00 00	120.2	
(F) Moist Count	1070	1100	01.6	020/	1030	1040	0::-	7007
(G) F ÷ Std Count	82	24 30	:: ::	3 3 3		_		
(H) Wt. Moist (1bs)	21.1	0.00	18.7	20,1	20.1	25.3	2000	20.1
(I) Dry Wt.	4.00	105.2	7.08.5	6301	108.9	67.01	a Ev.	0 40
(J) Proctor Wt.	C. 60 C	දුර ව	106.0	106	7× G	× .		
(K) % Comp. I ÷ J	, 00,	120	6.04.5	* * * *			s s	
(L) % Moist	5 19 ³ 4							
	00+11	10+85	12+00	OCTO	00751	5/+31	00++1	4-120
(N) Location	320' L7 &	1	1	*	325618	^	317,908	1
(0) Elev.	708	707	710	601	I.	0.7	1, C.	760
				*			7 2 1	

Compaction Required

Technician M.

PROJECT: 4: 1) 1.5 /e.
JOB NO.:

SEDALATION DIA

DATE: 2 20-73
DENSITY STANDARD COUNT: 23/

JEVIN	JEMPANION VIKE	The same of the sa						
(A) Test Number		2	M	17	1	2		
(8) Probe Depth	117							٠ <u>٠</u>
(C) Dens. Count	464.	200	769	456	421.	400)	417	11.11.
(D) C ÷ Std Count	73 27 2 3 4 4		1		9		1	<i>L</i> / <i>Y</i>
(E) Wet Wt. (1b/ft3)	123.0	119.0	123.0	124,0	121 5	100	1001	
(F) Moist Count	892	978	934	206	1 166	086	777	
(G) F ÷ Std Count	3 3 4 5	2		10			2016	3701
(H) Wt. Moist (1bs)	11.9	15.8	17.8	17.1	19,7	1.61	20 6	7 5
(I) Dry Wt.	106.1	103.2	2,201	1.06.9	1077	1000	(0 V	
(j) Proctor Wt.	106.7	/.0 >	105.		1/10/1/	0 1 / .	1, 20,	/ 101.
(K) % Comp. I ÷ J	100	00/	180	760	/20	100/	76.11	00 /
(L) % Moist								
(N)	08+01	56+01	11+25	1.871	13+25	SOFU	14/160	1620
	200 LL 210'2 G	18.5	2252 4 3502 4	73056	20074 22024	220724	3006	21562
(0) Elev.	806	209	709	710	711	216	7 /0	1 1 1
		i*				+	, , , , , , , , , , , , , , , , , , ,	

Compaction Required 100

CONFIDENTIAL BUSINESS INFORMATION

Technician U extstyle D / - / U

PROJECT: JOB NO.:

on the perchase was

DATE: E//:7 / 75
DENSITY STANDARD COUNT:

				74				3
(A) Test Number	5	0,		17	1). 2 1	in a		
(8) Probe Depth	: }:Û	Six inche.	· ' ' '	117				0 10 0 10 10 10 10 10 10 10 10 10 10 10 10
(C) Dens. Count	را. ن	440		202				
(D) C & Std Count								
(E) Wet Wt. (1b/ft ³)	10 - 10	12.6.7	0.641	5.51				
(F) Moist Count	43.4	000,	1050	0.00				
(G) F ÷ Std Count	· ·	ж ^п .	3. 3.	120 21 21 3				
(H) Wt. Moist (1bs)	(r)	· · · · · · · · · · · · · · · · · · ·	. 10		-	2		3 E 5 E
(I) Dry Wt.	オンつ	105.7	2.07	1.08.27		2 70	63 M6	
(J) Proctor Wt.		· ` ` (· ·	5		***	
(K) % Comp. I ÷ J	100	00/	100	00/				54 5 8
(L) % Moist					100 100 100 100 100 100 100 100 100 100	·		
(N) Location	12400	142112	13-1002	13+20				
(0) E1ev.	-104	\$000 1000	702	1001		1 m		Ē
		17.0				-	***************************************	

Compaction Required

Technician M. II

DATE: 8/17 CEDENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

PROJECT: JOB NO.:

H THE

			Ť	E)				
(A) Test Number	525	Ci.	ا. اير:		ν,	A. !	1	T)
(3) Probe Depth	NS	7.0	.; e		= 0		, ,	
(C) Dens. Count	() (0) T		740		:: : : : : : : : : : : : : : : : : : :	A 0.0	0 7 6	400
(D) C ÷ Std Count	·	5 5 2 2			3			1
(E) Wet Wt. (1b/ft ³)		10.8.2	1263			100000	127.41	0.841
(F) Moist Count	126	050/	1210	12.2	0:0/	11.30	1000	(20)
(G) F ÷ Std Count	-			2047 047 25 26 1	s.*) } :		5
(H) Wt. Moist (1bs)	25.4	1.02	22.2	2.5	۲. ن وي	4.00	10 % 1	0.0
(I) Dry Wt.	8.707		1001	. C.1.C.	6.2.0%	 00:0	1002	4101
(J) Proctor Wt.	0		.00.					
(K) % Comp. I ÷ J	100	100	100	100	00/	100	PQ/2	100
(L) % Moist		#1 S4 S4	<u>s</u>				# (F	mi.
(N) Location	22.55 32.527 E	0: 7=1	11450 370272	07 411	57.570	10450 325'A	13475 13440	24.70
(0) Elev.	70.5	631	\$ 97 <u>-</u>	N N		007	707	

CONFIDENTIAL BUSINESS INFORMATION

Technician /

Compaction Required_

PROJECT: JOB NO.:

DATE: 1 - 16 75 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

The state of the s		a A	3 3					
(A) Test Number	0	<		(/	2/	17		2 F2
(B) Probe Depth	7	* 12 22 (23) 0. [1]						
(C) Dens. Count	797	472	440	7.27	1.51.	1. 1. 2.	i .	8.9
(D) C + Std Count				9 7	3	764		
(E) Wet Wt. (1b/ft3)	123.0	123.0	125.0	0.801	10 40 10 TO	127 1		
(F) Moist Count	1084	P511	3001	1100	1076	17.25		
(G) F ÷ Std Count	27	# ≥ #84	8			2		
(H) Wt. Moist (1bs)	22.5	~ PC	22.5	23.0	21.0	77.0		
(I) Dry Wt.	100, 4	108,5	102,5	105,0.103.0	103.0	101.0		
(J) Proctor Wt.	, 007°	< > 0/	7.60	0.2.9/	0.10.		# 1112	
(K) % Comp. I ÷ J	. 707	00/	00/	\$ 2.7	100		m	
(L) % Moist	140 121 131 131 144				>		1 . i	
(N) Costica	1.00 Til	in Exten	12416	3666)	14-100	07+41	22	
	3 20 5 E	360SE	75085	3805 4 3605 A 300 5 4 3163 4	780 8 4	75315		* * * * * * * * * * * * * * * * * * *
(0) Elev.	704	205	205	206	206	101	2	
						+	7	

Compaction Required

Technician

7. 050 \mathcal{Q} ンング 1086 3 CA ... -t-07.0 16 DATE: / / / / / DENSITY STANDARD COUNT: 13475 27.0 \triangleleft 1070 507 180 2 4 2 128.0 (, 106.3 424 r } 77.7 7505 (j.) 2/11 201 Ó 10 7 Ŋ 22051 2000 0'9 01 5011 J. C. っろナて 100 705 106 N · 1810 3601 0.27 400 17051 11 +30 50 105 10 0 100 75081 100 . S ニナシャ 442 08% 10.7 50 211 とのク 40% 129.0 O~ ○~ いの名 000 416 してたら 70.7 0 90/ 3. 00 0 19054 123.0 0 Z S 1222 464 1 10+00 00, , <u>.</u>; r 707 (5 14 (E) Wet Wt. (1b/ft3) H) Wt. Moist (1bs) (D) C + Std Count G) F + Std Count C) Dens. Count A) Test Number (F) Moist Count B) Probe Depth J) Proctor Wt. K) % Comp. I (N) Location L) % Moist I) Dry Wt. Elev. JOB NO.: AREA: PROJECT 0

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required

DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT: PROJECT: JOB NO.:

West Mr.	War for how low							
(A) Test Number	7	/0		77	/3	17/		
(8) Probe Depth	" "							
(C) Dens. Count	4.72	416	85/2	777	462	41.8		
(D) C = Std Count	15 26	1200	7867	0561	2772	2025	18	
(E) Wet Wt. (1b/ft ³)	125.0	0,05	12410	015 ()	1255	122.		*
(F) Moist Count	11.12	1084	515	1642	1168	1200		100 100 100 100 100 100 100 100 100 100
(G) F = Std Count	775	852	777	817	168	747		
(H) Wt. Moist (1bs)	22.0	2),5	0,1,7	203	22,6	24.0	=	
(I) Dry Wt.	0.901	16715	13.5.0		5,101	5.26		
(J) Proctor Wt.	/(16.5	16.3.	100	5,)0/	36	36	8 =	==
(K) % Comp. I ÷ J	00/	200	ج- رز،	<i>(</i>)	100	700		
(L) % Moist			0 0 0 0		s e		3 32 3	
(N) Location	1 3	5 U	7.02	1. 1. Z.	30 th 30			
(0) Elev.	7.76	705	201	7.01		206	3	5
	7 21			5221	7 6	2		

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required

PROJECT: JOB NO.:

DATE: \$ - (3 1) DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT

111111111111111111111111111111111111111	1011				ar ti			
(A) Test Number	, -	2	M	4	1	7		(1)
(8) Probe Depth	29				7	0		Þ
(C) Dens. Count	480	478	420	476	467	470	15	
(D) C - Std Count	2018	7069	5181	1394	766	2/1/2	5/6	776
(E) Wet Wt. (1b/ft ³)	0.12/.0	121.5	128,5	27.5		7901	77.1	76.2
(F) Notst Count	1232	1130	436	9.00	<u>L</u>	10.70	1266	11130
(G) F - Std Count	9.67	688	7.36	806	888	1.58	2 2 2 2	070
(H) Wt. Moist (1bs)	, 24,1 24,1	27.5	0,81	01/1	21.0	311	0.36	7 (
(I) Dry Wt.	, ', 's'	970	5011	1.6.5.	1076	ומרום	7 7 7	
(J) Proctor Wt.	0.8/	$\mathbb{R}[\Lambda / r]$	101	101	103	, , ,	1 32	0.000
(K) % Comp. I ÷ J	98	/ 60	00/	00/	66	207		0 /
(L) % Moist	ReRuled			, h	RCK	Rekelled-		
(N) Location	08+11	14465	13 + 00	13420	12720	17+10	1/7:0	21:-11
	130/€	1701	150'E	140,0	5,21/	1.21	51 70%	= 77/
(0) Elev.	70%	900	703	704	701	201	707	707
						 	۱)

Compaction Required

EMSCOFWEST. TOE

Technician

PROJECT: JOB NO.:

EPARA FION

7522 DATE: 8/13/73
DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

		50	01						
(A) Test Number		a	<i>c</i> ~	4	\(\tau_{}		long	C	
14 200 Show (0)	11							e G	·
undan annu rol	9	7				√O			
(C) Dens. Count	264	777	2) (C)	120	458	FOR	497	252	4
(D) C + Std Count	£•		ii.	3	175	& & ~	C\$ 0X	100	-}
(E) Wet Wt. (1b/ft ³)	3 0501	196.0	37.6.5	124.8	1270	1230	1220	101.11	
(F) Moist Count	C8.7	1160	1200	1000	1660	Ca	057	2000	
(G) F + Std Count					60%	775	1 1 10	750	lle.
(H) Wt. Moist (1bs)	2. le:	71	0 7 6	19,2	27.5	18.5	000	2 7 7 0	
(I) Dry Wt.	1014	1008	105.4	105 2	1,005	1040	376	1063	
(J) Proctor Wt.	/00	/90	113	- Sammer	100/		0.15		
(K) % Comp. I ÷ J	001	100	73.3		TVV/	せる	- 77	7. \	
(L) % Moist							+000		
	11-450	11765	127-7-25	06 761	13400	(3+20)	1750	0 0+11	
(N) Location	\$200E	N	200 476	1	300718		- value pudpingung - quidananan annua mananan		9
	100	# 							
(0) Elev.	700	6.6.9	700	669	705	703	707	703	, ii
							***************************************	,	

000 Compaction Required

CONFIDENTIAL BUSINESS INFORMATION

Technician MIKE

DROJECT: W Ansland
JOB NO.:

DATE: 8 - /2-275
DENSITY STANDARD COUNT:
MOISTURE STANDARD COUNT:

(A) Test Number	6	9/	/	17.	13	14			
(8) Probe Depth	79								
(C) Dens. Count	404	406	4/6	406	4/4	420			
(D) C ÷ Std Count	2	*.				140			
(E) Wet Wt. (1b/ft ³)	130.5	130.0	129.0	130.0	129.5	127 6			
(F) Moist Count	11.72	1116	8011	780/	707.2	991.			
(G) F ÷ Std Count									-
(H) Wt. Moist (1bs)	23.4	23.4	22.0	21.7	19.9	193			***********
(I) Dry Wt.	107,1	106.6	107.0	:		1.0 2			
(J) Proctor Wt.	766.2	€.90/	<-90/			6.5 11	·	*3	
(K) % Comp. I ÷ J	/0⁄i	100	an'	180	/01	111			
(L) % Moist						22	e e	·	
(N) Location	13+75	58+61	sont 1	14710	es+W	12 +60	÷		
	2007 4 1851 4	18516	191	10.26	7 1.001	1201/4		. =1	
(0) Elev.	only	406	703	702	655	700			181.
				T					

Compaction Required 100

CONFIDENTIAL BUSINESS INFORMATION

NUCLEAR DENSITY METER WORK SHEET

PROJECT: JOB NO.:

SEP MM

DENSITY STANDARD COUNT:

(A) Test Number	_	2		4	\	7		0
(8) Probe Depth	119	29	3.9),,,	,,,	7		D
	7/5			2	9	٥	٥	
(C) Dens. Count	42X	430	426	404	452	450	11/6	412
(D) C ÷ Std Count			127.5	130.5	124.5	124.5	138.0	129 5
(E) Wet Wt. (1b/ft3)	122.5	1270	7				2	2,17,
(F) Moist Count	976	pc07	800/	09 01	2111	972	9/9/9	(013
(G) F ÷ Std Count								~
(H) Wt. Moist (1bs)	18,9	1121	19.6	20.8	72.0	9 1847	7 8	, ,
(I) Dry Wt.	1036	1000	1079	1007	100	100	18.1	77.0
				, ,	1012	105,8	110,5	107.5
(J) Proctor Wt.	701	C. 90/	(06.3	108	<i>حام/</i>	106,3	705	10%
(K) % Comp. I ÷ J	SD /	98	1001	100	100	66	1.67	47,
(L) % Moist							20)	001
(N) Location	11435	54+11	11+50	SL+11	12+85	12775	13400	13+15
	Along Li	tch -		1	100'R.	12 OF F	1202 6 720'2 4240'R	240'R &
(0) Elev.	699	359	859	659	859	\$19	702	Car
								}

Compaction Required_

CONFIDENTIAL BUSINESS INFORMATION

both were 28.8.

PROJECT: JOB NO.:

DENSITY STANDARD COUNT:

	1	ŀ	X						
	7:00 Am	7:05 Am	7/30.40	1:30 pm	3.35.pm	2.30.000			
(A) Test Number		ત્ર	M	1.	6	19	ad ex s	3.30 pm	
(8) Probe Depth			1.9	1,9	(")	1.9	"	3	
(C) Dens. Count			226	252	254	121	3 6	2 6	
(D) C ÷ Std Count			1.725	1.930	1940	0881	4621	120	
(E) Wet Wt. (1b/ft ³)	=	:000	122.80	122,00	121.50	13300	17650	133 069	
(F) Moist Count	755	676	847	575	458	672	543	SHS	
(G) F + Std Count	0.850	0.890	1111	0.755	0.605	0.883	0.718	0.716	
(H) Wt. Moist (1bs)	23.50	21.75	28,20	17.75	13.50	21.50	16.79	16.75	
(I) Dry Wt.	= - h		966	10325	108,00	101.50	103.75	164.36	
(J) Proctor Wt.		(10.00 9	106.3	1030	(06.3	106.3	103.0	1.06.3	
(K) % Comp. I ÷ J		= =	346	100 40	10001	0/0 26	180%	100 %	
(L) % Moist			N 5°	e.	E -	i IC			
(N) Location	Str. 13 too 125' E.ut W. Toe	5tg. 13tg5 120' E.oth, Toc	5t#, 13t00 150' E.onw. Toc	\$10.14400 \$0.100	5tp. 12+50 75'W.o.t.	Sta. 13too 85' Word Core	5tp. (2400 to W. of	5th. 13400 86'W. 04 6050	
(0) Elev.	701.0	7010	2010	697.0	697.0	6.82	1.00.	Kotest of the Co	
	* (+ Test	+ Test # 142 for mosture only	sture only			0 %	
							1		

10

Compaction Required

Technician

PROJECT: JOB NO.:

DATE: 8// 2 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

	4.3c por	14.4 PM	PL.40 PM					
(A) Test Number	6	- Z	01				11	
(8) Probe Depth	2/2	Y	200					
(C) Dens. Count	235	1	231					3 000
(D) C + Std Count	1.795	St.	1 7%0					
(E) Wet Wt. (1b/ft ³)	125,50	Scol	(26.00		3			
(F) Moist Count	578	540	560					
(G) F ÷ Std Count	0,760	4786	0.735					**************************************
(H) Wt. Moist (1bs)	18,00	926	54.71					
(I) Dry Wt.	107.50	一种人	108.75		=			
(J) Proctor Wt.	106.3	JAN NO	166.3			i.	2	
(K) % Comp. I ÷ J	120%+		18007					
(L) % Moist								
(N) Location	5%- 11450 35' M of core		340.11450 43°W.00 coru	=			9	
						3		
(O) Elev.	698.0	1 2 2	863					
		•						

CONFIDENTIAL BUSINESS INFORMATION

Compaction Required

Technician

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GLUNGIA LUNER CUMPANY - SUILS NUCLEAR DENSITY METER WORK SHEET

PROJECT: JOB NO.:

DATE: 8/10/7.3
DENSITY STANDARD COUNT:

			2					
(A) Test Number		a	3	4	8	7	1	Ø
(8) Probe Depth	jo	"9	119	"9	119		111	
(C) Dens. Count	206	222	206	224	2//	716	210	3 6
(D) C ÷ Std Count	1.57	1,70	1.57	1.71	197	777	2/2	6/2/
(E) Wet Wt. (1b/ft ³)	133.00	128.50	133.00	00.861	131.00	129.50	131.00	131.00
(F) Moist Count	625	634	703	688	632	949	1,38	12.50
(G) F ÷ Std Count	0.900	0.830	0.924	0.405	0.830	0.840	2/2	2 8/20
(H) Wt. Moist (1bs)	22.00	20.00	22.75	کته بحدی	20.00	20.75	20.25	20.00
(I) Dry Wt.	111.00	108.16	110.25	106.75	111.8	108.25	100	00:00
(J) Proctor Wt.	106.3	106.3	106.3	106.3	106.3	106.3	111.3	12/2
(K) % Comp. I ÷ J	1008+	100%+	+ 9001	14081	12001	t&021	13.67	11184
(L) % Moist								000
	570.13.400	5ta. 13400	5tp. 11450	Sta. 14+50	540.14750	Sto 13+00	5/2 13+10	STA 1720
(N) Location	180 E. al W. Toe	10 E. of W. To.	110 E. of W. Too 160 F.OF W. TOC	150 Foll. W. toe	156 E ON WIDE	So Ed Wtoe	55 E. ophotoe	SO E. of M.
	700	74.	1.00					130
(0) Elev.	1000	0.001	677.0	705.0	7050	700.5	700.5	700.5

Compaction Required_

Technician

PROJECT: JOB NO.: AREA:

DATE: 8/10/23 DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

(A) Took Mark	7	0	2	27				
A) Test Namber		Ö	7	10	//			
(8) Probe Depth	6"	"9	19	",9	"9	 12		
(C) Dens. Count	346	224	231	225	145		-	33
(D) C ÷ Std Count	1.88	1.71	1.76	17.1	184	18		
(E) Wet Wt. (1b/ft ³)	123,00	12800	126.50	138.00	124,00			
(F) Moist Count	863	755/	693	Saz	728			
(G) F ; Std Count	1.13	. 990	.910	1,080	0.620			
(H) Wt. Moist (1bs)	28.50	24.75	22,25	2725	24.26			20
(I) Dry Wt.	94.50	107.25	104.25	100.75	99.75		1	
(J) Proctor Wt.	166.3	106.3	106.3	106.3	106.2			
(K) % Comp. I ÷ J	89%	+95081	98%	25%	2676	11 II	-	
(L) % Moist	- R			: a				
(N) Location	5th. 13too 125'E. OPWTOO	5% 12+50 125'E.O.M.TOC	5ta, 13t00 135' E. et W Tae	5th. 13+00	Sta. 13+25'	- - 		
(0) Elev.	706.0	700.5	200.0	701.0	701.0			
Compaction Required	100%		Note: Material "Note: Moisture the Speedy" in was 2the "	Moisture was to wet, fill was staged Moisture was checked want falso a Speedy" in this areasit Technician	fill was stapped. Ked watth Also with	a. with Burn		

CONFIDENTIAL BUSINESS INFORMATION

PROJECT: WANSLEY
JOB NO.: SEP. DAM

762 DATE: AUG DENSITY STANDARD COUNT: MOISTURE STANDARD COUNT:

(A) Test Number	_	7	n	X	5	7		0
	111	7)	,		0		Ď
(8) Probe Depth	0	2	E)	-				
(C) Dens. Count	366	484	796	790	490			
(D) C + Std Count	6. deg	1.85	687	1.87	1.87			
(E) Wet Wt. (15/ft ³)	126.0	1240	/23,0	123,5	123.5	×	Ł	-
(F) Moist Count	1294	13/6	1426	1/88	1258			
(G) F ÷ Std Count	0.850	0.863	0,935	0.780	0,825	٠,٠	177	1 1
(H) Wt. Moist (1bs)	20.5	21.0	23.0	18.5	20,0			
(I) Dry Wt.	105.5	103	1000	10500	103.5			
(J) Proctor Wt.	105	103	103.0	0'50/	103	S.		*
(K) % Comp. I ÷ J	100%	10090	9790	100%	1.00 tg)			
(L) % Moist			RENOLLED			5	10	
(N) Location	13 +50	13400	3400	13+50	13+25			
	200 WG	200 WE	250 WE 250 WE		225 W E			
(0) Elev.	969	697	697	698	969			
						2 . 2	T	

1000% Compaction Required

Technician EDWANDS

PROJECT: WANSLEY
JOB NO.:
AREA: SEINKAIDOW

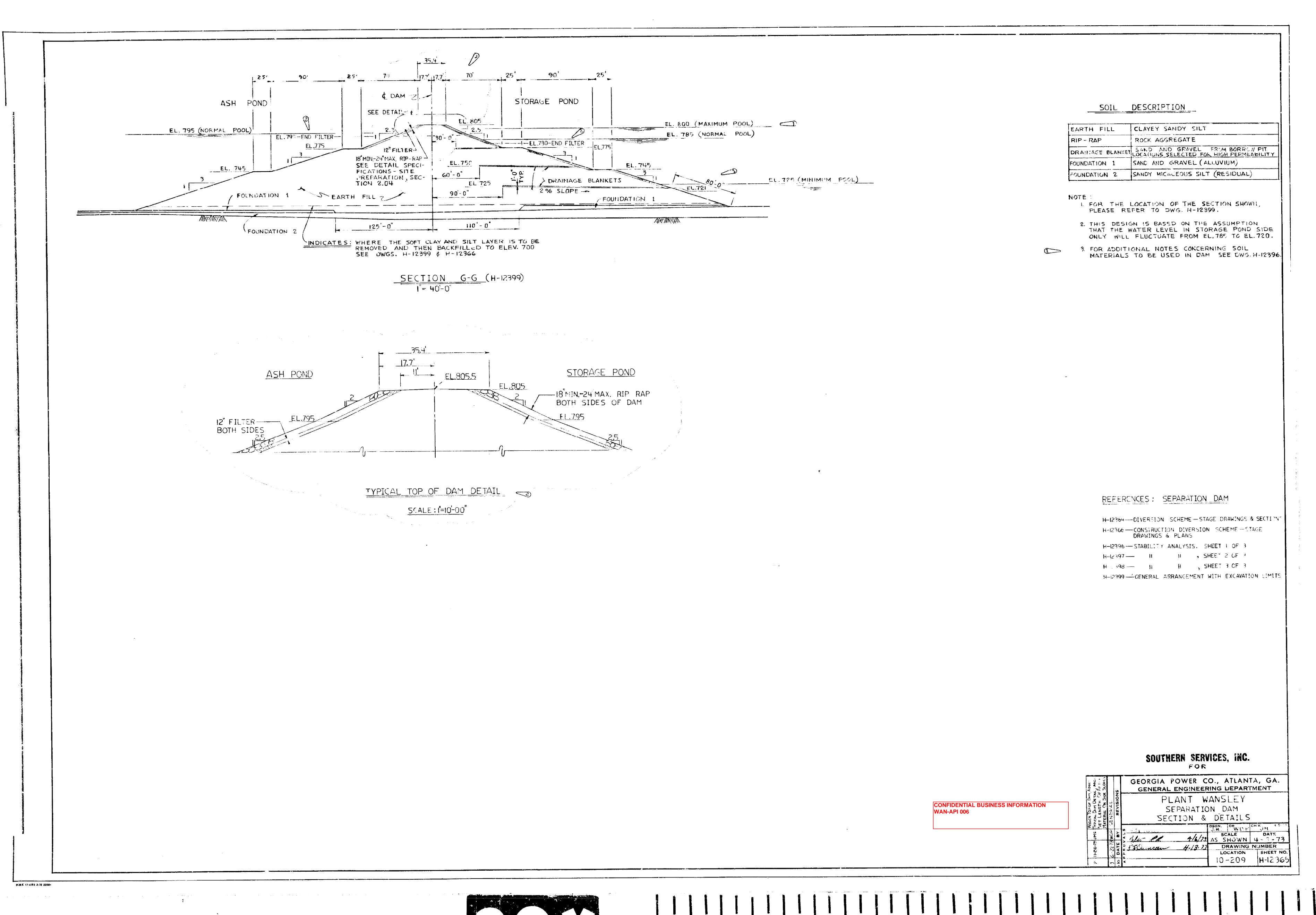
SEPAKADOW DAM

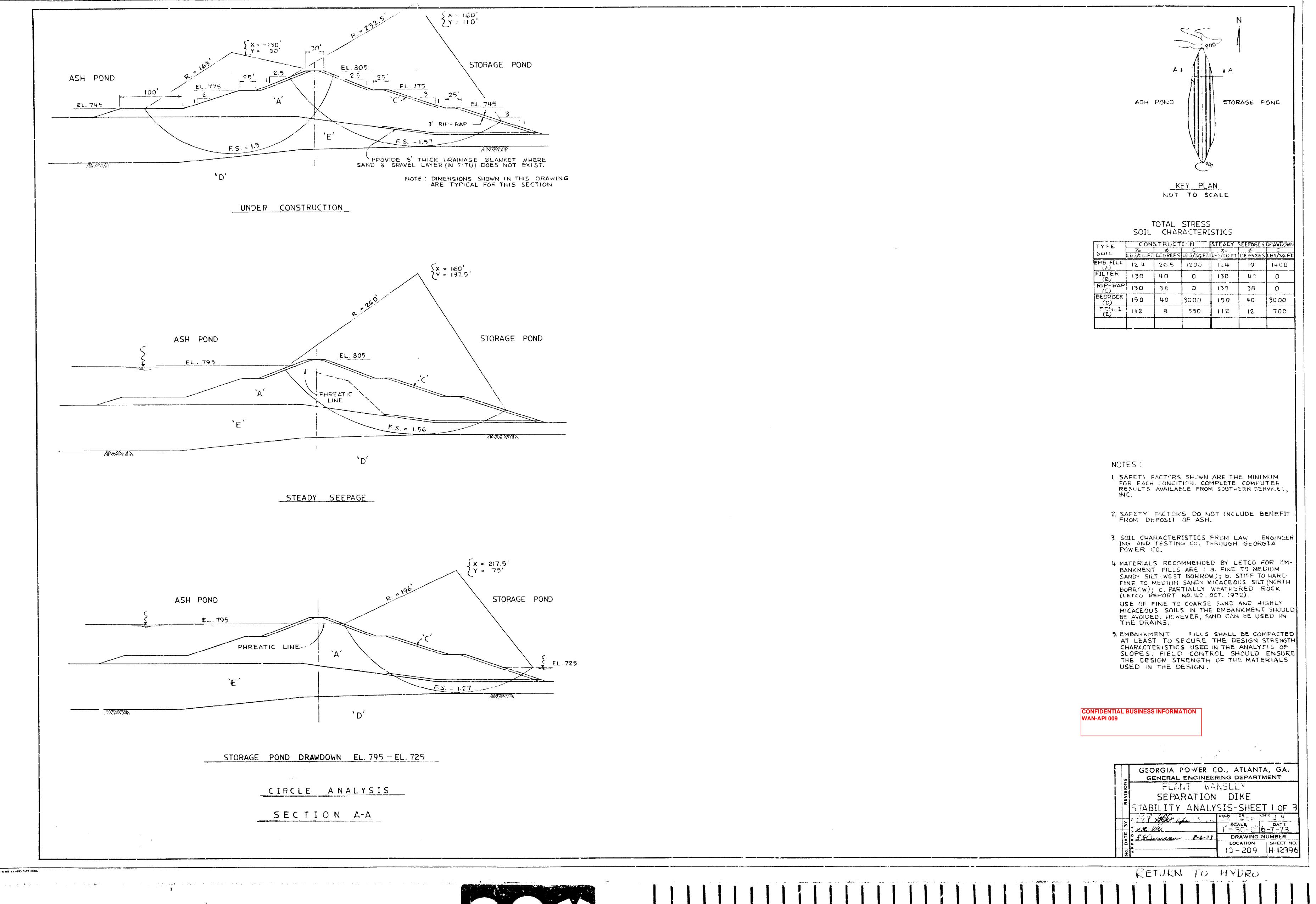
DATE: AUG 6 1973
DENSITY STANDARD COUNT: 251 262
MOISTURE STANDARD COUNT: 251 152

			×-					
(A) Test Number		7	3	4	8	9	7	a
(8) Probe Depth		9,,9	,,9	,,9	119	1,9	1,9	23
(C) Dens. Count	1250	504	4 94	5 4 8				
(D) C ÷ Std Count	124.83	1.92	1.89	2.09	K			= = = = = = = = = = = = = = = = = = =
(E) Wet Wt. (1b/ft3)	124.5	122.0	122,5	117.0	: :			
(F) Moist Count	1038	1226	1670	1086				
(G) F ÷ Std Count	0.683	O. 805	0,704	0,7/3	39			
(H) Wt. Moist (1bs)	15.8	19.2	16.3	9.9/				
(I) Dry Wt.	108.7	102.8	106,2	100,4	190			
(J) Proctor Wt.	101	703	0.901	103		2		
(K) % Comp. I ÷ J	100 to	100%	+ %,00/	9790	· ·			
(L) % Moist		37	1	REKOUED				
(N) Location		14 +00	13+50 50 7 63	13440	=			
	10 E. BY	TOF	W. TOF	W. 70E				
(0) Elev.	695	569	469	56 9	c			
		20						21

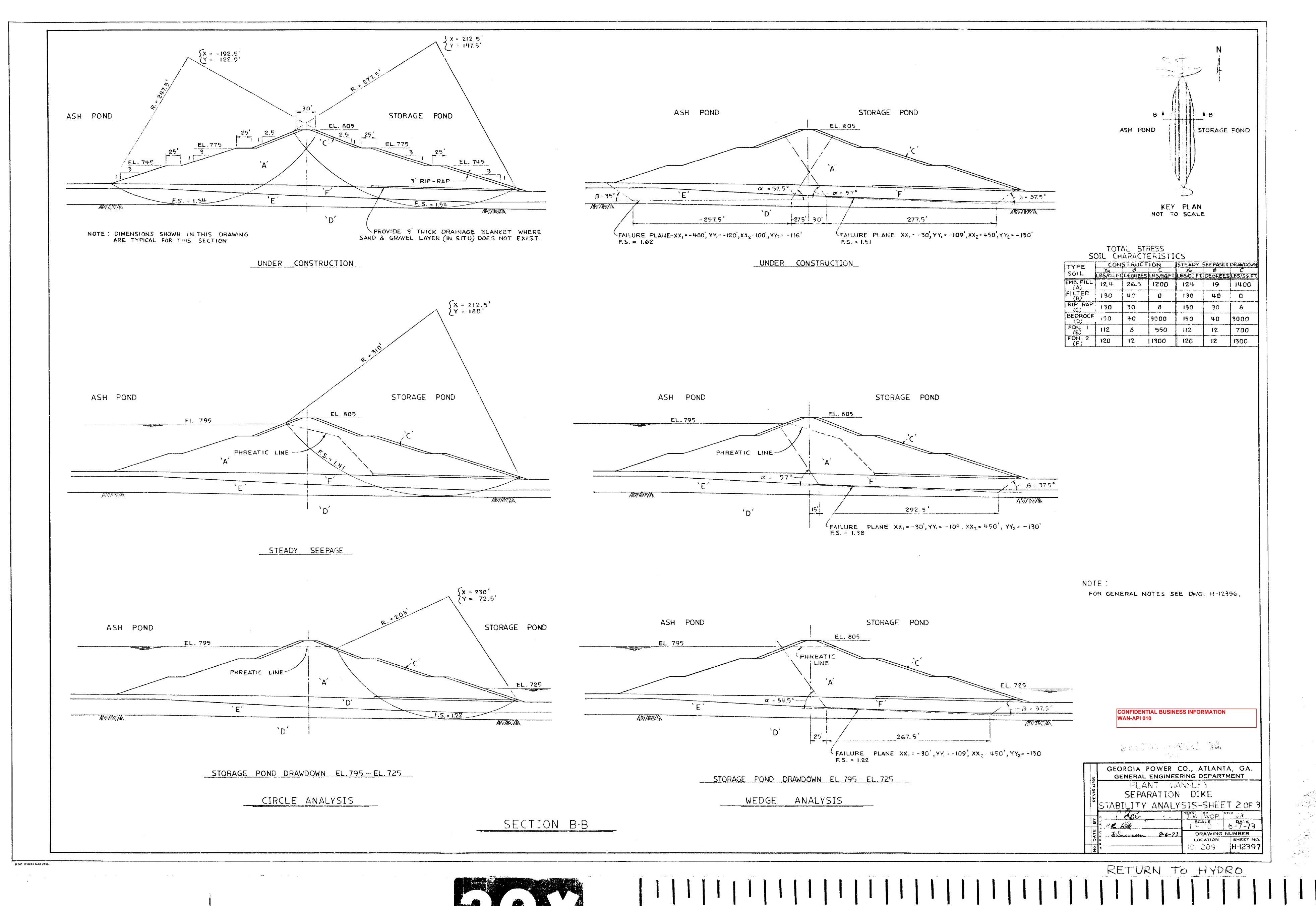
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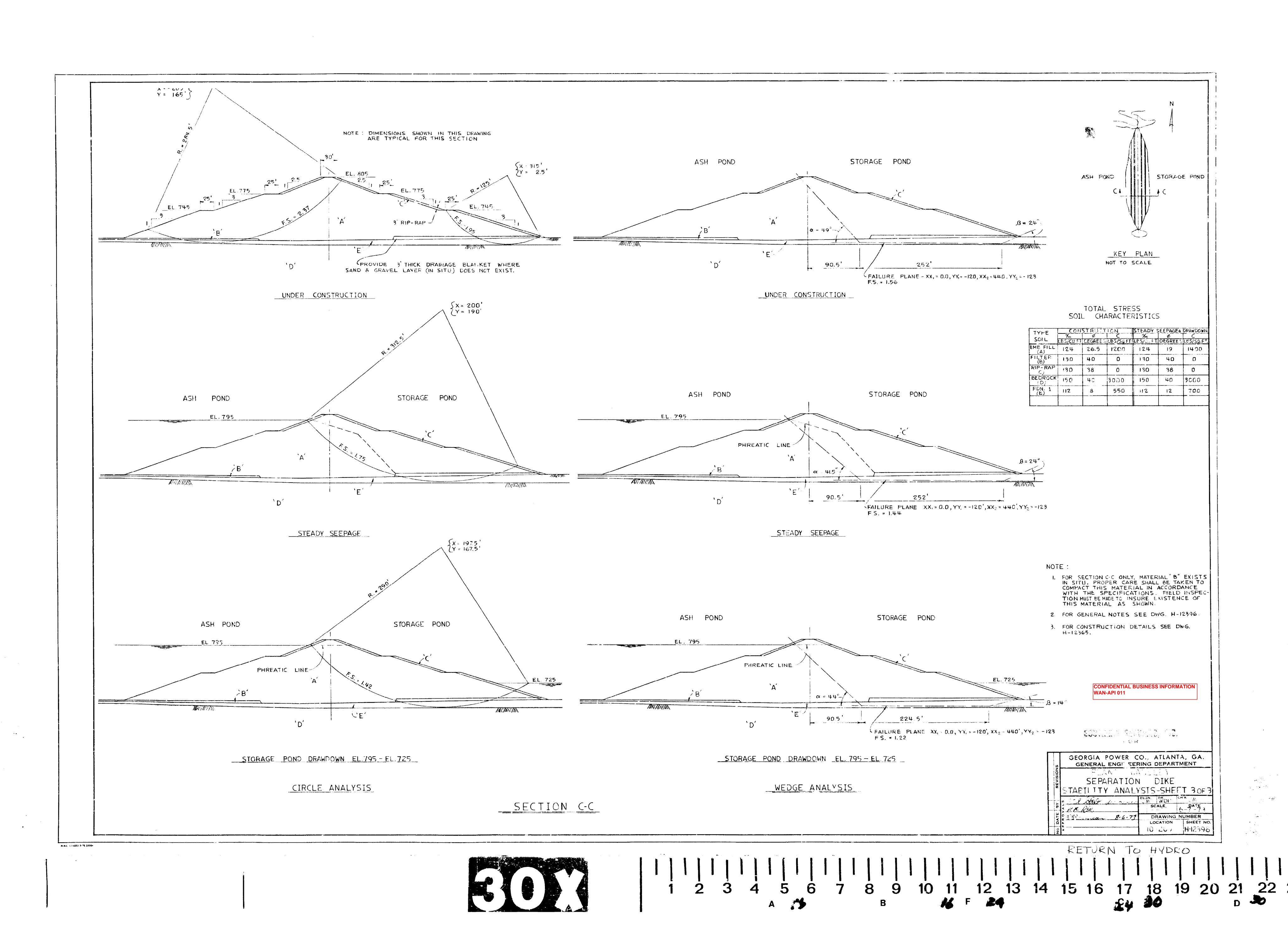


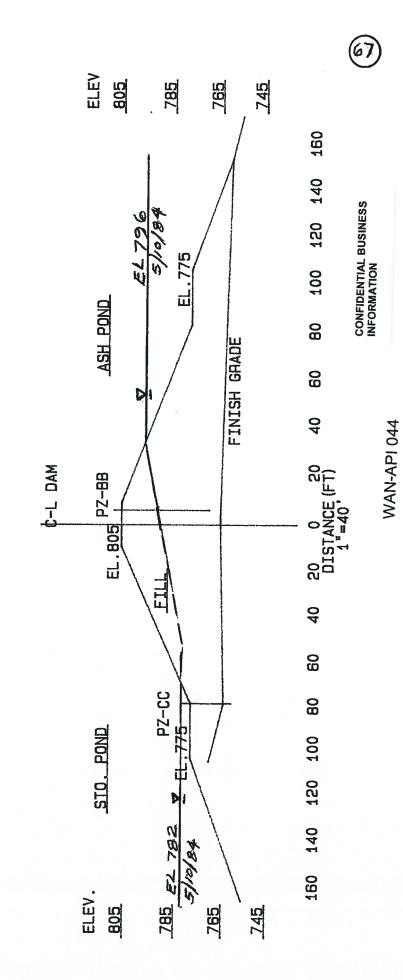


1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 2



10 11 12 13 14 15 16 17 18 19 20 21 22 2

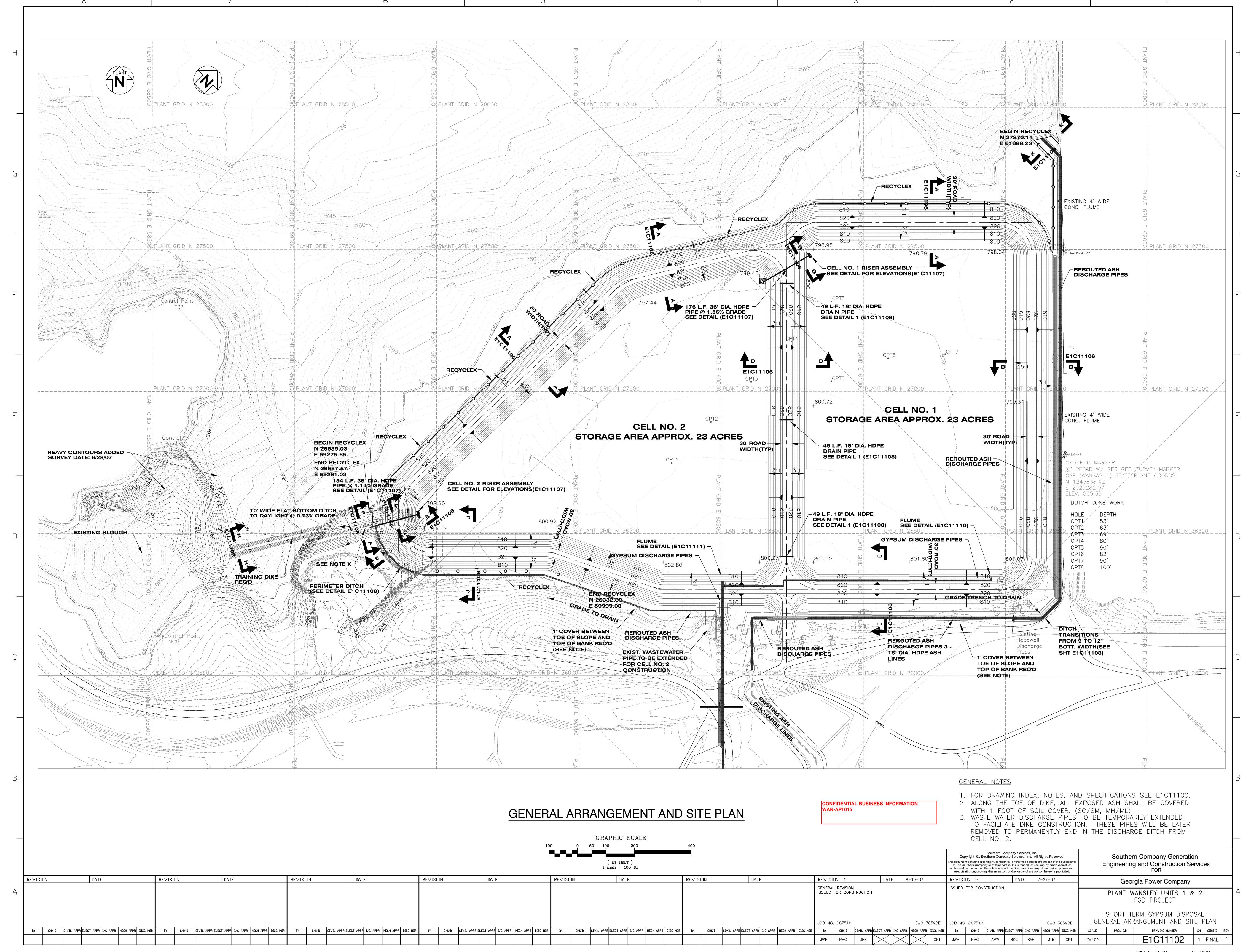


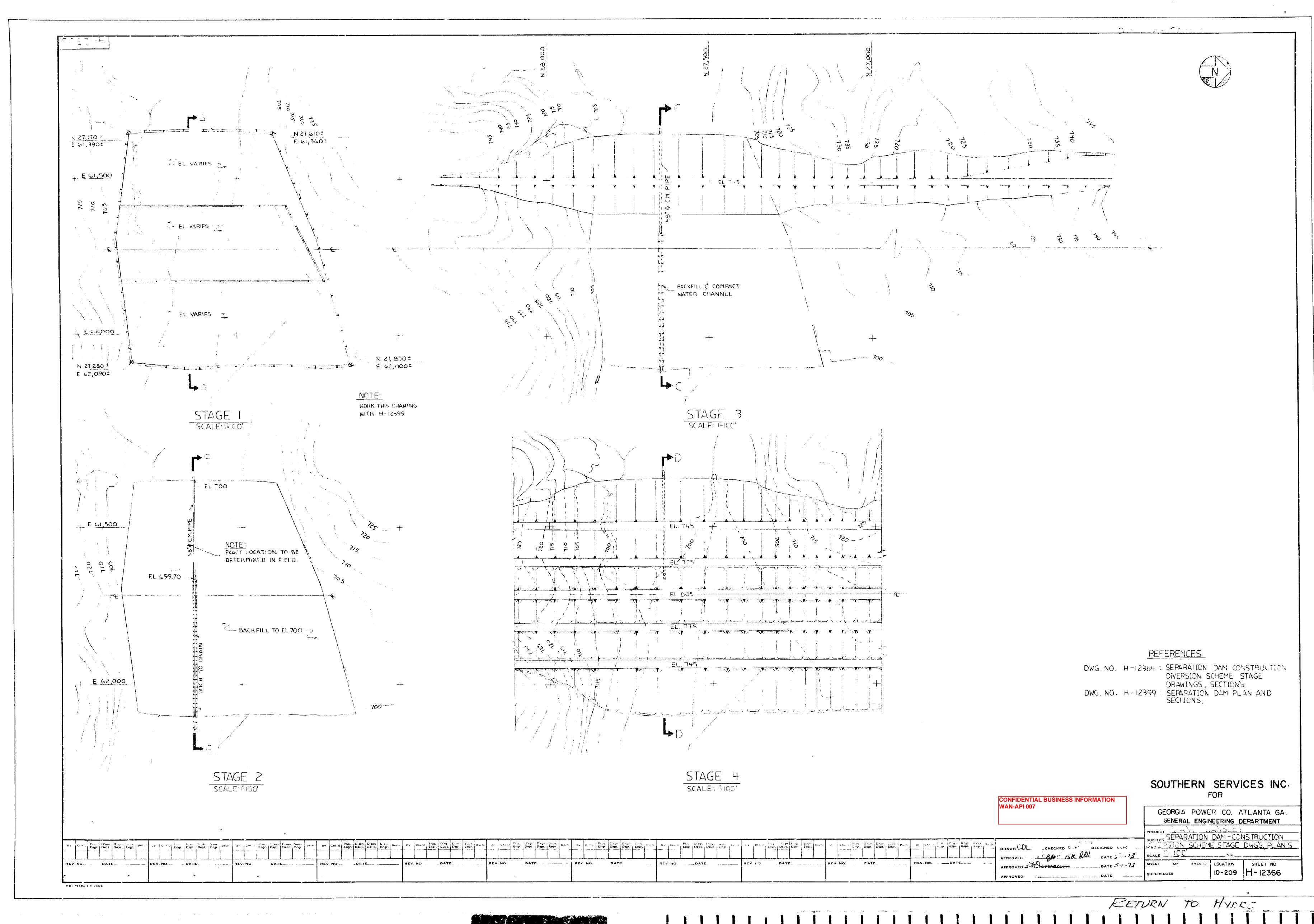


PLANT WANSLEY

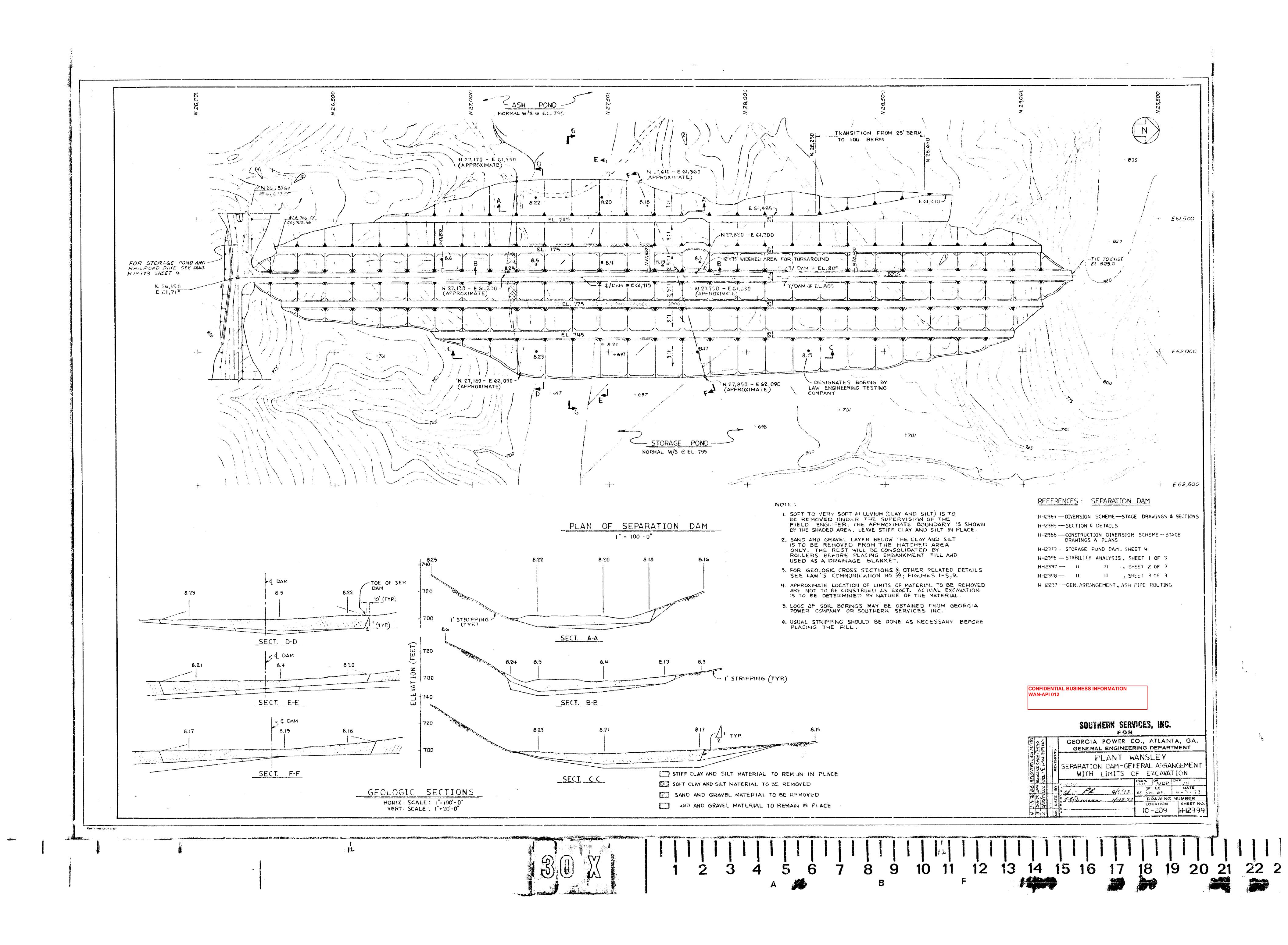
SEPAHATION DAM STA 2+00

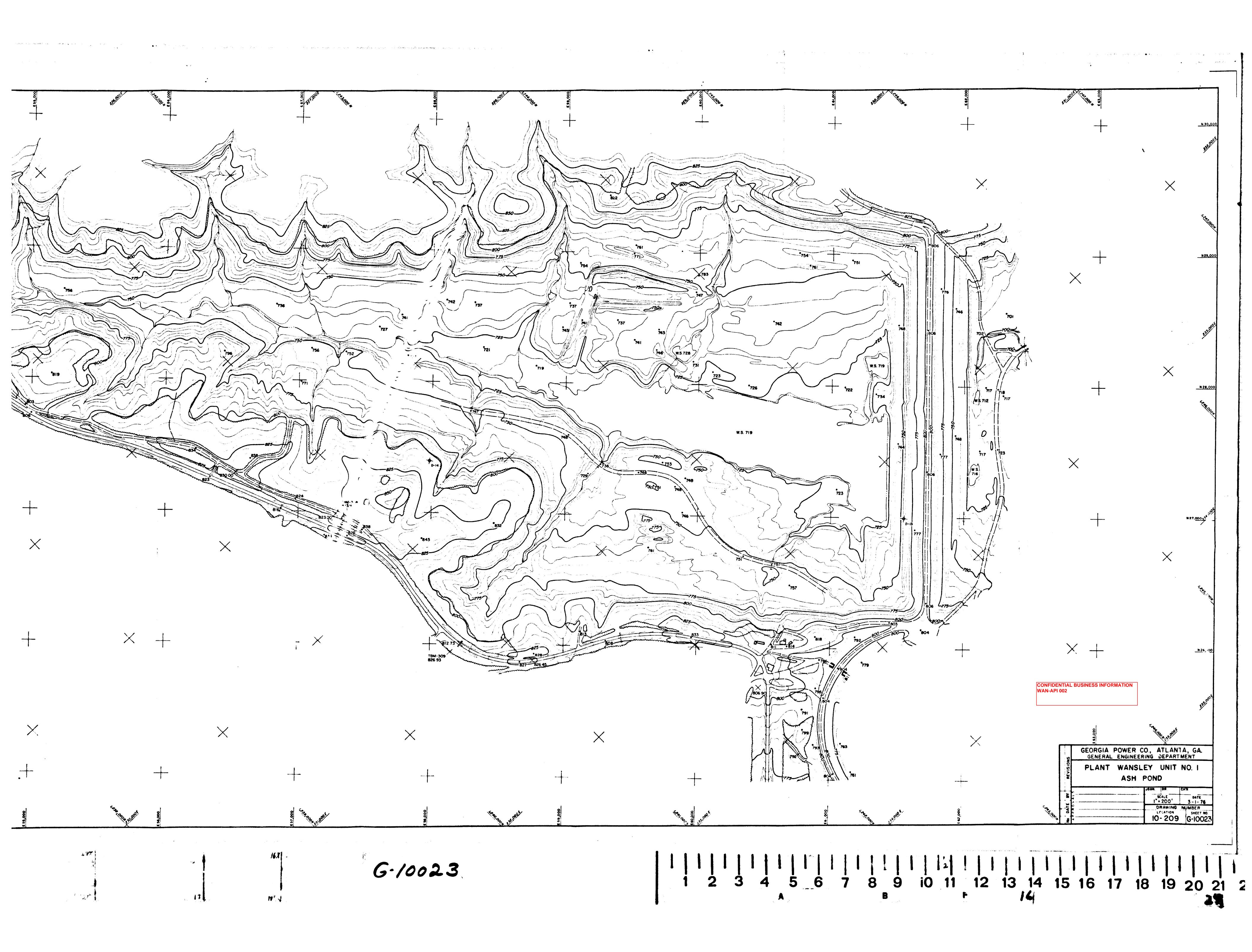
PLANT WANSLEY SEPARATION DAM STA. 12+50

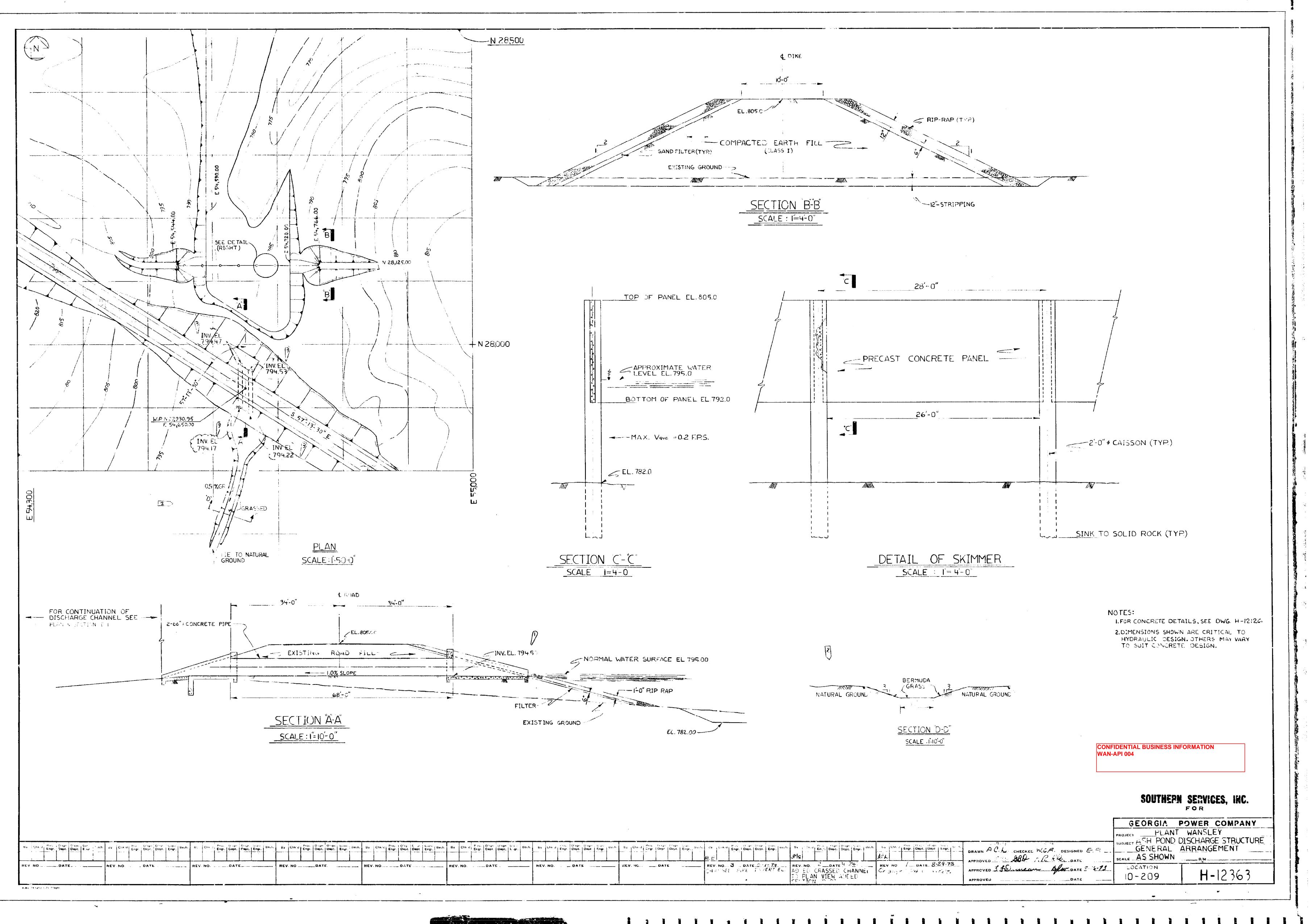
















Engineering and Construction Services Calculation

Calculation Number: TV-WN-ECS3201-001

Project/Plant:	Unit(s):	Discipline/Area:
Plant Wansley Ash Pond	Units 1-2	ES&EE
Title/Subject:		
Slope Stability Analyses of Ash Pond Separation	n Dike	
Purpose/Objective:		
Analyze slope stability of Ash Pond Separation [Dike	
System or Equipment Tag Numbers:	Originator:	
NA	Wa	yne Wang

Contents

	1		1
		Attachments	
Topic	Page	(Computer Printouts, Tech. Papers, Sketches, Correspondence)	# of Pages
Purpose of Calculation	1	Attachment A – Figure 1	1
Methodology	1	Attachment B – Boring Log	4
Criteria & Assumptions	1	Attachment C – Soil Laboratory Analyses	8
Summary of Conclusions	3	Attachment D – Historic Soil Laboratory Analyses	1
Design Inputs/References	3		
Body of Calculation (print outs)	4 - 9		
Total # of pages including cover sheet & attachments:	30		

Revision Record

Rev. No.	Description	Originator Initial / Date	Reviewer Initial / Date	Approver Initial / Date
0	Issued for Information	WW/8-6-10	GHM/8-10-10	JCP/8-10-10

Notes:

Purpose of Calculation

Plant Wansley has disposed of coal combustion by-products (ash) in one main storage impoundment since 1976. The Plant Wansley ash pond was commissioned in 1975. The separation dike (ash pond dike) was installed in 1975 between the ash pond and the downstream storage water pond. The dike was constructed to a crest elevation of El. 805 with 2.3 (H):1(V) and 3 (H):1(V) upstream and downstream slopes, intermediate berms at El. 775 and El. 745, and a maximum height of approximately 105 feet.

The purpose of this calculation is to determine the stability of the separation dike of the Ash Pond.

Methodology

The calculation was performed using the following methods and software:

GeoStudio 2007 (Version 7.16, Build 4840), Copyright 1991-2008, GEO-SLOPE International, Ltd.

Bishop, Ordinary, Janbu, and Morgenstern-Price analytical methods were run. Morgenstern-Price was reported.

Criteria and Assumptions

The slope stability models were run using the following assumptions and design criteria:

- According to the USGS earthquake acceleration probability maps for the vicinity of Plant Wansley, and "Pseudostatic Coefficient for Use in Simplified Seismic Slope Stability Evaluation" published in *Journal of Geotechnical and Geoenvironmental* Engineering, ASCE September 2009 by Jonathan Bray and Thaleia Travasarou, a seismic load of 0.15g was used in the analyses.
- The current required minimum criteria (factors of safety) were taken from US Corps of Engineers Manual EM 1110-2-1902, October 2003 and the Georgia Department of Natural Resources, Environmental Protection, Rules for Dam Safety, Rule 391-3-8-09 Standards for the Design and Evaluation of Dams.
- The soil properties of unit weight, phi angle, and cohesion were obtained from triaxial shear testing performed on UD samples of the dike fill material obtained during drilling in July 2010 and from parameters used during the 1973 stability analysis indicated on Drawings H12396 H12398. The triaxial shear testing was performed according to ASTM D 4767.
- Properties for ash were based on laboratory testing performed on undisturbed and remolded samples of ash from various plants and on previous project experience.
- The data obtained from piezometers BB and DD was used to provide phreatic data for the slope stability analysis for the separation dike.
- The cross-section of the dike was obtained using the following sources:

- 1) Original design Drawing No. H12365 Section G-G.
- 2) A boring conducted in July 2010.

The following soil properties were used in the analyses:

	Dry Unit Weight (pcf)	Moist Unit Weight	Effective Stress Parameters Total Stress Parameters			
	Dry Omt Weight (pci)	(pci)	Internal Friction Angle Cohesion (psf)		Internal Friction Angle	Cohesion (psf)
Embankment Fill	102	123	32	140	29	400
Foundation Soil		112	37	0	24	80
Foundation (Gravel Filter)		130	40	0	40	0
Sluiced Ash		80	10	0	10	0
Rock		150	40	3000	40	3000

As shown on drawing H-12398, the 1973 stability analysis used unconsolidated - undrained (UU) strength parameters of c = 700 psf and phi = 12° for the foundation soils under a steady state loading condition. The Law Engineering Testing Company Report dated April 3, 1972, reported results for a consolidated – undrained (CU) triaxial shear test performed on a foundation sample from Boring 8.12, sample depth of 14 ft to 16 ft, or approximate Elev. 696. The total and effective parameters from this test were used in this 2010 analysis.

The following hydraulic information was used in the analyses:

Elevation (ft)	Min. Pool	Normal Pool	Max. Pool
Ash Pond		795	802.6
Storage Pond	725	780	

Based on Georgia Power's (GP) Land Department Drawing M-187-6, Plant Wansley Ash Pond – August 2005 Survey, the top elevation of the ash in the impoundment is approximately El. 765. The normal pool is El. 795. The maximum surcharge pool is El. 802.6 which corresponds to the crest elevation of the emergency spillway. According to the Plant Wansley Ash Pond Storm Water Analysis prepared by SCG Hydro Services, the storage capacity of the ash pond from, conservatively, El. 799 to El. 802.6 is 1044 ac-ft, 2.22 times the storage necessary for the 100 year, 24 hour storm event. For our analysis, we assumed that rapid drawdown would occur from El 802.6 of the maximum surcharge pool to El. 795, the normal pool elevation.

The normal (and maximum) pool elevation of the storage water pond is El. 780. This maximum level constraint has been established to minimize the occurrence of excessive seepage conditions along the downstream slope/toe of the dike. Based on the 1973 original slope analyses shown on

GP Drawings H12396 –H12398, the minimum pool is El. 725. We assume conservatively that rapid drawdown occurs from El. 780 to El. 725.

Summary of Conclusions

The following table lists the factors of safety for various slope stability failure conditions. All conditions are steady state except where noted. Construction cases were not considered. Based on the results of these analyses, the dike is stable. The minimum factor of safety for each load case was taken from the USACOE EM 1110-2-1902 (2003) or the Georgia Department of Natural Resources, Environmental Protection, Rules for Dam Safety, Rule 391-3-8-09 Standards for the Design and Evaluation of Dams.

Failure Conditions	Computed Factor of Safety	Required Minimum Factor of Safety ¹
Downstream Steady State	1.9	1.5
Downstream Seismic	1.2	1.1
Downstream Maximum Surcharge Pool (Ash Pond)	1.7	1.4
Upstream Rapid Drawdown (Ash Pond)	1.9	1.3
Downstream Rapid Drawdown (Storage Pond)	1.4	1.3

¹ US Corps of Engineers Manual EM 1110-2-1902, October 2003

The analyses show that in all cases the separation dike is stable. Safety factors for all cases were acceptable and exceeded the minimum safety factors required.

Design Inputs/References

USGS Earthquake Hazards website, http://www.usgs.gov/hazards/earthquakes/.

NOAA website, http://www.srh.noaa.gov/ffc/html/rva.php.

Georgia Department of Natural Resources, Environmental Protection, Rules for Dam Safety.

GPC Land Department Drawing M-187-6 Plant Wansley Ash Pond – August 2005 Survey

GPC Drawing H10027 Project Location Map

GPC Drawing H12363 - Plant Wansley Ash Pond Discharge Structure General Arrangement

GPC Drawing H12364 - Plant Wansley Separation Dike Construction

GPC Drawing H12365 - Plant Wansley Separation Dike section and Details

GPC Drawing H12366 - Plant Wansley Separation Dike Construction

GPC Drawing H12396 - Plant Wansley Separation Dike Stability Analysis – Sheet 1 of 3

GPC Drawing H12397 - Plant Wansley Separation Dike Stability Analysis – Sheet 2 of 3

GPC Drawing H12398 - Plant Wansley Separation Dike Stability Analysis – Sheet 3 of 3

GPC Drawing H12399 - Plant Wansley Separation Dike General Arrangement

SCG Hydro Services - Dam Safety Surveillance, 4th Quarter 2009 Report, Plant Wansley

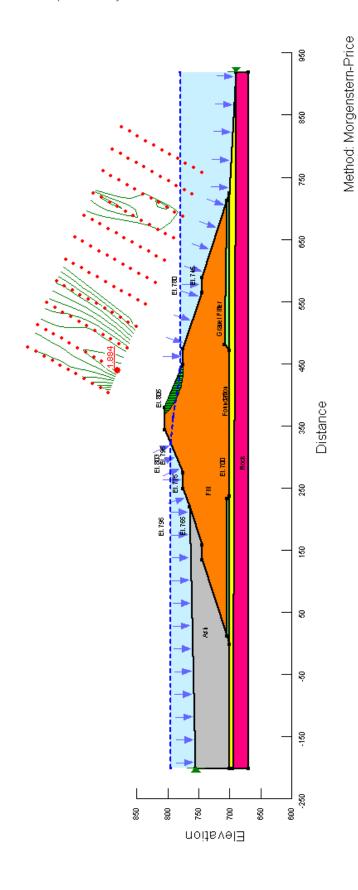
Evaluate Storm Water Capacity of Wansley Ash Pond by SCG Hydro Services – August 2010

Body of Calculation

Calculation consists of Slope-W modeling attached.

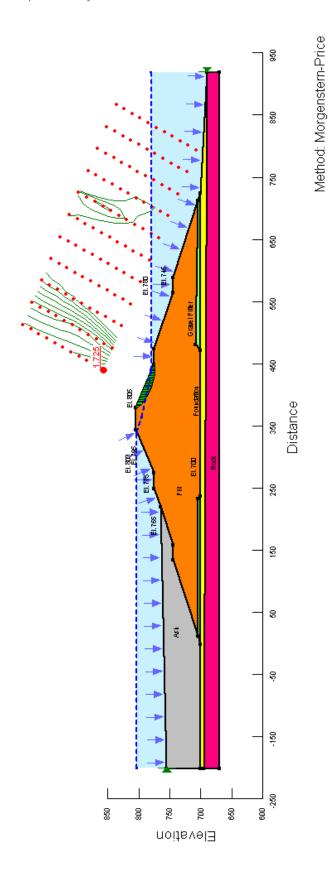
Plant Wansley Ash Pond Separation Dam Stability Analysis

Downstream Steady State



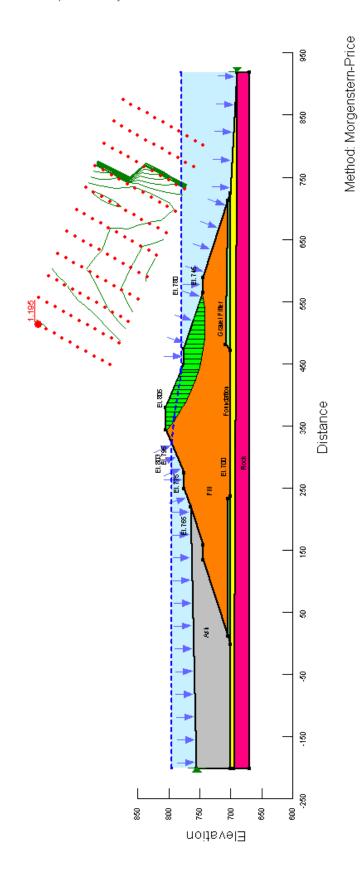
Plant Wansley Ash Pond Separation Dam Stability Analysis

Downstream Max. Surcharge Pool (Ash Pond)



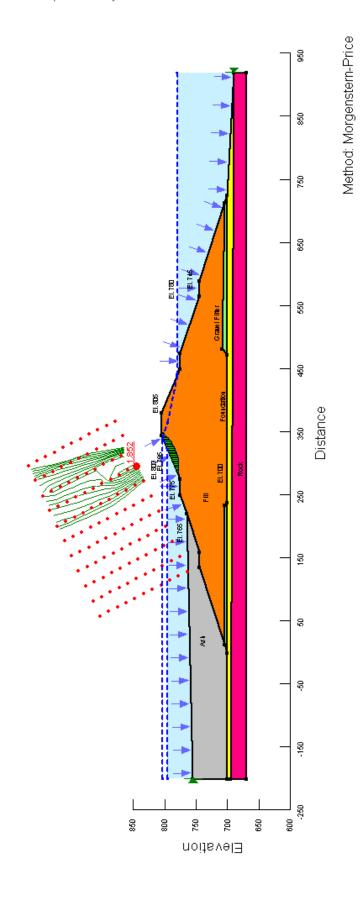
Plant Wansley Ash Pond Separation Dam Stability Analysis

Downstream Seismic



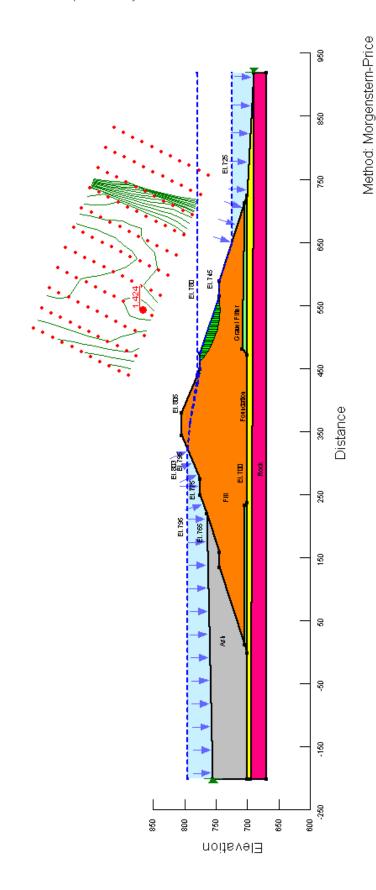
Plant Wansley Ash Pond Separation Dam Stability Analysis

Rapid Drawdown (Ash Pond)



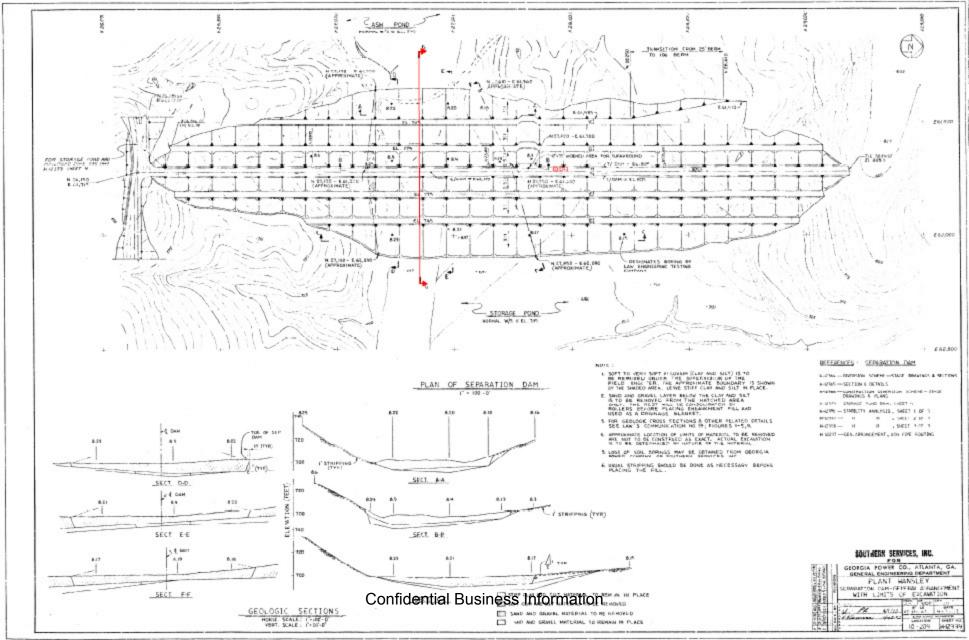
Plant Wansley Ash Pond Separation Dam Stability Analysis

Rapid Drawdown (Storage Pond)



Attachment A

Figure - Boring Location



Attachment B

Soil Log

sou:	THERN		NG L	.OG			Hole No.	D-1	
Energy	COMP to Serve You	r World" GEOLOGICA					Sheet 1 of		
SITE _		Plant Wansley Seperation Dyke			HOLE DEPTH	106	SURF.ELEV	80	06
		Plant Wansley						716.41	
ANGLE		0 BEARING 0	CONTR	RACTOR	Ranger	D		N/A	
DRILLIN	NG METHO	D H.S.A. NO. SAMPLES		21	NO. U	.D. SAMPL	ES	4	
CASING	G SIZE	N/A LENGTH N/A	_ co	RE SIZE	N/A	TOTAL '		N/A	
WATER	R TABLE DE	PTH 26' ELEV. N/A TIME OF THE PORTION OF THE PROPERTY N/A	ЛЕ AFTE	ER COMP.	17 hours	DAT	E TAKEN 7/3	3/2010	
TYPE G	GROUT	Portland QUANTITY N/A	N	ııx1	I:1 DRII	LING STA	RT DATE7/	7/2010	
DRILLE	:R	Justen Crowe RECORDER Korey Young APPROV	/ED _	Korey Yo	oung DRII	LING CON	MP. DATE7/	7/2010	
Depth	Elev.	Material Description, Classification and Remarks	Sample No.		ndard Penetration Test Blows	N	Comments	% Rec	RQD
0									
1							.O.B. water level ras 62'		
2						l l	as 02		
3									
4									
5		red brown micacous SILT (ML)	1	3.5-5	11-12-14	26			
6									
7									
8									
9									
10		red brown micacous SILT (ML)	2	8.5-10	7-6-8	14			
		Ted brown micacous ofer (we)		0.5 10	7-00	14			
11									
12									
13									
14									
15		red brown micacous SILT (ML)	3	13.5-15	8-10-12	22			
16									
17									
						υ	D 16-18.5		
18		-							
19									
20		red brown micacous SILT (ML)	4	18.5-20	8-13-17	30			
21									
22						U	D 21-23.5		
23									
. .									

SOU'	THERN COMF	DRIL	LING L	.OG			Hole No.	D-1	
Energy	to Serve Yo	ur World" GEOLOGI	CAL SE	RVICES			Sheet 2 of	4	
SITE _		Plant Wansley Seperation Dyke			TOTAL DEPTH	106	SURF.ELEV	·. <u>80</u>	06
Depth	Elev.	Material Description, Classification and Remarks	Sample No.	Stan From To	dard Penetration Test Blows	N	Comments	% Rec	RQD
25		red brown micacous SILT (ML)	5	23.5-25	7-12-10	22			
26									
27									
28									
29									
30		red brown micacous SILT (ML)	6	28.5-30	8-10-12	22			
31									
32									
33									
34									
35		red brown micacous SILT (ML)	7	33.5-35	6-8-9	17			
36									
37									
38									
39									
40		red brown micacous SILT (ML)	8	38.5-40	6-9-13	22			
41									
42									
43									
44									
45		red brown micacous SILT (ML)	9	43.5-45	7-9-12	21			
46									
47						sl	ID 46-47.5 hort push due to		
48						h	ard material		
49 50		red brown micacous SILT (ML	10	48.5-50	7-9-10	19			
51		TO STATE OF THE ST	10	10.0 00	7 3 10	13			
52						U	D 51-53		
53						sl	hort push due tc ard materia		
54									
55		red brown micacous SILT (ML	11	53.5-55	5-7-12	19			
	1	<u> </u>	I	1	1	1 1		Ī	ĺ

sou [.]	THERN	DRILI	LING L	.OG			Hole No.	D-1	
Energy	to Serve Yo	ur World* GEOLOGI	CAL SE	RVICES			Sheet 3 of	4	
SITE		Plant Wansley Seperation Dyke			TOTAL DEPTH	106	SURF.ELEV	·80	06
Depth	Elev.	Material Description, Classification and Remarks	Sample No.	Stan From To	dard Penetration Test Blows	N	Comments	% Rec	RQD
57								7, 7, 7, 7	
58									
59									
60		multi colored sandy SILT (ML)	12	58.5-60	5-10-16	26			
61									
62									
63									
64									
65		brown gray micacous SILT (ML)	13	63.5-65	7-16-14	30			
66		5-5-11 g.u,505-5-12 (2)		00.0 00					
67									
68									
69									
70		brown gray micacous SILT (ML)	14	68.5-70	7-14-16	30			
71									
72									
73									
74									
75		brown gray micacous SILT (ML)	15	73.5-75	7-12-15	27			
76		,							
77									
78									
79									
80		tan fine sandy SILT (ML)	16	78.5-80	6-12-17	29			
81									
82									
83									
84									
85		brown micacous SILT (ML	17	83.5-85	7-16-13	29			
86									
87									
	1							1	

SOU	THERN COMP	DRIL	LLING L	.OG			Hole No.	D-1	
Energy	to Serve You	r World GEOLOG	SICAL SE	RVICES			Sheet 4 of	4	
SITE		Plant Wansley Seperation Dyke			TOTAL DEPTH	106	SURF.ELEV	80)6
Depth	Elev.	Material Description, Classification and Remarks	Sample No.	Stand From To	dard Penetration Test Blows	N	Comments	% Rec	RQD
89		material Scoriptori, Glassinoscion una remano					Commonte	76 NGC	
90		pink brown micacous SILT (ML)	18	88.5-90	13-20-22	42			
91									
92									
93									
94									
95		tan brown micacous SILT (ML)	19	93.5-95	9-13-19	32			
96 97									
98									
99									
100		tan brown silty CLAY (CL)	20	98.5-100	9-12-15	27			
101									
102									
103									
104									
105		tan orange clayey fine SAND (SC)	21	103-5-105	12-18-23	41			
106		Auger Refusal @ 106'							
107									
109									
110									
111									
112									
113									
114									
115									
116									
117									
118									
113	<u> </u>								

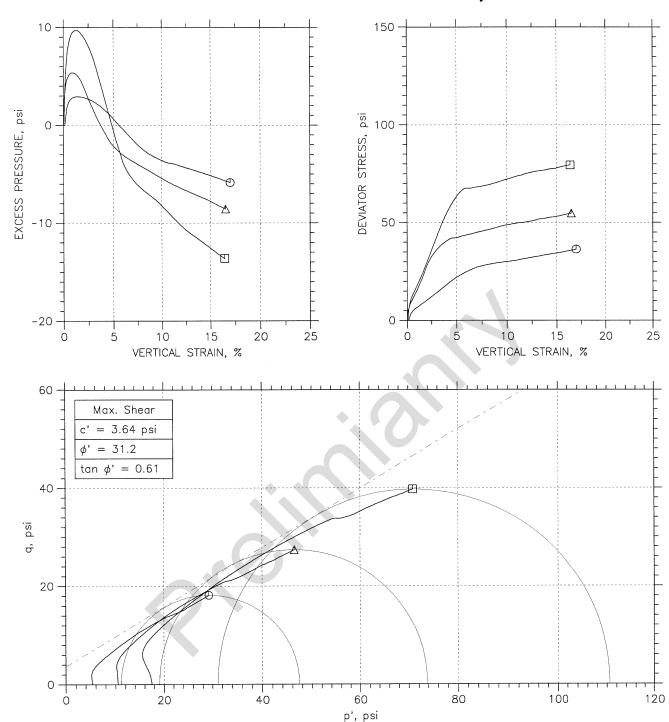
Attachment C

Soil Laboratory Analyses by MACTEC Engineering and Consulting.

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767 Max. Shear = 3.64 psi $\phi' = 31.2$ $\tan \phi' = 0.61$ 40 psi ò 20 20 40 60 80 100 120 p', psi 0 Symbol Δ UD UD UD Sample No. 140 Test No. 10353.1 10353.2 10353.3 Depth 21-23.3 ft21-23.3 ft21-23.3 f Diameter, in 2.865 2.872 2.873 120 Height, in 5.57 5.973 5.572 Water Content, % 19.2 19.7 20.4 100 104.7 101.1 101.1 Dry Density, pcf psi. Saturation, % 87.8 82.2 84.8 DEVIATOR STRESS, 0.58 0.636 0.636 Void Ratio 80 23.5 23.1 Water Content, % 21.8 Shear 104.8 102. 102.6 Dry Density, pcf 60 100.0 100.0 100.0 Saturation*, % Void Ratio 0.578 0.622 0.613 Back Press., psi 120. 110. 110. 40 10.5 17.49 Ver. Eff. Cons. Stress, psi 5.252 18.14 27.35 39.71 Shear Strength, psi 20 Strain at Failure, % 17 16.5 16.4 Strain Rate, %/min 0.05 0.05 0.05 0.96 0.93 0.95 B-Value 10 15 20 2.65 Estimated Specific Gravity 2.65 2.65 VERTICAL STRAIN, % Liquid Limit ____ ___ Plastic Limit Project: Plant Wansley Ash Pond Location: D-1 Project No.: 6152100244 **MACTEC** Boring No.: D-1 Sample Type: Undisturbed Description: Brown Silty Sand Remarks: ASTM D4767-04

Phase calculations be confidential Business that formation

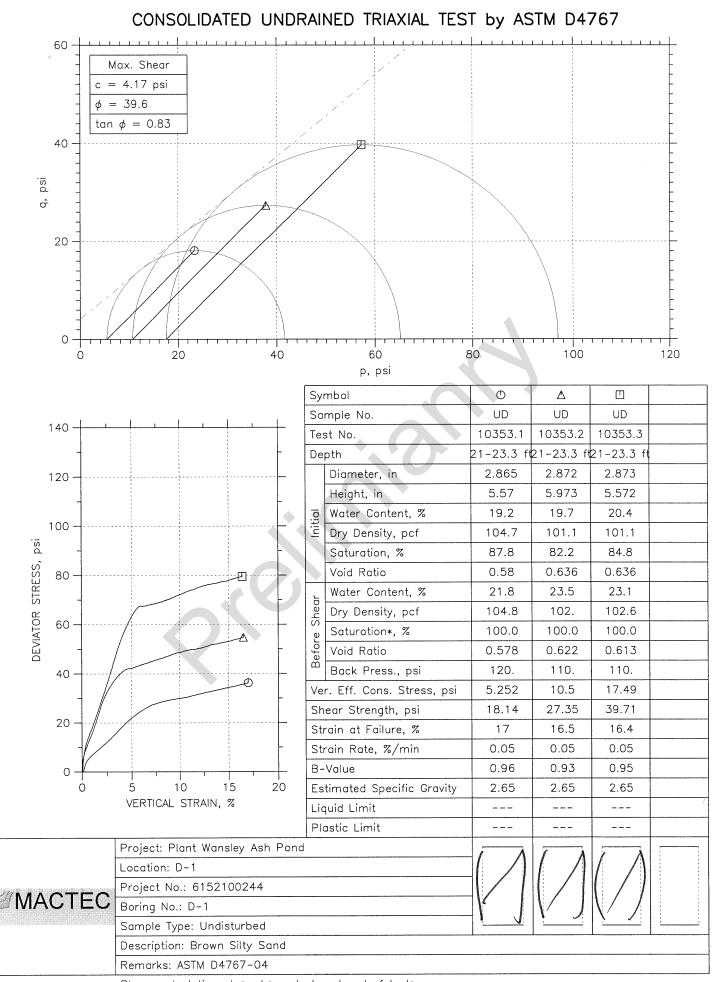
CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
0	UD	10353.1	21-23.3 ft	JW	7/26/10			10353.1_2581.dat
Δ	UD	10353.2	21-23.3 ft	JW	7/26/10			10353.2a_2582.dat
	UD	10353.3	21-23.3 ft	JW	7/26/10			10353.3a_2583.dat

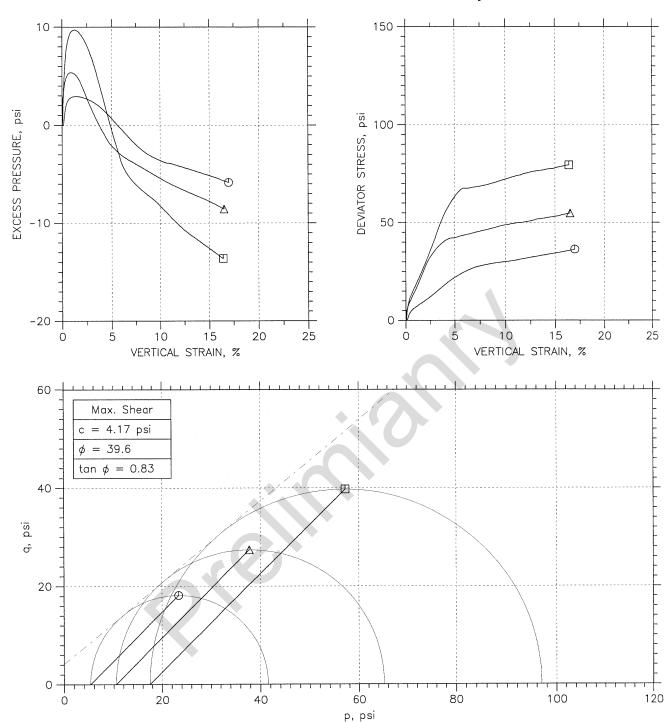
MACTEC	0-T-0	- A	/ 🕳
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Project: Plant Wansley Ash Pond	Location: D-1	Project No.: 6152100244
Boring No.: D-1	Sample Type: Undisturbed	
Description: Brown Silty Sand		
Remarks: ASTM D4767-04		



Phase calculations beornfidential Business information

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
0	UD	10353.1	21-23.3 ft	JW	7/26/10			10353.1_2581.dat
Δ	UD	10353.2	21-23.3 ft	JW	7/26/10			10353.2a_2582.dat
	UD	10353.3	21-23.3 ft	JW	7/26/10			10353.3a_2583.dat

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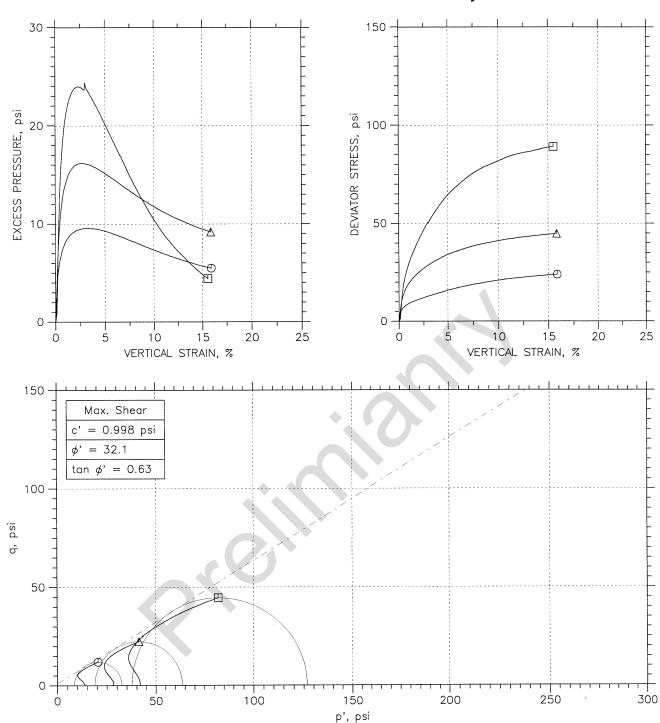
Project: Plant Wansley Ash Pond	Location: D-1	Project No.: 6152100244
Boring No.: D-1	Sample Type: Undisturbed	
Description: Brown Silty Sand		
Remarks: ASTM D4767-04		

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767 Max. Shear = 0.998 psi $\phi' = 32.1$ $tan \phi' = 0.63$ 100 psi. ô 50 50 100 150 200 250 300 p', psi Symbol Δ UD UD UD Sample No. 140 10352.1 10352.2 10352.3 Test No. 51-53 ft 51-53 ft 51-53 ft Depth 2.843 2.872 2.872 Diameter, in 120 Height, in 5.56 5.955 5.562 Water Content, % 23.3 22.5 19.3 100 Dry Density, pcf 97.62 99.59 107.4 psi. 88.9 90.2 94.8 Saturation, % DEVIATOR STRESS, Void Ratio 0.695 0.661 0.54 80 23.9 25.5 20.1 Water Content, % Dry Density, pcf 98.7 101.3 108. 60 Saturation*, % 100.0 100.0 100.0 0.532 Void Ratio 0.676 0.633 Back Press., psi 120. 110. 100. 40 42. Ver. Eff. Cons. Stress, psi 14. 27.99 22.38 44.54 Shear Strength, psi 11.93 20 15.5 Strain at Failure, % 15.9 15.8 Strain Rate, %/min 0.05 0.05 0.05 B-Value 0.96 0.92 0.96 10 15 20 2.65 Estimated Specific Gravity 2.65 2.65 VERTICAL STRAIN, % Liquid Limit Plastic Limit ___ Project: Plant Wansley Ash Pond Location: D-1 Project No.: 6152100244 **MACTEC** Boring No.: D-1 Sample Type: Undisturbed Description: Brown Silty Sand Remarks: ASTM D4767-04

Phase calculations be confidential Business that formation

* Saturation is set to 100% for phase calculations.

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
0	UD	10352.1	51-53 ft	JW	7/23/10			10352.1a_2581.dat
Δ	UD	10352.2	51-53 ft	JW	7/23/10			10352.2a_2582.dat
	UD	10352.3	51-53 ft	JW	7/23/10			10352.3a_2583.dat

N				
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11/	17.			
	••		-	-

	Project: Plant Wansley Ash Pond	Location: D-1	Project No.: 6152100244
,	Boring No.: D-1	Sample Type: Undisturbed	
	Description: Brown Silty Sand		

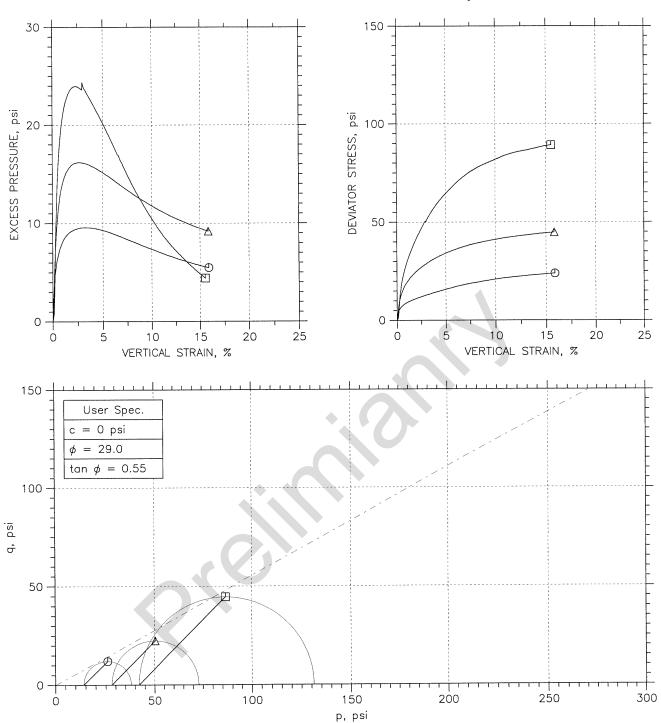
Benerica ASTM D4767 04

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767 User Spec. = 0 psi $\phi = 29.0$ $tan \phi = 0.55$ 100 psi ó 50 300 50 100 150 200 250 p, psi Symbol Δ UD UD UD Sample No. 140 10352.1 10352.2 10352.3 Test No. 51-53 ft 51-53 ft 51-53 ft Depth 2.872 2.872 Diameter, in 2.843 120 5.562 Height, in 5.56 5.955 23.3 22.5 19.3 Water Content, % 100 Dry Density, pcf 97.62 99.59 107.4 ps: 94.8 Saturation, % 88.9 90.2 DEVIATOR STRESS, Void Ratio 0.695 0.661 0.54 80 Water Content, % 25.5 23.9 20.1 Shear Dry Density, pcf 98.7 101.3 108. 60 Saturation*, % 100.0 100.0 100.0 0.532 Void Ratio 0.676 0.633 Back Press., psi 120. 110. 100. 40 42. Ver. Eff. Cons. Stress, psi 14. 27.99 11.93 22.38 44.54 Shear Strength, psi 20 15.5 Strain at Failure, % 15.9 15.8 0.05 Strain Rate, %/min 0.05 0.05 B-Value 0.96 0.92 0.96 0 10 20 2.65 2.65 2.65 Estimated Specific Gravity VERTICAL STRAIN, % Liquid Limit Plastic Limit Project: Plant Wansley Ash Pond Location: D-1 Project No.: 6152100244 **MACTEC** Boring No.: D-1 Sample Type: Undisturbed Description: Brown Silty Sand Remarks: ASTM D4767-04

Phase calculations Conflictential Business information

* Saturation is set to 100% for phase calculations.

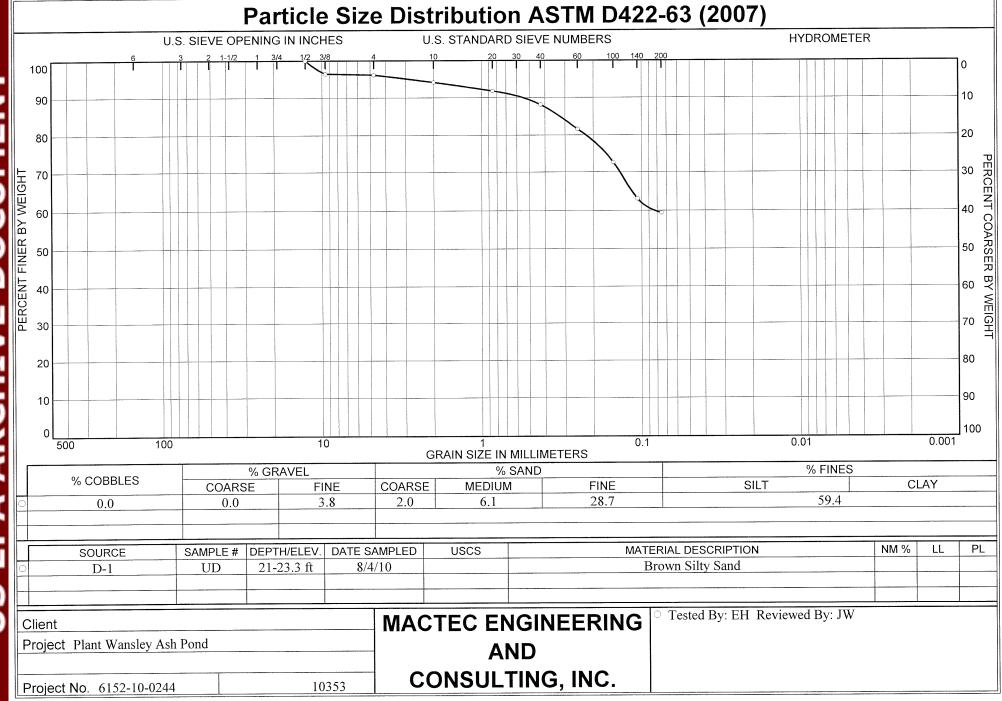
CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767

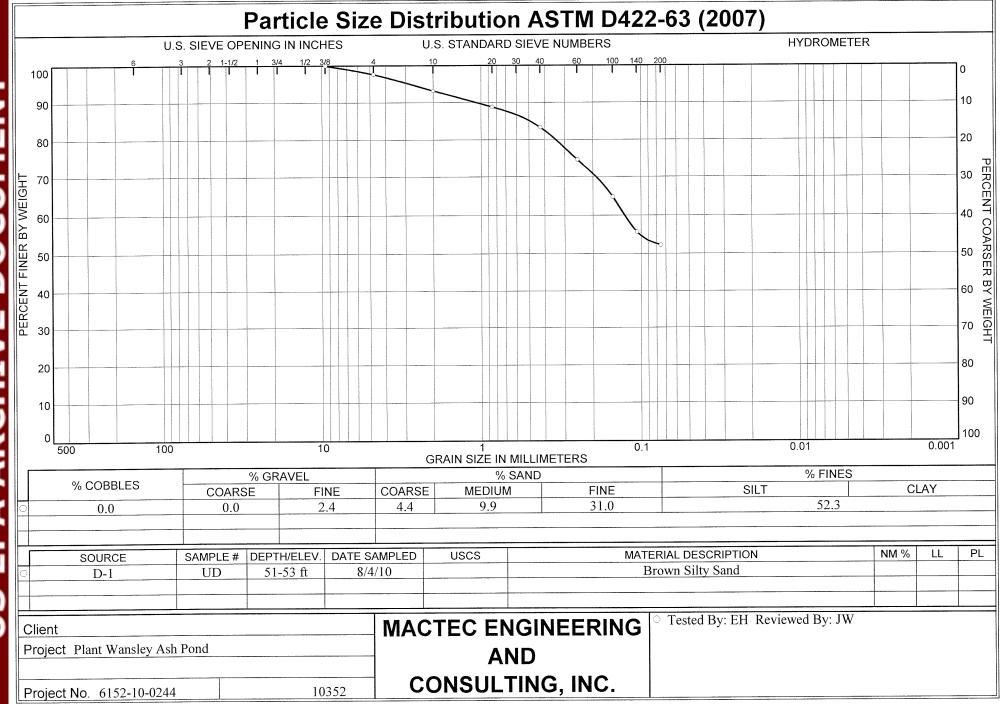


	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
0	UD	10352.1	51-53 ft	JW	7/23/10			10352.1a_2581.dat
Δ	UD	10352.2	51-53 ft	JW	7/23/10			10352.2a_2582.dat
	UD	10352.3	51-53 ft	JW	7/23/10			10352.3a_2583.dat

///	Project: Plant Wansley Ash Pond	Location: D-1	Project No.: 6152100244
MACTEC	Boring No.: D-1	Sample Type: Undisturbed	
	Description: Brown Silty Sand		
	Remarks: ASTM D4767-04		





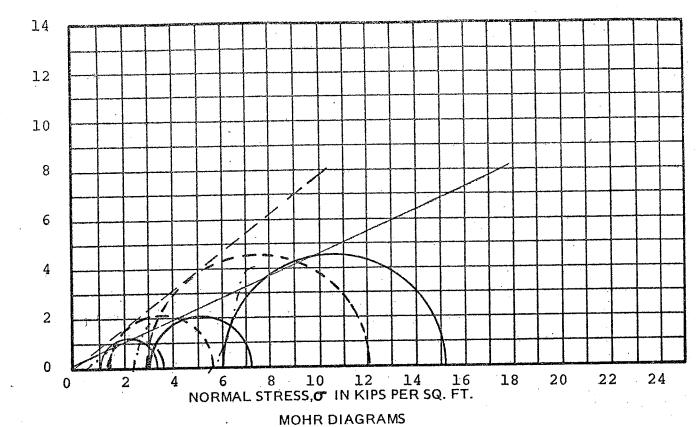


Attachment D

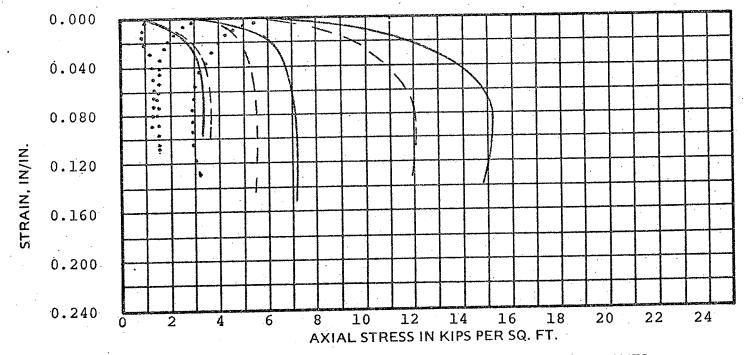
Historic Soil Laboratory Analyses by Law Engineering Testing Company



SHEAR STRESS IN KIPS PER SQ. FT.



EXCESS PORE PRESSURE IN KIPS PER SQ. FT.



STRESS-STRAIN AND PORE PRESSURE-STRAIN CURVES

effective cohesion, c 0 37.5 Total cohesion, c 0.08 KSF		SATURATED, CONS UNDRAINED TRIAX TEST WITH PORE P MEASUREMENTS	XIAL SHEAR
	NGLE, Ф <u>24</u>	SAMPLE NO. UD	_BORING NO B-8.12
With the state of	Total oi	DEPTH 14-16	
• 4 0 0 0 0 4 4 4 .	Effective σ_1 Confidential Bus	siness Information ENGINEERING TE	STING COMPANY

Strass Path



					Calculation Nu		
Project					SH-WN10911-0	1	
li Toject	Wansley				Discipline Hydro Service	25	
Objective				-	Number		
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	Evaluate Stormwater Capacity of	Ash Pond					
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Criteria		1					
Major Equ	ation Sources/	<u> </u>					
Derivation	Methods	1					
Assumption	ons	1					
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	f Revisions						
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NOTES:							
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Form No. 9-363F

US EPA ARCHIVE DOCUMENT

See Back for Instructions



Energy to Serve Your World

Project Plant Wansley	Prepared By Fred L. Cox, Jr.	Date 8/1/2010
Subject/Title Evaluate Stormwater capacity of Ash Pond	Reviewed By Courtenay O'Mara	Date 8/4/2010
·	Calculation Number SH-WN10911-01	Sheet 1 of 10

1.0 Purpose of Calculation:

Determine the storm water handling capacity of the Wansley Ash Pond

2.0 **Summary of Conclusions:**

The Wansley Ash Pond can handle a 24 hour rainfall runoff of: 1) 16.9 inches of rainfall runoff, which is 2.13 times the 100 year storm, at the level of the emergency spillway crest, and 2) 23.5 inches of rain, which is 2.96 times the 100 year storm, at a level one foot below the crest of the dike. The probability of occurrence of these storms is exceedingly low. Based on these data and Georgia Power's tolerance for risk of this type of storm occurring, it is concluded that the capacity of the ash pond is far more than adequate.

3.0 Criteria:

See Section 7.0

4.0 References:

ARCHIVE DOCUMENT

- 1. Georgia EPD Safe Dam Program Rules for Dam Safety
- 2. Dwg. 10-209-B1008, Plant Wansley Ash Pond Area-Volume Curve
- 3. Dwg. 10-209-E1C11102, Short Term Gypsum Disposal, General Arrangement and Site Plan
- 4. Georgia Stormwater Management Manual, Rainfall Table for Peachtree City
- 5. 4th Quarter 2009 report on Dam Safety Surveillance for Plant Wansley
- 6. 2002 NPDES Co-Treatment Calculations

5.0 Assumptions:

See Section 7.0

6.0 Major Equation Sources/Derivation of Methods:

See Section 7.0

7.0 Body of Calculations

Ash Pond Data:

The Plant Wansley Ash Pond is not a Category I structure according to the Georgia EPD Safe Dam Program Rules for Dam Safety. Because of this; it is not required to pass the PMF, or some fraction of the PMF. A lesser storm event is acceptable.



Energy to Serve Your World

Project Plant Wansley	Prepared By Fred L. Cox, Jr.	Date 8/1/2010
Subject/Title Evaluate Stormwater capacity of Ash Pond	Reviewed By Courtenay O'Mara	Date 8/4/2010
	Calculation Number SH-WN10911-01	Sheet 2 of 10

Elevation – Volume – Area Data

(Drawing. 10-209-B10008) (See Figure 1, page 5)

Elevation	Volume	Area	
	Ac-Ft	Ac	
790	12300	312	
795	14000	333	
800	15700	354	
805	17400	375	

Top of Dike: 805

Emergency Spillway Crest: 802.6

Pond Operating Level: See attached of 2000-2009 levels from 2009 4th Qtr Inspection Report. The Ash Pond level generally fluctuates between 795 and 799. (See Figure 2, page 6)

Drainage Area: 711 Acres (2002 Co-Treatment calculations)

Rainfall Data: From the Georgia Stormwater Management Manual rainfall tables, the closest city is Peachtree City (See Figure 3, page 7)

Return Period	24 Hour Intensity	24 Hour Rainfall
Yrs	Inches/hour	Inches
2	.17	4.08
5	.21	5.04
10	.24	5.76
25.	.28	6.72
50	.30	7.20
100	.33	7.92



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Project Plant Wansley	Prepared By Fred L. Cox, Jr.	Date 8/1/2010
Subject/Title Evaluate Stormwater capacity of Ash Pond	Reviewed By Courtenay O'Mara	Date 8/4/2010
	Calculation Number SH-WN10911-01	Sheet 3 of 10

Gypsum Stacking Area in Ash Pond:

In 2007 a 62 acre gypsum stacking area was established in the Ash Pond. The gypsum is stacked above the pond surface. The volume used by this stack must be deducted from the available storage volume in the pond. See Figure 4, page 8, for a drawing of the stack and Figure 5, page 9, for an aerial photo of the ash pond and gypsum stack.

Analysis

Determine how man inches of runoff can be stored up to the crest of the emergency spillway.

Assume a conservatively high starting ash pond level of 799 (see Figure 2, page 6, for a plot of ash pond levels from 2000 – 2009).

Original volume from 799 to 802.6 (crest of emergency spillway) = 1224 ac-ft

Adjustment for gypsum stack = 1224 - (3.6 ft x 62 acres) = 1000.8 ac-ft

Assuming 100 % runoff determine how many inches of rain can be stored

1000.8 ac-ft of storage / 711 ac of drainage = 1.408 ft of rain runoff storage available = 16.9 inches of rain storage available

16.9 inches of rain is 2.13 times the 100 year 24 hour rain (7.92 inches)

Determine how man inches of runoff can be stored up one foot of the top of the dike

Original volume from 799 to 804 (one foot below top dike) = 1700 ac-ft

Adjustment for gypsum stack = 1700 - (5 ft x 62 ac) = 1390 ac-ft

Assuming 100 % runoff & neglecting emergency spillway discharge

1390 ac-ft of storage / 711 ac of drainage = 1.955 ft of rain runoff storage available = 23.5 inches of rain storage available

23.5 inches of rain is 2.96 times the 100 year 24 hour rain (7.92 inches)

This analysis conservatively neglects spillway discharge during the storm



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Project Plant Wansley	Prepared By Fred L. Cox, Jr.	Date 8/1/2010
Subject/Title Evaluate Stormwater capacity of Ash Pond	Reviewed By Courtenay O'Mara	Date 8/4/2010
	Calculation Number SH-WN10911-01	Sheet 4 of 10

Summary

The Wansley Ash Pond can handle a 24 hour rainfall runoff of: 1) 16.9 inches of rainfall runoff, which is 2.13 times the 100 year storm, at the level of the emergency spillway crest, and 2) 23.5 inches of rain, which is 2.96 times the 100 year storm, at a level one foot below the crest of the dike.

Figure 6 (see page 10) is a semilog plot of the 2 to 100 year 24 hour rainfall with a line fit extended out to 200 years. It is obvious from this plot that the 16.9 inch runoff capacity up to the crest of the emergency spillway, and the 23.5 inch runoff capacity up to one foot below the crest of the dike, have very low probabilities of occurrence (probability of occurrence in a given year = the inverse of the return period).

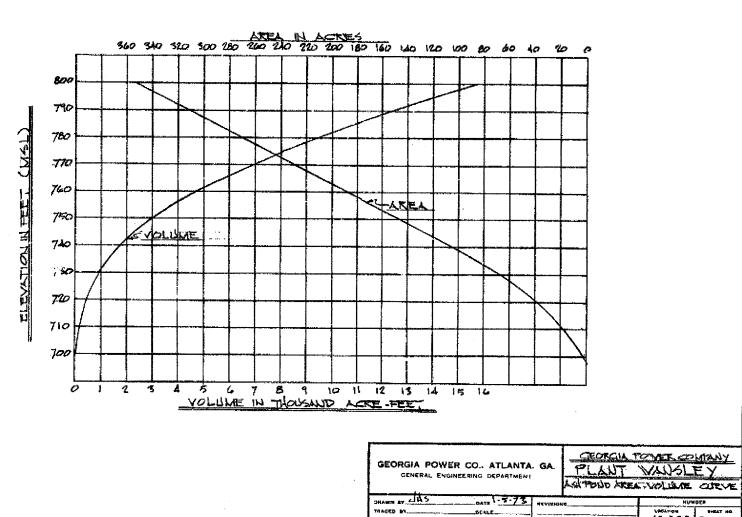
The probability of occurrence of these storms is exceedingly low. Based on these data and Georgia Power's tolerance for risk of this type of storm occurring, it is concluded that the capacity of the ash pond is far more than adequate.



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Project Plant Wansley	Prepared By Fred L. Cox, Jr.	Date 8/1/2010
Subject/Title Evaluate Stormwater capacity of Ash Pond	Reviewed By Courtenay O'Mara	Date 8/4/2010
	Calculation Number SH-WN10911-01	Sheet 5 of 10

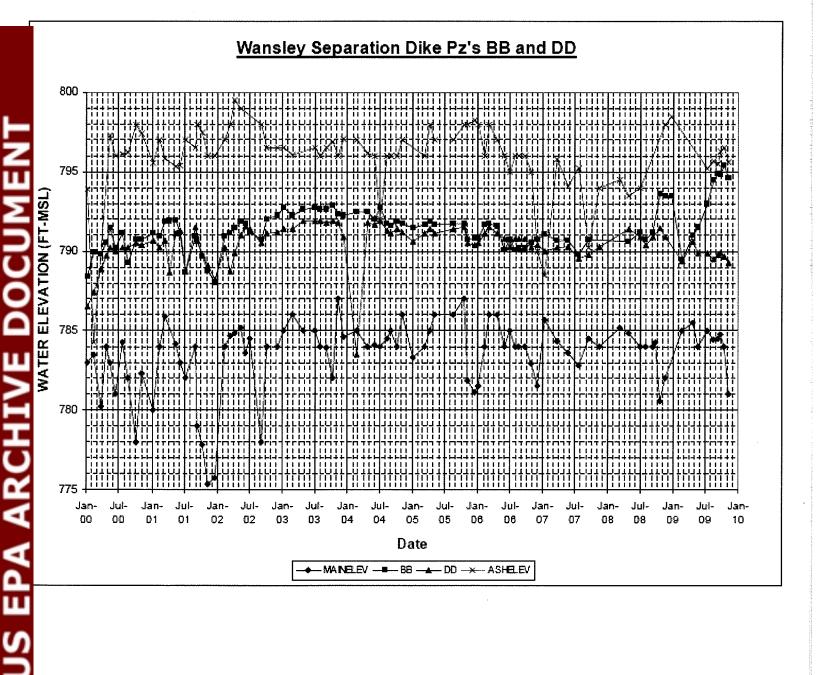
Figure 1:



Energy to Serve Your World

Project Plant Wansley	Prepared By Fred L. Cox, Jr.	Date 8/1/2010
Subject/Title Evaluate Stormwater capacity of Ash Pond	Reviewed By Courtenay O'Mara	Date 8/4/2010
	Calculation Number SH-WN10911-01	Sheet 6 of 10

Figure 2:





Energy to Serve Your World"

Project Plant Wansley	Prepared By Fred L. Cox, Jr.	Date 8/1/2010
Subject/Title Evaluate Stormwater capacity of Ash Pond	Reviewed By Courtenay O'Mara	Date 8/4/2010
	Calculation Number SH-WN10911-01	Sheet 7 of 10

Figure 3:

able A-								
eachtre	e city	Return	Period					
		1	2	5	10	25	50	100
	n	0.7669	0.8184	0.7769	0.7471	0.7191	0.7243	0.7107
	a	38.81	57.93	61.46	61.89	64.80	74.70	77.93
	b	8	11	12	12	12	13	13
Hours	Minutes		Intensity	12	12	12	13	13
0.08		5.43	5.99	6.80	7.45	8.45	9.21	9 99
0.06	5 6	5.43 5.13	5.70	6.51	7.45 7.14	8.45 8.11	9.21 8.85	9.99
	ž	4.86	5.44	6.24	6.86	7.80	8.53	9.27
	8	4.63	5.20	6.00	6.60	7.52	8.23	8.95
	9	4.42	4.99	5.77	6.36	7.26	7.96	8.66
	10	4.23	4.79	5.57	6.15	7.02	7.71	8.39
	11 12	4.06 3.90	4.62 4.45	5.38 5.20	5.95 5.76	6.80 6.59	7.48 7.26	8.14 7.91
	13	3.76	4.45	5.04	5.59	6.40	7.26	7.69
	14	3.63	4.16	4.89	5.43	6.22	6.86	7.49
0.25	15	3.50	4.03	4.75	5.28	6.06	6.69	7.30
	16	3.39	3.90	4.62	5.13	5.90	6.52	7.12
	17 18	3.29 3.19	3.79 3.68	4.49 4.38	5.00 4.88	5.75 5.61	6.36 6.21	6.95 6.79
	19	3.19	3.58	4.38 4.27	4.88 4.76	5.61 5.48	6.07	6.79
	20	3.01	3.49	4.16	4.65	5.36	5.94	6.49
	21	2.93	3.40	4.06	4.54	5.24	5.81	6.36
	22	2.86	3.31	3.97	4.44	5.13	5.69	6.23
	23 24	2.79 2.72	3.23 3.16	3.88	4.35	5.03	5.57	6.10
	25	2.72	3.10	3.80 3.72	4.25 4.17	4.92 4.83	5.46 5.36	5.99 5.87
	26	2.60	3.02	3.64	4.09	4.74	5.26	5.77
	27	2.54	2.95	3.57	4.01	4.65	5.16	5.66
	28	2.49	2.89	3.50	3.93	4.57	5.07	5.57
0.50	29	2.43	2.83	3.43	3.86	4.49	4.98	5.47
0.50	30 31	2.38 2.34	2.77 2.72	3.37 3.31	3.79 3.73	4.41 4.33	4.90 4.82	5.38 5.29
	32	2.29	2.67	3.25	3.66	4.26	4.74	5.21
	33	2.25	2.62	3.19	3.60	4.19	4.67	5.13
	34	2.21	2.57	3.14	3.54	4.13	4.59	5.05
	35	2.17	2.52	3.09	3.49	4.07	4.52	4.98
	36 37	2.13 2.09	2.48 2.44	3.04 2.99	3.43 3.38	4.00 3.95	4.46 4.39	4.90 4.83
	38	2.06	2.40	2.94	3.33	3.89	4.33	4.03
	39	2.03	2.36	2.90	3.28	3.83	4.27	4.70
	40	1.99	2.32	2.85	3.23	3.78	4.21	4.64
	41	1.96	2.28	2.81	3.19	3.73	4.15	4.58
	42 43	1.93 1.90	2.25 2.21	2.77 2.73	3.14 3.10	3.68 3.63	4.10 4.05	4.52 4.46
	44	1.87	2.18	2.69	3.06	3.58	4.00	4.40
0.75	45	1.85	2.15	2.66	3.02	3.54	3.95	4.35
	46	1.82	2.12	2.62	2.98	3.49	3.90	4.30
	47	1.80	2.09	2.59	2.94	3.45	3.85	4.25
	48 49	1.77 1.75	2.06 2.03	2.55 2.52	2.90	3.41	3.80	4.20
	50	1.75	2.03	2.52	2.87 2.83	3.37	3.76 3.72	4.15 4.10
	51	1.70	1.98	2.46	2.80	3.29	3.67	4.06
	52	1.68	1.95	2.43	2.77	3.26	3.63	4.01
	53	1.66	1.93	2.40	2.74	3.22	3.59	3.97
	54 55	1.64	1.90 1.88	2.37	2.71	3.18	3.55	3.93
	56	1.62 1.60	1.86	2.34	2.68 2.65	3.15	3.52 3.48	3.88 3.84
	57	1.58	1.83	2.32	2.62	3.12	3.44	3.84
	58	1.56	1.81	2.27	2.59	3.05	3.41	3.77
	59	1.54	1.79	2.24	2.56	3.02	3.37	3.73
1	60	1.53	1.77	2.22	2.54	2.99	3.34	3.69
2	120	0.97	1.19	1.44	1.60	1.85	2.07	2.24
<u>3</u>	180 360	0.69 0.40	0.82 0.49	1.02 0.61	1.16 0.70	1.33 0.81	1.47 0.91	1.62
12	720	0.40	0.49	0.61	0.70	0.81	0.91	0.98 0.58
24	1440	0.14	0.29	0.30	0.42	0.48	0.30	0.33

A-12 Georgia Stormwater Management Manual

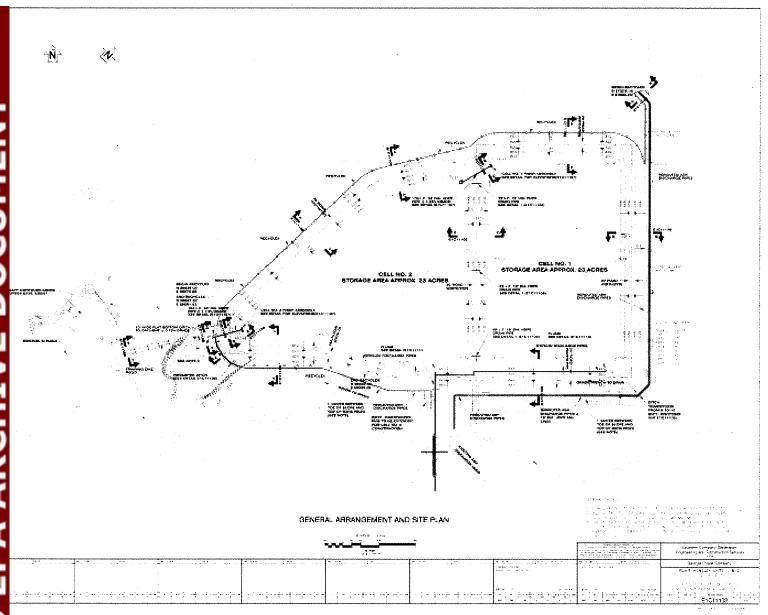
Volume 2 (Technical Handbook)



Energy to Serve Your World

Project Plant Wansley	Prepared By Fred L. Cox, Jr.		Date 8/1/2010	
Subject/Title Evaluate Stormwater capacity of Ash Pond	Reviewed By Courtenay O'Mara	MAI	Date 8/4/2010	
	Calculation Number SH-WN10911-01	<i>\(\frac{1}{2}\)</i>	Sheet 8 of 10	

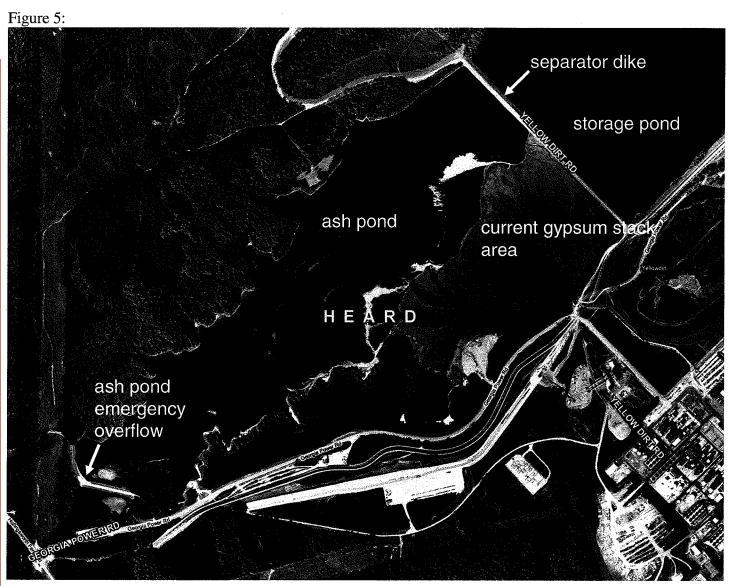
Figure 4:





Energy to Serve Your World"

Project Plant Wansley	Prepared By Fred L. Cox, Jr.		Date 8/1/2010
Subject/Title Evaluate Stormwater capacity of Ash Pond	Reviewed By Courtenay O'Mara	11 // 1	
	Calculation Number SH-WN10911-01	V	Sheet 9 of 10

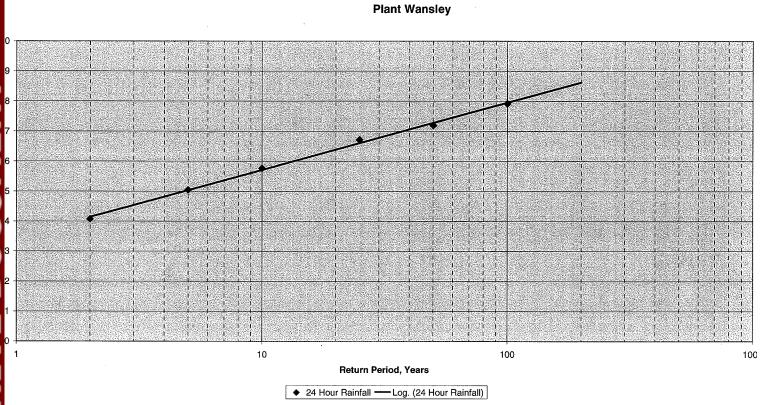




Energy to Serve Your World"

Project Plant Wansley	Prepared By Fred L. Cox, Jr.	Date 8/1/2010
Subject/Title Evaluate Stormwater capacity of Ash Pond	Reviewed By Courtenay O'Mara	Date 8/4/2010
	Calculation Number SH-WN10911-01	Sheet 10 of 10

Figure 6:





Site Name: Greorgita Powers - Unit Name: ASH Pond	•	Operator's Name:		THE PERSON NAMED IN COLUMN NAM
Unit I.D.:		Hazard Potential Classification: High	Significa	int Low
Inspector's Name: Frederic SH	MURAK	& JUSTIN CORN - No	10.	
tion the appropriate box below. Provine comments wh	en annronriata	If not applicable or not available, record "N/A". Any unusua large diked embankments, separate checklists may be used	condition	ns or
mbankment areas. If separate forms are used, identify a	pproximate area	that the form applies to in comments.	for differ	<u>ent</u>
	Yes No		Yes	No
Frequency of Company's Dam Inspections?	/	18. Sloughing or bulging on slopes?		
Pool elevation (operator records)?	796.5	19. Major erosion or slope deterioration?		-
3. Decant inlet elevation (operator records)?	800,29			
4. Open channel spillway elevation (operator records)?	802.57			11/1
5. Lowest dam crest elevation (operator records)?	805.0	Is water exiting outlet, but not entering inlet?		NI
 If instrumentation is present, are readings recorded (operator records)? 	Momh			N/A
7. Is the embankment currently under construction?	1	21. Seepage (specify location, if seepage carries fines, and approximate seepage rate below):		
Foundation preparation (remove vegetation,stumps, opsoil in area where embankment fill will be placed)?	N/A	From underdrain?		N/A
Trees growing on embankment? (If so, indicate largest diameter below)	/	At isolated points on embankment slopes?		1
Cracks or scarps on crest?	/	At natural hillside in the embankment area?		
Is there significant settlement along the crest?	/	Over widespread areas?		/
Are decant trashracks clear and in place?	N/A	From downstream foundation area?		/
Depressions or sinkholes in tailings surface or whirlpool in the pool area?	V	"Boils" beneath stream or ponded water?		1
4. Clogged spillways, groin or diversion ditches?	/	Around the outside of the decant pipe?		~
5. Are spillway or ditch linings deteriorated?	/	22. Surface movements in valley bottom or on hillside?		
5. Are outlets of decant or underdrains blocked?	V	23. Water against downstream toe?	V	
7. Cracks or scarps on slopes?	/	24. Were Photos taken during the dam inspection?	./	
lajor adverse changes in these items coul urther evaluation. Adverse conditions not olume, etc.) in the space below and on the spection Issue #	ed in these i	tems should normally be described levent	ocatio	٦,
Daily Weekly, Quenter	cy and	Annual		
N.P. WSE 795,0				
Monron U/S undermi	ning Po	Terricl & CRACKS Along	Cov	0,07
channel (NOT A Cof	7/155	We A+ The Timel		
o. No discharge from	n fond	(NONE observed History	Coll.	<u>//</u>
o. No discharge from B. Cooling worser Pond 1000ted Along DIS	n fond È Stoi	MONE Observer History	cell,	y) nd

U. S. Environmental Protection Agency

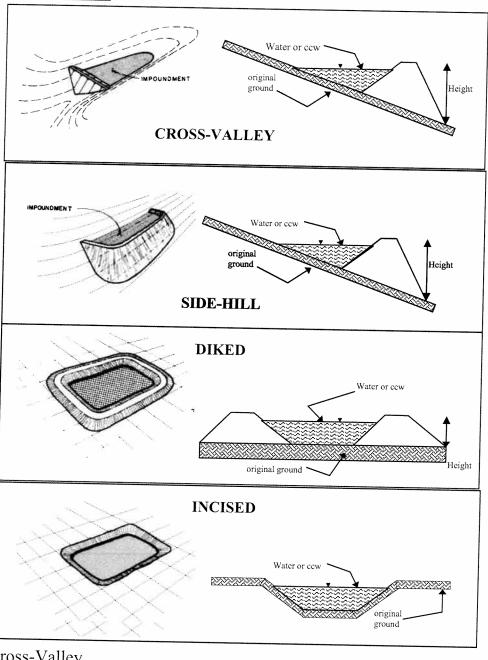


Coal Combustion Waste (CCW) Impoundment Inspection

Impoundment NPDES Permit # GA 0026778 INSPECTOR Dewberry
Date 30 JUN 2010
Impoundment Name Ash Poncl Impoundment Company Georgia Rowen EPA Region IV
EPA Region IV
State Agency (Field Office) Addresss
Name of Impoundment Ash Pool (Report each impoundment on a separate form under the same Impoundment NPDES
(Report each impoundment on a separate form under the same Impoundment NPDES Permit number)
New Update
Is impoundment currently under construction? Is water or ccw currently being pumped into the impoundment? Yes No Yes No
IMPOUNDMENT FUNCTION: Och Settling & Storage
Nearest Downstream Town: Name <u>Communicate</u> Distance from the impoundment <u>3.6 miles SW (Not pirected Distance of Distance of</u>
Does a state agency regulate this impoundment? YES NO
If So Which State Agency? GA Safe Dans ProGram -
caregon/2 Dom

HAZARD POTENTIAL (In the event the impoundment should fail, the following would occur):
LESS THAN LOW HAZARD POTENTIAL: Failure or misoperation of the dam results in no probable loss of human life or economic or environmental losses.
LOW HAZARD POTENTIAL: Dams assigned the low hazard potential classification are those where failure or misoperation results in no probable loss of human life and low economic and/or environmental losses. Losses are principally limited to the owner's property.
SIGNIFICANT HAZARD POTENTIAL: Dams assigned the significant hazard potential classification are those dams where failure or misoperation results in no probable loss of human life but can cause economic loss, environmental damage, disruption of lifeline facilities, or can impact other concerns. Significant hazard potential classification dams are often located in predominantly rural or agricultural areas but could be located in areas with population and significant infrastructure.
HIGH HAZARD POTENTIAL: Dams assigned the high hazard potential classification are those where failure or misoperation will probably cause loss of human life.
DESCRIBE REASONING FOR HAZARD RATING CHOSEN:
embenknest system as Low Hezerol

CONFIGURATION:



Cross-Valley			
Side-Hill			
Diked			
Incised (form comple	etion optional)		
Combination Inci	sed/Diked		
Embankment Height	10 feet	Embankment Material	C_:1
	43 acres	Liner NONE	3016
Current Freeboard 8	.5 feet	Liner Permeability	Ni /a

$\underline{\textbf{TYPE OF OUTLET}} \ (\text{Mark all that apply})$

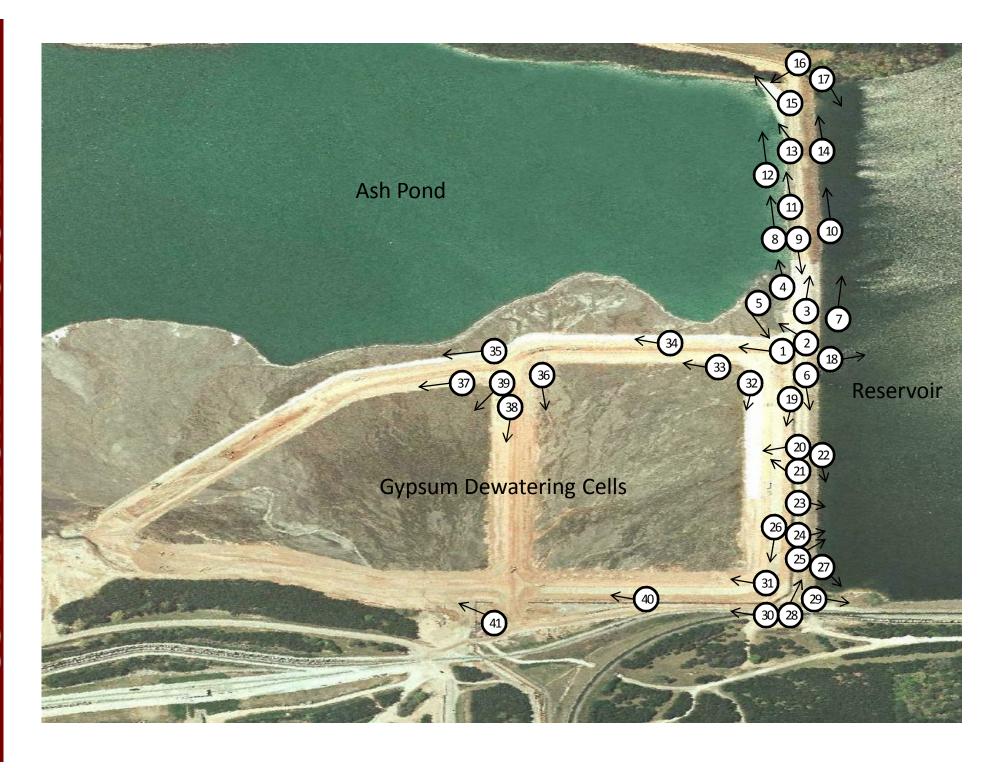
Open Channel Spillway	TRAPEZOIDAL	TRIANGULAR
Trapezoidal Emergerey Triangular Seilliuc Y Rectangular Irregular	Top Width Depth Bottom Width	Top Width Depth
depth bottom (or average) width top width	RECTANGULAR Depth Width	IRREGULAR Average Width Avg Depth
Outlet		
inside diameter		
Material corrugated metal welded steel concrete plastic (hdpe, pvc, etc.) other (specify)		Inside Diameter
s water flowing through the outlet?	YES NO	O
No Outlet		
Other Type of Outlet (special	fy) Broad cres	concrete wells
The Impoundment was Designed By	Southern	Company Services

Has there ever been a failure at this site? YES	NO
If So When?	
If So Please Describe :	

Has there ever been significant seepages at this site? YESNONO	
If So When?	
IF So Please Describe:	

Has there ever been any measures undertaken to monitor/lower Phreatic water table levels based on past seepages or breaches				
at this site?	YES	NO _	Sandar .	
If so, which method (e.g., piezometers,	gw pumping,)?			
If so Please Describe :				

		·····		
		······································		



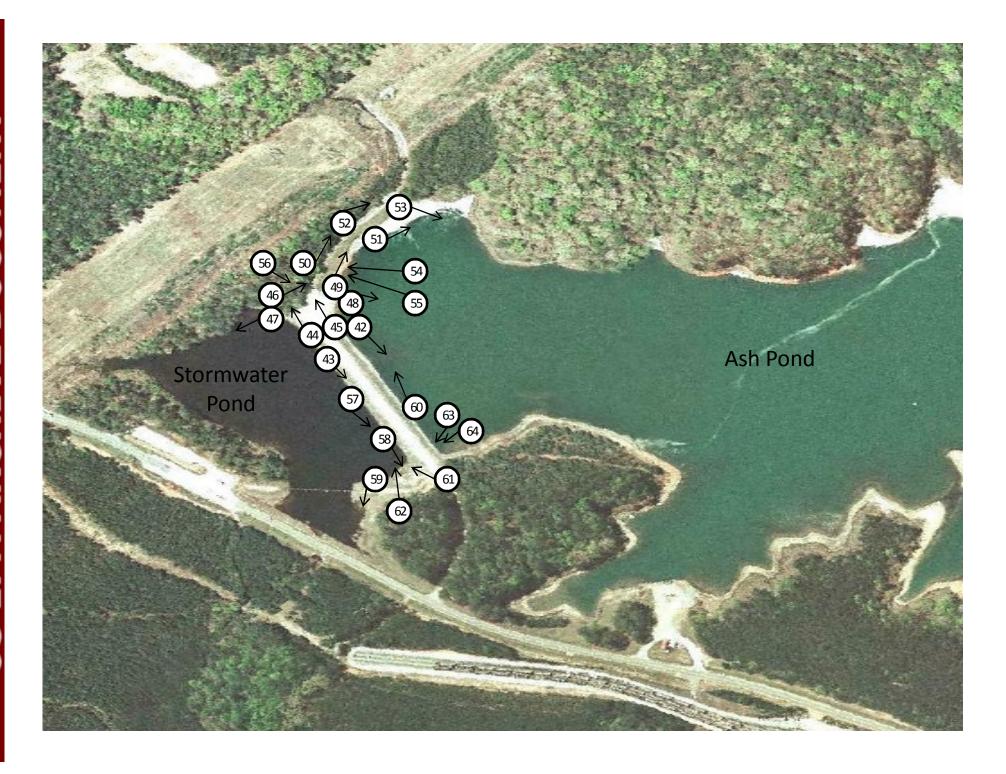




Photo 1: Upstream Slope adjacent to Gypsum Cells



Photo 2: Ash Pond inlet



Photo 3: Separation Dike Crest facing North



Photo 4: Close up where seeding occurred



Photo 5: Ash Pond Inlet



Photo 6: Separation Dike Crest downstream slope



Photo 7: Separation Dike downstream slope



Photo 8: Separation Dike upstream slope



Photo 9: Separation Dike upstream slope



Photo 10: Separation Dike downstream slope



Photo 11: Separation Dike upstream slope



Photo 12: Concrete ash piping support (not in use)



Photo 13: Separation Dike upstream slope



Photo 13B: Separation Dike upstream slope



Photo 14: Separation Dike downstream slope



Photo 15: Separation Dike left abutment upstream side



Photo 16: Separation Dike left abutment upstream side



Photo 17: Separation Dike left abutment downstream side



Photo 18: Erosion maintenance



Photo 19: Influent piping



Photo 20: Piezometer DD



Photo 21: Piezometer DD



Photo 22: Separation Dike downstream slope



Photo 23: Close up of downstream slope



Photo 24: Downstream slope erosion repair



Photo 25: Reservoir pond levels



Photo 26: Influent piping facing Gypsum Cells



Photo 27: Right abutment downstream slope



Photo 28: Separation Dike downstream slope



Photo 29: Right abutment facing reservoir



Photo 30: Influent piping



Photo 31: Piezometer BB



Photo 32: Gypsum Cell upstream slope



Photo 33: Gypsum Cell upstream slope



Photo 34: Gypsum Cell downstream slope facing Ash Pond



Photo 35: Gypsum Cell downstream slope facing Ash Pond



Photo 36: Gypsum Cell Discharge



Photo 37: Gypsum Cell upstream slope



Photo 38: Gypsum Cell upstream slope



Photo 39: Gypsum Cell upstream slope



Photo 40: Gypsum Cell downstream slope



Photo 41: Gypsum Cell downstream slope



Photo 42: Western Dike downstream slope



Photo 43: Western Dike downstream slope



Photo 44: Western Dike downstream slope



Photo 45: Western Dike overflow spillway



Photo 46: Stormwater channel



Photo 47: Stormwater channel discharge



Photo 48: Western Dike upstream slope



Photo 49: Western Dike upstream slope



Photo 50: Area adjacent to Western Dike



Photo 51: Area adjacent to Western Dike



Photo 52: Stormwater Channel



Photo 53: Western Dike upstream slope



Photo 54: Potential undermining at overflow



Photo 55: Potential undermining at overflow



Photo 56: Cracking along overflow channel



Photo 57: Western Dike downstream slope



Photo 58: Western Dike downstream slope



Photo 59: Stormwater Pond Dam







Photo 61: Western Dike outlet at Stormwater pond



Photo 62: Outlet at Stormwater pond



Photo 63: Western Dike outlet



Photo 64: Western Dike upstream slope and outlet